

ARCHITECT Boston, MA 02210

Bargmann Hendrie + Archetype, Inc. 9 Channel Center Street, Suite 300 (617) 350 0450

PROJECT NAME **Pembroke Community Center** 128 Center Street

Pembroke, MA, 02359

Town of Pembroke

PROJECT TEAM

100 Center Street

Pembroke, MA, 02359

Civil Engineer & Site Surveyor Merrill Engineers & Land Surveyors 427 Columbia Road Hanover, MA, 02339 (781) 826-9200

Landscape Architect Kyle Zick Landscape Architecture, Inc. 36 Bromfield Street, Suite 202 Boston, MA, 02108 (617) 451-1018

Structural Engineer Foley Buhl Roberts & Associates, Inc. 2150 Washington Street Newton, MA, 02462

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Audio/Visual Pro AV Systems 275 Billerica Road, Suite 3 Chelmsford, MA 01824 (978) 692-5111

> **Elevator Consultant** B Squared Engineering

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Technology and Security Building Technology Consulting 992 Bedford Street Bridgewater, MA 02424 (508) 819-1550

2 Addendum #2 08/03/2022

Room

DRAWING TITLE **Enlarged Plans and Elevations, MP**

DRAWING INFORMATION

DATE OF ISSUE

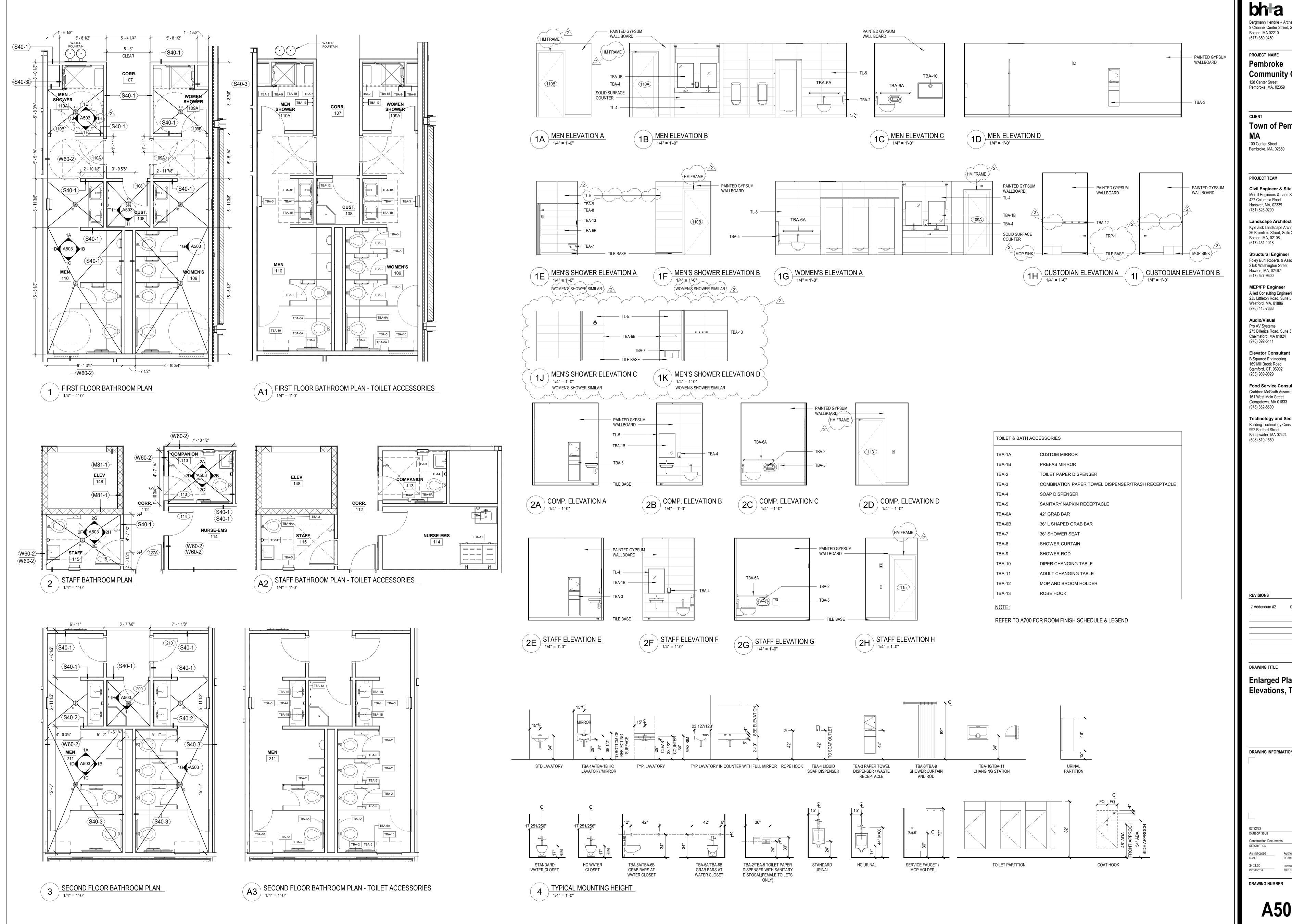
Construction Documents DESCRIPTION As indicated PROJECT#

DRAWING NUMBER

A501



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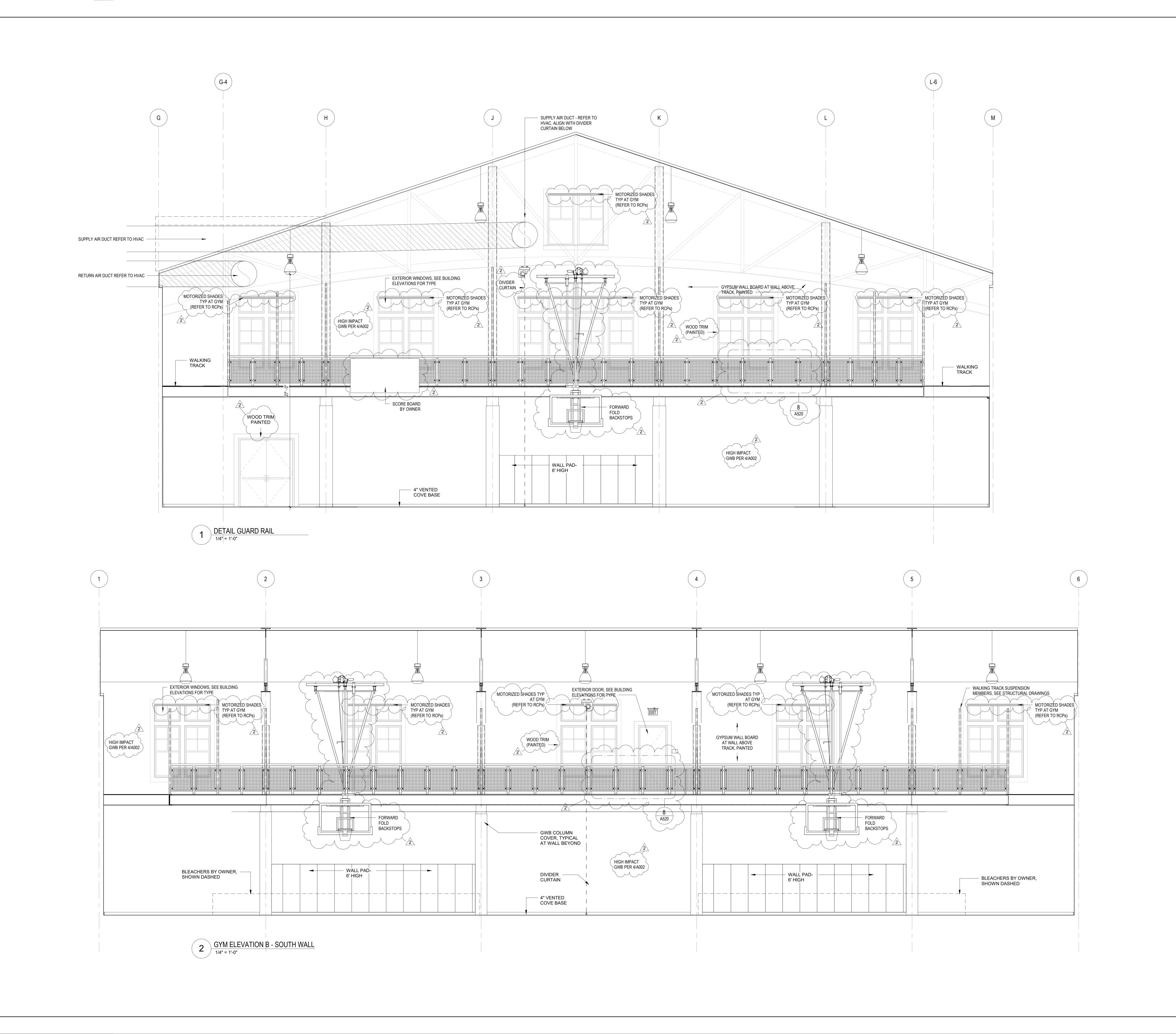
Enlarged Plans & **Elevations, Toilets**

DRAWING INFORMATION

Construction Documents DESCRIPTION

DRAWING NUMBER

A503



ARCHITECT

Control

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REVISIONS

2 Addendum #2 08/03/2022

DRAWING TITLE

Gym Elevations

DRAWING INFORMATION

07/22/22

Construction Documents

DESCRIPTION

1/4" = 1'-0" Author

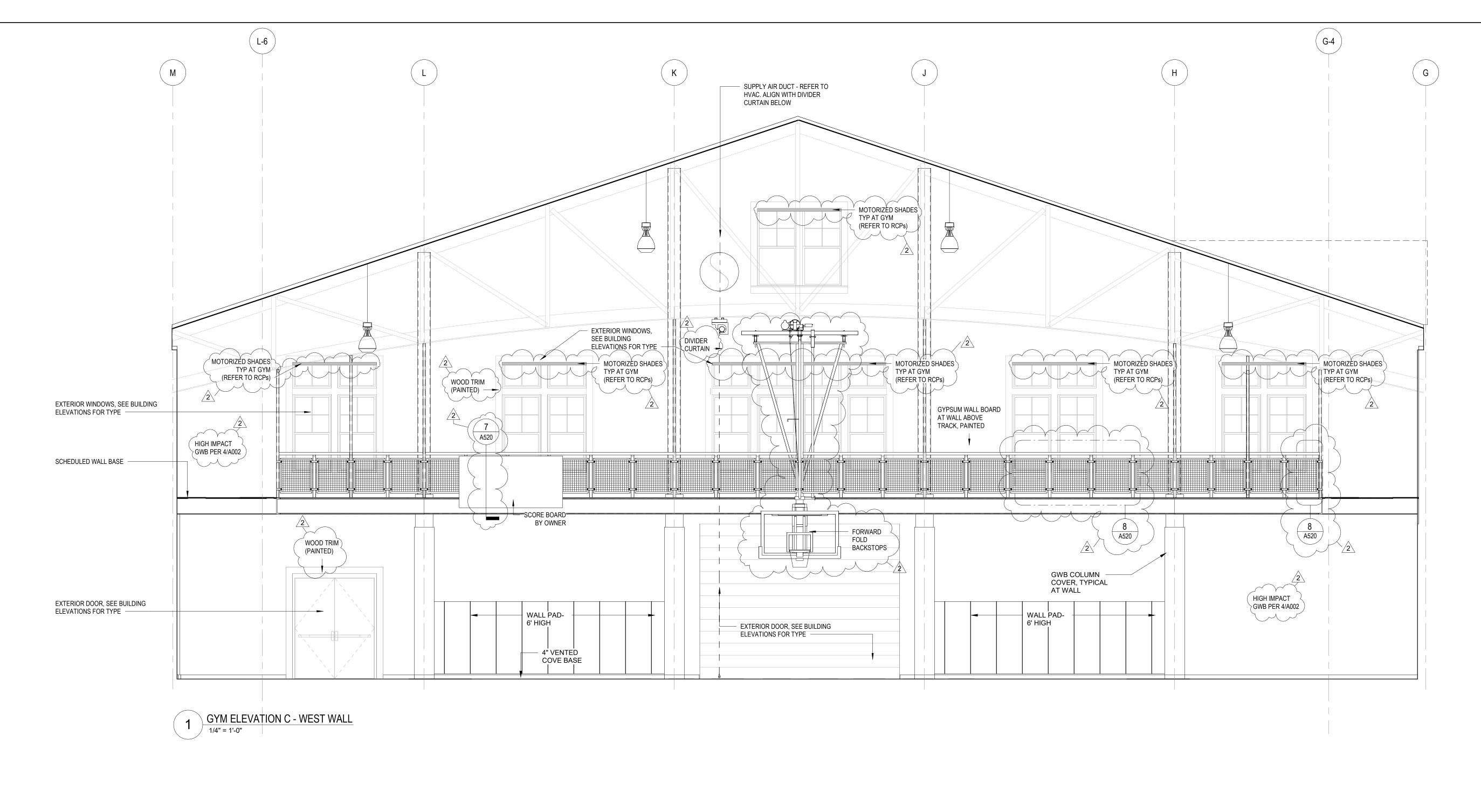
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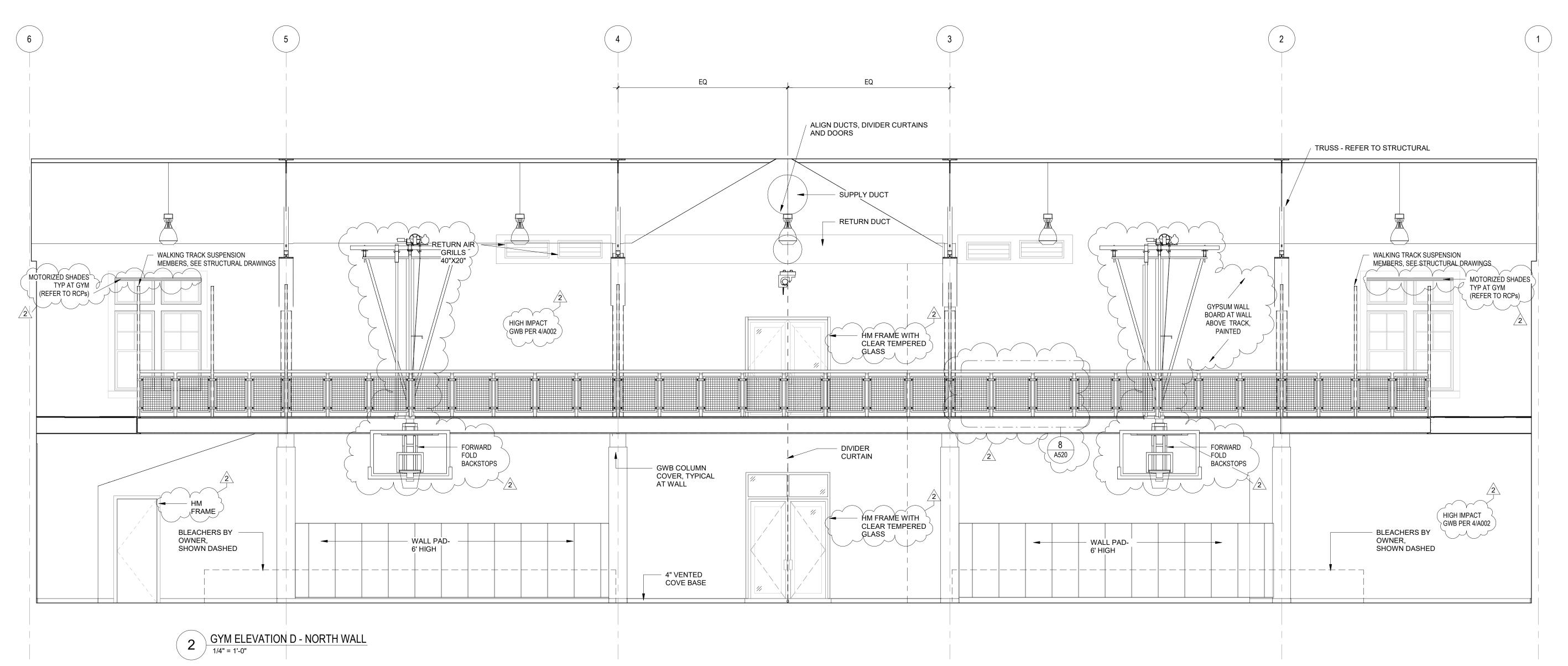
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ARCHITECT OF A

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Town of Pembroke MA

100 Center Street Pembroke, MA, 02359

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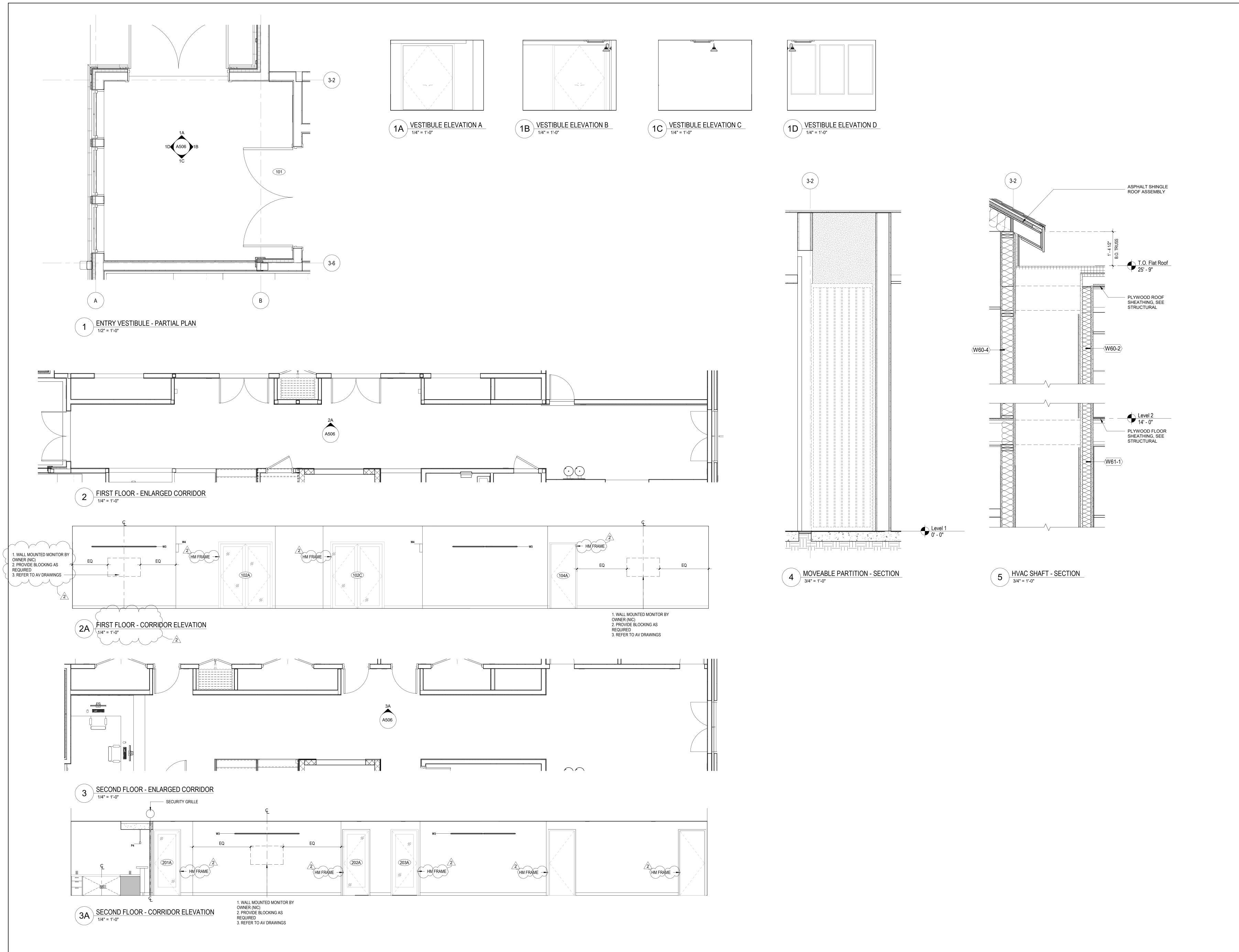
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Town of Pembroke

100 Center Street Pembroke, MA, 02359

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 2 Addendum #2
 08/03/2022

Enlarged Plans & Elevations

DRAWING INFORMATION

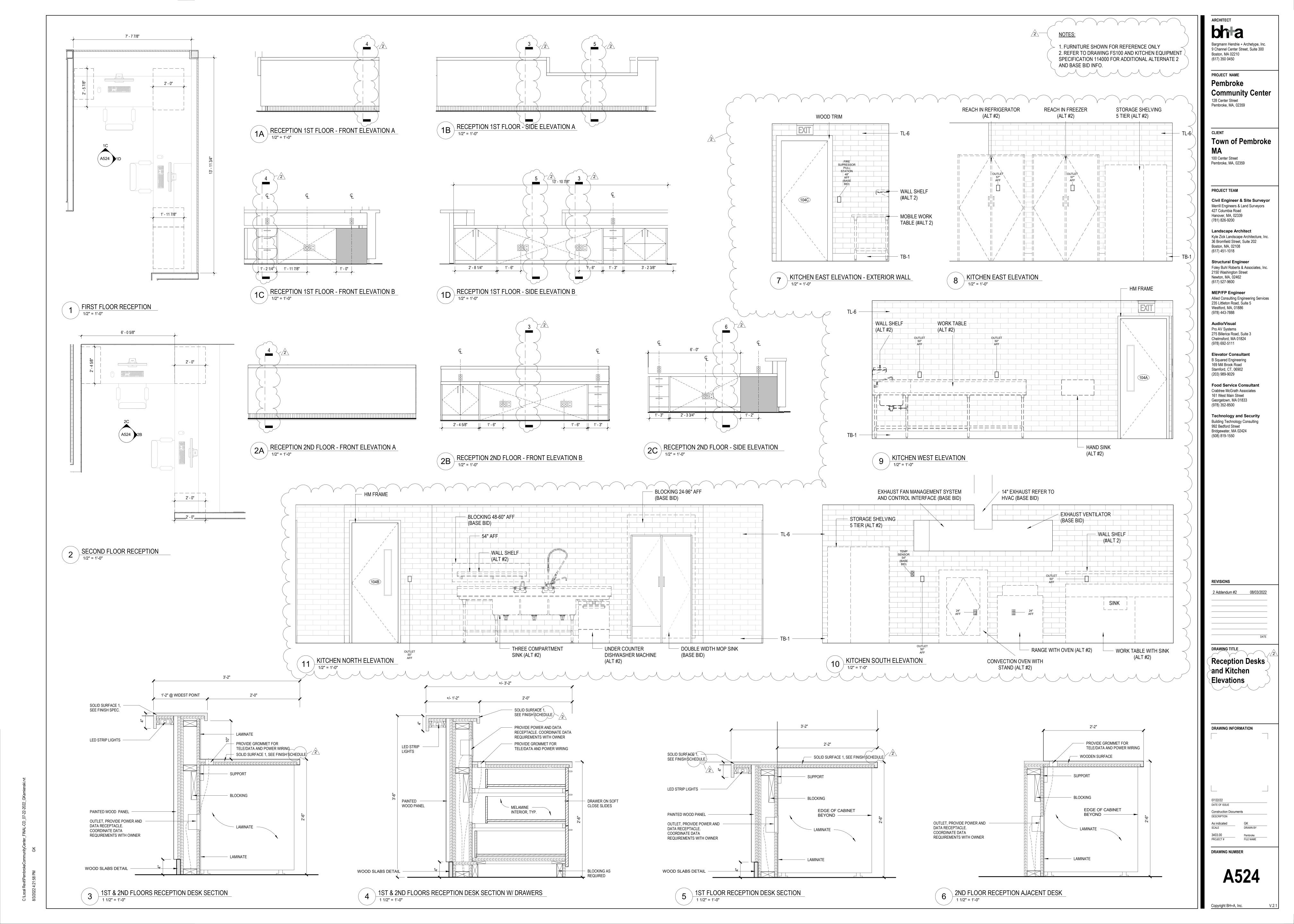
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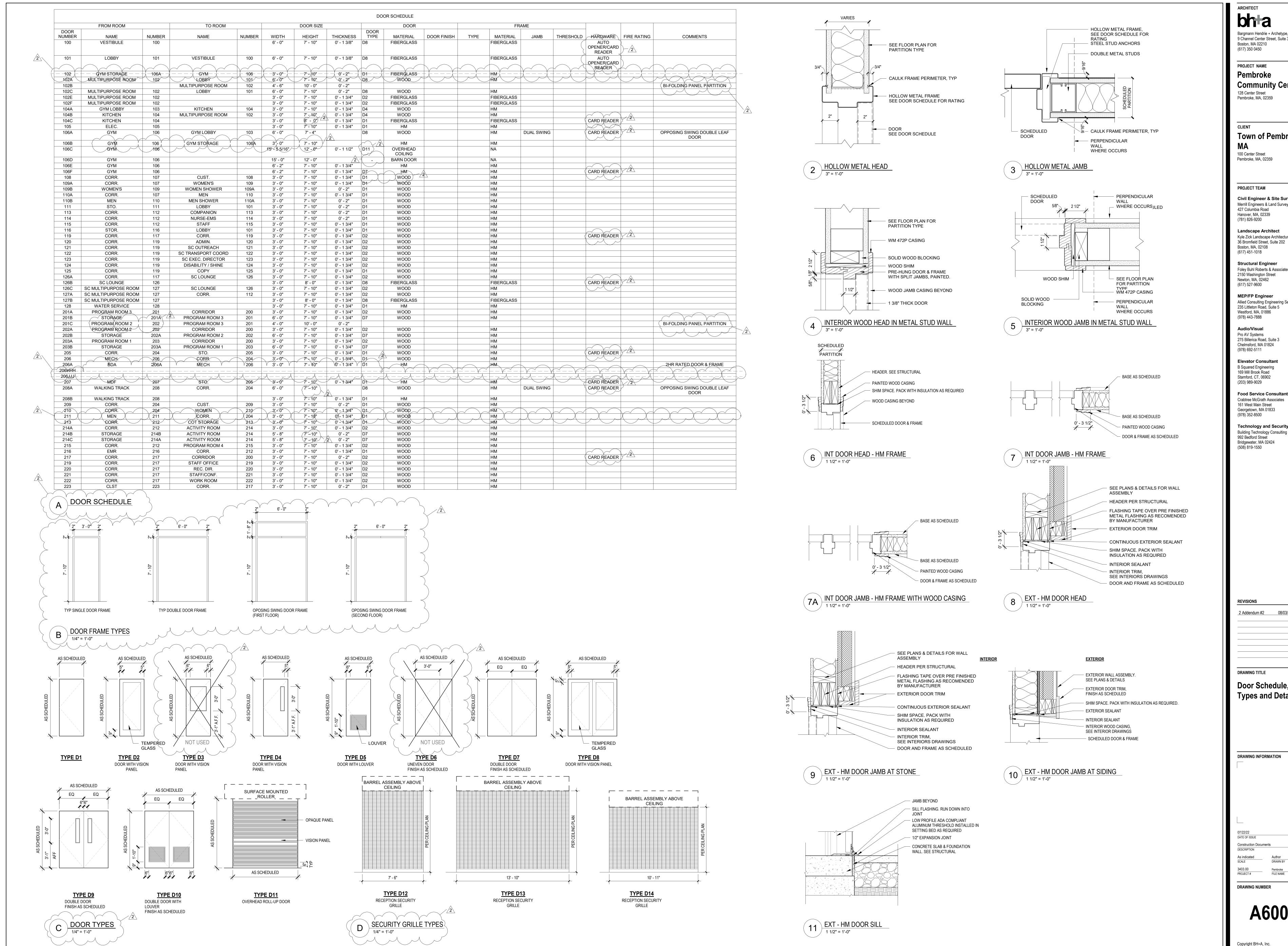
Construction Documents
DESCRIPTION
As indicated Author
DRAWN BY

3403.00 Pembroke

DRAWING NUMBER

A506





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Boston, MA 02210 (617) 350 0450 PROJECT NAME

Community Center 128 Center Street Pembroke, MA, 02359

Town of Pembroke

100 Center Street Pembroke, MA, 02359

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REVISIONS 2 Addendum #2 08/03/2022

Door Schedule, Types and Details

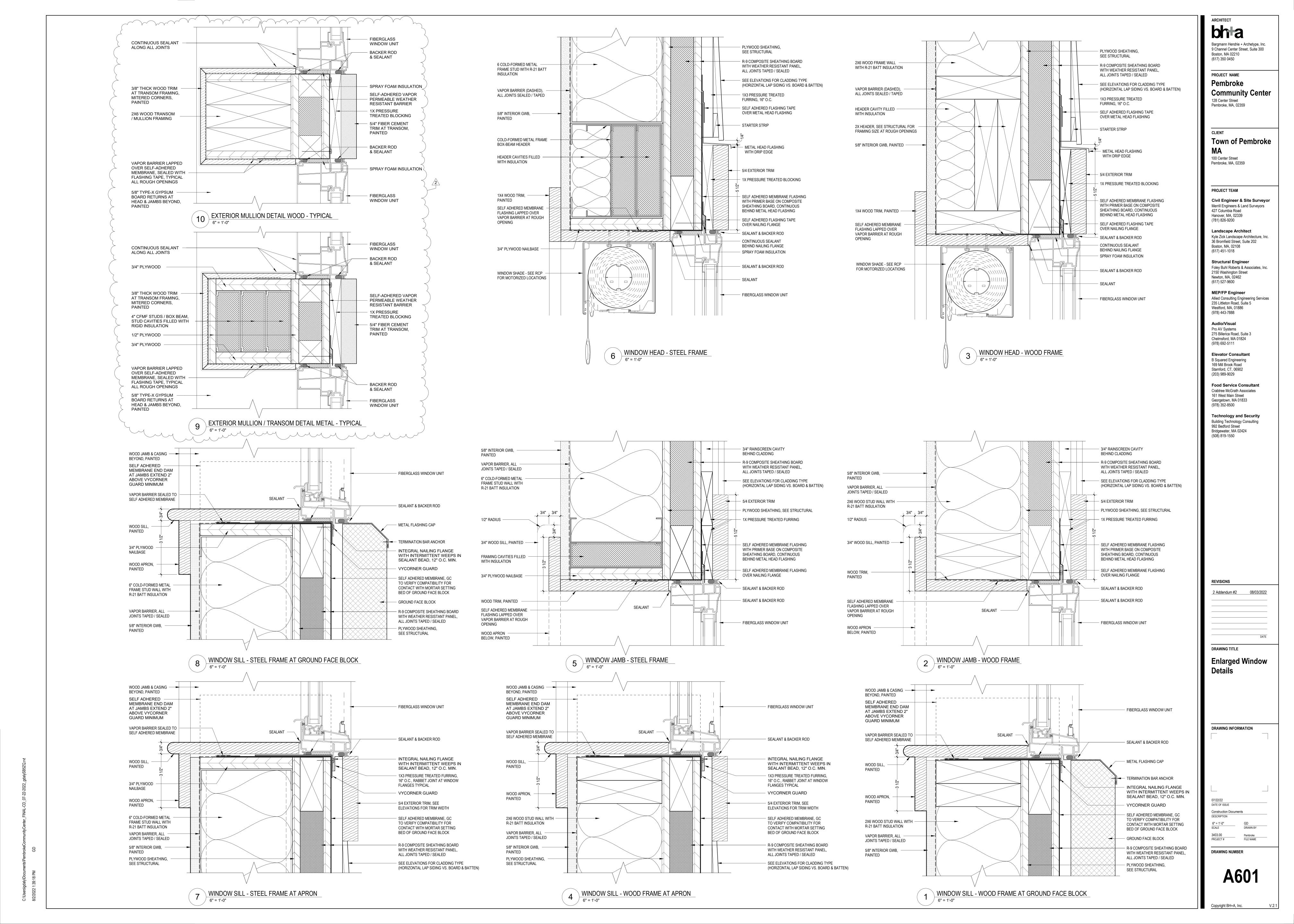
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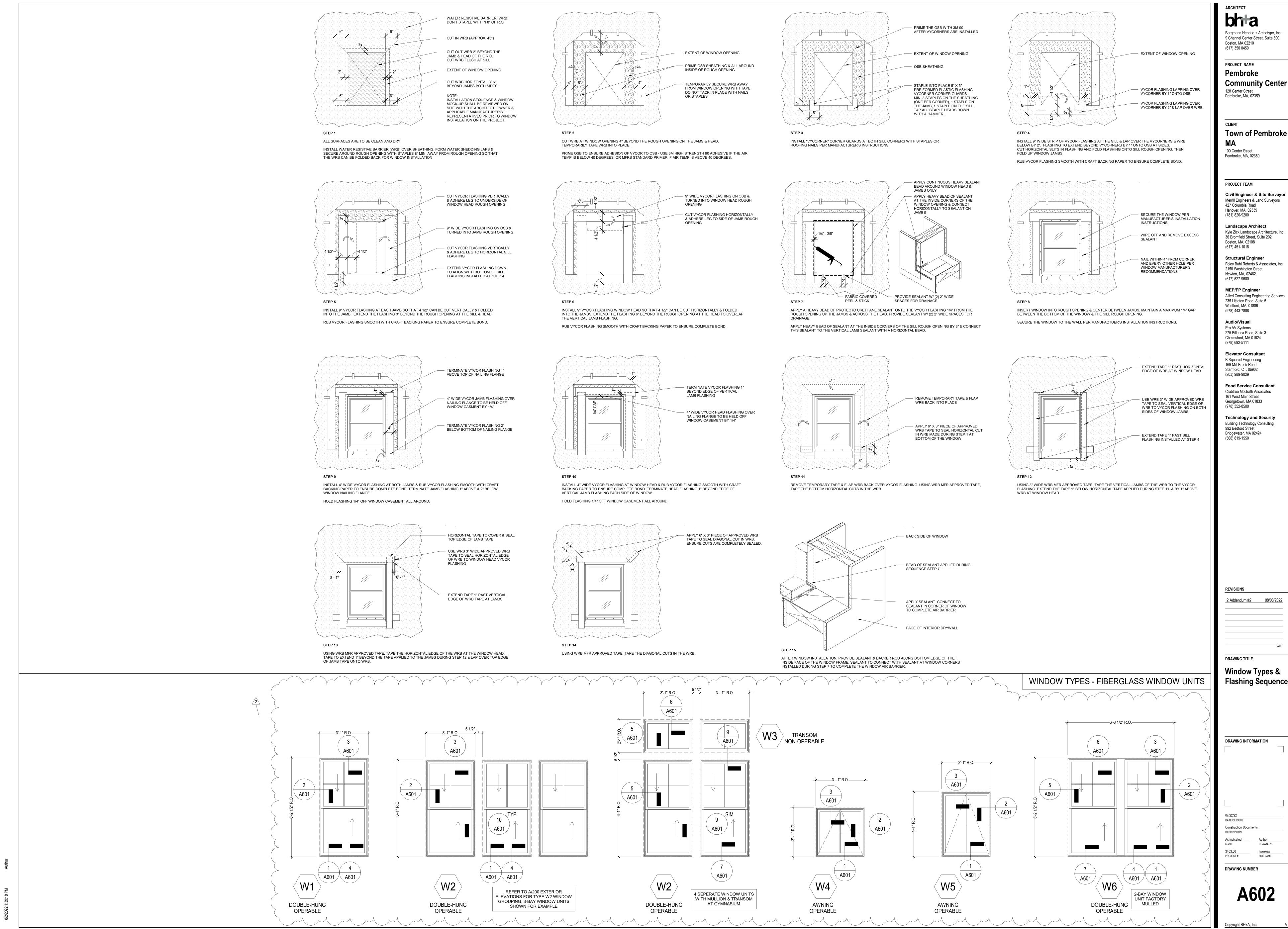
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Town of Pembroke

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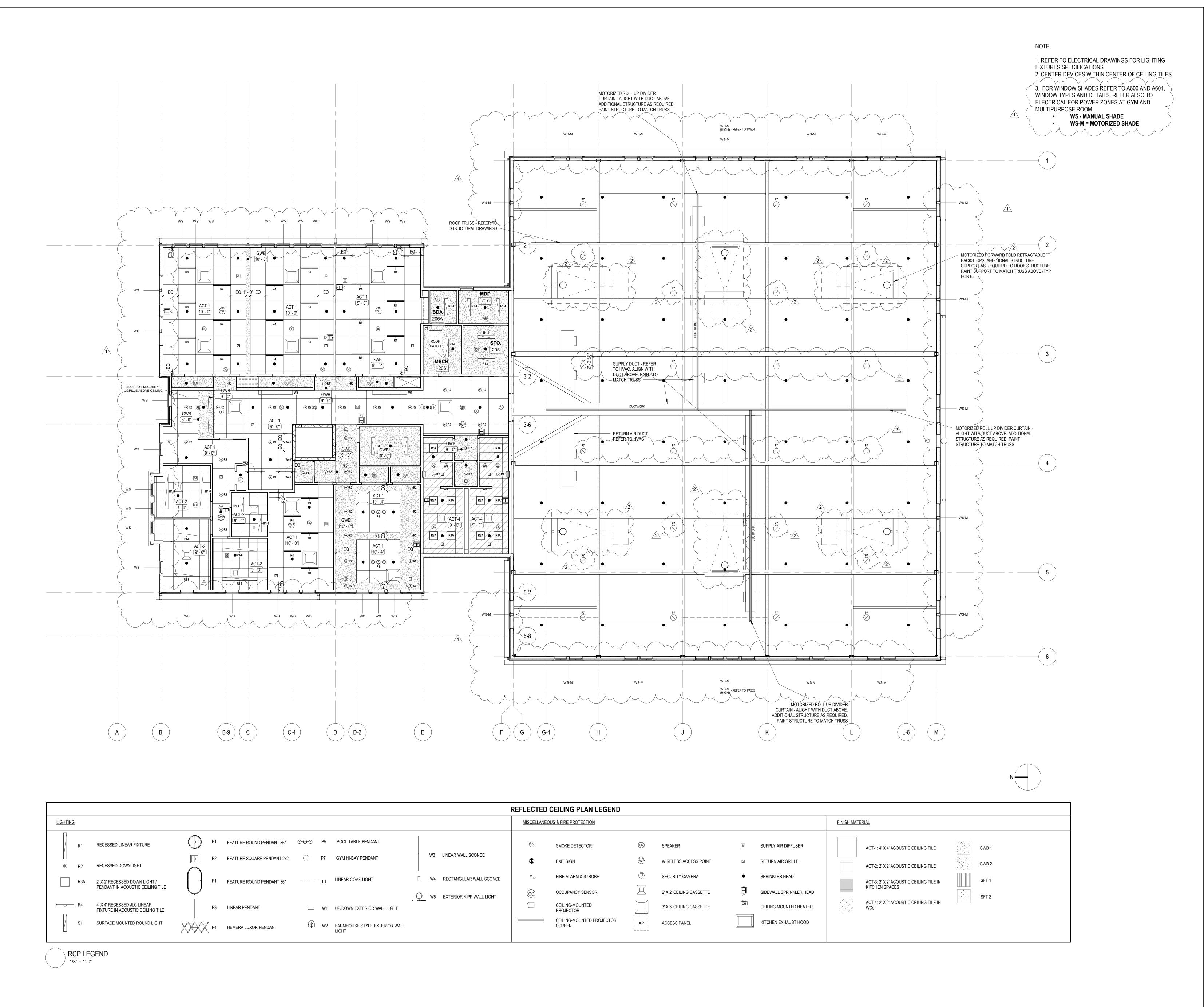
Allied Consulting Engineering Services

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Window Types & Flashing Sequence

A602



Bargmann Hendrie + Archetype, Inc. 9 Channel Center Street, Suite 300

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Pembroke
Community Center
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REVISIONS

1 Addendum #1 07/29/2022 2 Addendum #2 08/03/2022

DRAWING TITLE

Second Floor Reflected Ceiling

DRAWING INFORMATION

07/22/22
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Construction Documents

DESCRIPTION

DRAWING NUMBER

A802

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<u>GENERAL</u> CAST-IN-PLACE CONCRETE (SECTION 033000) STEEL DECK (SECTION 053100) **FOUNDATION (SECTION 310000)** G1. THE GENERAL NOTES APPLY UNLESS NOTED OTHERWISE ON THE DRAWINGS OR IN THE SPECIFICATIONS C1. CONCRETE WORK SHALL CONFORM TO THE LATEST "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318)". STEEL DECK WORK SHALL CONFORM TO "SPECIFICATION FOR DESIGN OF LIGHT GAUGE COLD-FORMED STEEL STRUCTURAL FOUNDATION WORK SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING REPORT BY McARDLE GANNON MEMBERS (AISI)": "STEEL DECK INSTITUTE DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS": ASSOCIATES, INC., FILE # W0890, DATED DECEMBER 28, 2021. G2. STRUCTURAL WORK SHALL CONFORM TO REQUIREMENTS OF THE "MASSACHUSETTS STATE BUILDING CODE" 780 CMR, NINTH CONCRETE SHALL BE PLACED IN THE PRESENCE OF THE APPROVED TESTING AGENCY. "STRUCTURAL STEEL WELDING CODE - STEEL (AWS D1.1)"; AND "STRUCTURAL WELDING CODE - SHEET STEEL (AWS D1.3)". THE OWNER AND ARCHITECT ASSUME NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS EDITION AND ALL AMENDMENTS. CONCRETE QUALITY IN ACCORDANCE WITH THE REQUIREMENTS OF THESE DRAWINGS AND SPECIFICATIONS IS ESSENTIAL TO COMPOSITE STEEL FLOOR DECK SHALL BE MADE FROM STEEL CONFORMING TO ASTM A653 - MINIMUM GRADE 40 (Fy = 40 KSI) DESCRIBED ON THE DRAWINGS, SPECIFICATIONS, BORING LOGS, OR TEST PITS. THIS DATA IS INCLUDED ONLY TO ASSIST G3. THE INTENT OF THE STRUCTURAL DRAWINGS IS TO SHOW THE MAIN STRUCTURAL FEATURES AND DESIGN FOR THE THE STRUCTURAL PERFORMANCE OF THE BUILDING. CONCRETE THAT IS NOT IN ACCORDANCE WITH THE DRAWINGS AND AND GALVANIZED IN ACCORDANCE WITH ASTM A653, COATING CLASS G60. COMPOSITE STEEL FLOOR DECK TYPE, DEPTH, AND THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND REPRESENT CONDITIONS ONLY OF THOSE COMPLETED PROJECT. ARCHITECTURAL DETAILS AND OTHER COMPONENTS THAT MAY BE NECESSARY TO CONSTRUCT THE SPECIFICATIONS WILL NOT BE ACCEPTED. GAUGE SHALL BE AS NOTED ON THE DRAWINGS. SPECIFIED LOCATIONS AT THE PARTICULAR TIME THEY WERE MADE. PROJECT ARE SHOWN INCIDENTALLY ONLY AND NOT COMPLETELY. C4. CONCRETE EXPOSED TO WEATHER SHALL CONTAIN AN AIR ENTRAINMENT ADMIXTURE. STEEL ROOF DECK SHALL BE MADE FROM STEEL CONFORMING TO ASTM A653 - GRADE 33 (Fy = 33 KSI) AND GALVANIZED IN THE CONTRACTOR SHALL INFORM THE ARCHITECT AND RELOCATE, AS REQUIRED, ANY EXISTING UTILITY LINES THAT MAY ACCORDANCE WITH ASTM A653, COATING CLASS G60. STEEL ROOF DECK TYPE, DEPTH, AND GAUGE SHALL BE AS NOTED ON THE G4. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, SITE, MECHANICAL, ELECTRICAL. INTERFERE WITH NEW FOUNDATIONS. THE CONTRACTOR SHALL REMOVE ANY EXISTING UTILITY LINES THAT ARE BEING PLUMBING. AND FIRE PROTECTION DRAWINGS. APPROVED SHOP DRAWINGS. AND SPECIFICATIONS. C5. NORMAL WEIGHT CONCRETE SHALL HAVE AN AIR-DRY UNIT WEIGHT OF 145 PCF ABANDONED IN THE VICINITY OF THE NEW FOUNDATION AND BACKFILL THE AREA WITH COMPACTED STRUCTURAL FILL. G5. REFER TO ARCHITECTURAL, SITE, MECHANICAL, ELECTRICAL, PLUMBING, AND FIRE PROTECTION DRAWINGS FOR C6. CONCRETE MINIMUM 28-DAY STRENGTH, UNLESS NOTED OTHERWISE, SHALL CONFORM TO FOLLOWING: PROVIDE CONTINUOUS POUR STOP ANGLES WITH RETURN LIP AT BUILDING PERIMETER AND AT ALL INTERIOR OPENINGS. REFER F4. THE BOTTOM SURFACE OF ALL SPREAD FOOTINGS SHALL REST ON A 3" THICK LAYER OF COMPACTED 3/4" CRUSHED FOOTINGS, PIERS, FOUNDATION WALLS, + GRADE BEAMS: 3000 PSI (NORMAL WEIGHT) VERIFICATION OF LOCATIONS AND DIMENSIONS OF ALL SHAFTS, INSERTS, CURBS, OPENINGS, SLEEVES, ANCHOR BOLTS, STONE OVER UNDISTURBED APPROVED SOIL OR COMPACTED STRUCTURAL FILL, WITH A MINIMUM ALLOWABLE BEARING FLOOR PITCHES, ANGLE FRAMES, AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS. SLABS ON GRADE + SLABS ON METAL DECK: 4000 PSI (NORMAL WEIGHT) PRESSURE OF 2.5 TONS PER SQUARE FOOT. REMOVE ALL ORGANICS, CLAYS, SILTS, OR UNSUITABLE OR UNCOMPACTED SITE PAVING: 4500 PSI (NORMAL WEIGHT) COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO THE SUPPORTING STRUCTURE WITH THE FOLLOWING MINIMUM FILL MATERIALS FROM BENEATH NEW FOOTINGS AND REPLACE WITH COMPACTED STRUCTURAL FILL. G6. THE CONTRACTOR SHALL INFORM THE ARCHITECT OF ALL DISCREPANCIES BETWEEN DRAWINGS OF DIFFERENT TRADES REQUIREMENTS: PRIOR TO INITIATION OF ANY WORK. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED BARS. LAP ALL CONTINUOUS BARS A MINIMUM OF PANEL ENDS AND END LAPS 5/8"Ø PUDDLE WELD AT 12" o.c. MAX. THE ESTIMATED BOTTOM ELEVATION OF EACH FOOTING IS INDICATED THUS [X'-X"] ON PLAN. THE BOTTOM OF EACH 40 DIAMETERS, UNLESS NOTED. PROVIDE MATCHING CORNER AND INTERSECTION BARS. INTERMEDIATE SUPPORTS 5/8"Ø PUDDLE WELD AT 12" o.c. MAX. EXTERIOR FOOTING SHALL BE A MINIMUM OF 4'-0" BELOW ADJACENT EXTERIOR FINISH GRADE. G7. EXISTING DIMENSIONS AND CONDITIONS MUST BE VERIFIED OR DETERMINED IN THE FIELD AND ANY DISCREPANCIES SHALL BE LONGITUDINAL EDGES AT SIDE SUPPORTS 5/8"Ø PUDDLE WELD AT 12" o.c. MAX. BROUGHT TO THE ATTENTION OF THE ARCHITECT BEFORE PROCEEDING WITH THE AFFECTED PORTION OF THE WORK. PROVIDE A MINIMUM OF #4 AT 12" EACH WAY, EACH FACE, FOR ALL WALLS, FOOTINGS, PITS, OR PADS, UNLESS NOTED SIDE LAPS OF ADJACENT UNITS BUTTON PUNCHED AT 24" o.c. MAX. PROVIDE A VAPOR BARRIER AND AN 8" MINIMUM THICKNESS OF 3/4" CRUSHED STONE UNDER INTERIOR CONCRETE SLABS G8. THE CONTRACTOR SHALL PROVIDE ALL THE NECESSARY ENGINEERED TEMPORARY BRACING AND SHORING TO SAFELY D6. CELLULAR ACOUSTIC STEEL ROOF DECK SHALL BE ATTACHED TO THE SUPPORTING STRUCTURE WITH THE FOLLOWING MINIMUM WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 1064 IN FLAT SHEETS. LAP ONE AND ONE-HALF SQUARES AT ALL JOINTS AND BACKFILL AGAINST WALLS, MATS, AND FOOTINGS SHALL BE ENGINEERED BACKFILL COMPACTED IN SPECIFIED LIFTS TO 95 SUPPORT THE NEW AND EXISTING WORK AND THE APPLIED LOADS UNTIL THE PERMANENT STRUCTURE IS FULLY INSTALLED REQUIREMENTS: PERCENT OF MAXIMUM DENSITY, UNLESS OTHERWISE INDICATED OR SPECIFIED. REFER TO GEOTECHNICAL REPORT AND TIE AT 3'-0" o.c. AT SLAB ON GRADE, PLACE WELDED WIRE FABRIC ON SLAB BOLSTERS WITH SAND PLATES SPACED AT 3'-0" o.c. PANEL ENDS AND END LAPS 5/8"Ø PUDDLE WELD AT EACH RIB 5/8"Ø PUDDLE WELD AT EACH RIB EARTHWORK SPECIFICATION FOR ADDITIONAL REQUIREMENTS. INTERMEDIATE SUPPORTS SHOP DRAWINGS FOR REINFORCING STEEL, STRUCTURAL STEEL, STEEL DECK AND PREFABRICATED WOOD TRUSSES SHALL LONGITUDINAL EDGES AT SIDE SUPPORTS 5/8"Ø PUDDLE WELD AT 12" o.c. MAX. BE SUBMITTED TO THE ARCHITECT AND A STAMPED APPROVAL RECEIVED BEFORE FABRICATION MAY PROCEED. FABRICATION PROVIDE REINFORCING STEEL DETAILING, LAP SPLICES, EMBEDMENTS, BAR SUPPORTS, SPACERS, AND ACCESSORIES AS VERIFY LOCATIONS AND REQUIREMENTS FOR INSERTS, SLEEVES, CONDUITS, EMBEDMENTS, AND PENETRATIONS WITH SIDE LAPS OF ADJACENT UNITS 1-1/2" SEAM WELD AT 24" o.c. MAX. RECOMMENDED IN THE LATEST EDITION OF THE "ACI DETAILING MANUAL". ACCESSORIES, SUCH AS SLAB BOLSTERS AND BEAM AND AND ERECTION SHALL PROCEED FROM APPROVED SHOP DRAWINGS ONLY. RESPECTIVE TRADES BEFORE PLACING CONCRETE. SLAB CHAIRS, IN CONTACT WITH EXPOSED SURFACES, SHALL BE ZINC COATED AND PLASTIC TIPPED. DO NOT HANG M.E.P., F.P. OR ARCHITECTURAL COMPONENTS FROM METAL ROOF DECK. PROVIDE ENGINEERED SUPPLEMENTAL G10. NOTES AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS, FRAMING BY THE COMPONENT CONTRACTOR TO SPAN BETWEEN STRUCTURAL BEAMS. FOUNDATIONS SHALL BE CENTERED UNDER SUPPORTED MEMBERS, UNLESS NOTED OTHERWISE. C11. REINFORCING STEEL DETAILS NOT SHOWN ON THE DRAWINGS SHALL BE IN ACCORDANCE WITH THE "ACI DETAILING MANUAL 2004". F10. DOWELS FROM FOUNDATIONS INTO PIERS, COLUMNS, BUTTRESSES, OR WALLS SHALL BE THE SAME SIZE AND NUMBER AS ROUGH CARPENTRY (SECTION 061000) REINFORCEMENT IN PIERS, COLUMNS, AND BUTTRESSES, OR WALLS ABOVE, UNLESS NOTED OTHERWISE. G11. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN. C12. CLEAR CONCRETE COVER FOR REINFORCING BARS OR WELDED WIRE FABRIC SHALL CONFORM TO THE FOLLOWING, UNLESS NOTED: FOOTINGS: **FOUNDATION WALLS:** RC1. STRUCTURAL LUMBER SHALL CONFORM TO THE AF&PA, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AND NO CONCRETE SHALL BE PLACED UNDER WATER OR ON FROZEN SUBGRADE. PROTECT IN-PLACE FOUNDATIONS AND G12. THE CONTRACTOR SHALL SUBMIT LOADING DATA FOR ANY EQUIPMENT SUCH AS LIFTS, ETC. PROPOSED FOR USE ON THE 1-1/2" SUPPLEMENT, "DESIGN VALUES FOR WOOD CONSTRUCTION" LATEST EDITION. MAXIMUM MOISTURE CONTENT SHALL BE BUILDING DURING CONSTRUCTION. PILASTERS/PIERS: 1-1/2" TO TIES SLABS FROM FROST PENETRATION UNTIL PROJECT IS COMPLETED. 1-1/2" FROM TOP INTERIOR SLABS ON GRADE G13. BUILDING CONSTRUCTION TYPE IS IIIB, FLOOR AND ROOF CONSTRUCTION ARE TO BE CONSIDERED RESTRAINED. MID-DEPTH F12. DO NOT BACK FILL FOUNDATION WALLS UNTIL WALLS HAVE REACHED THEIR 28 DAY STRENGTHS AND FLOOR SLABS AT EXTERIOR SLABS ON GRADE: RC2. SAWN LUMBER SHALL BE SPRUCE-PINE-FIR NO. 2 OR BETTER, INCLUDING JOISTS, RAFTERS, BEAMS, STUDS, POSTS AND TOPPINGS ON METAL DECK: 1" FROM TOP THE TOP AND BOTTOM OF WALLS ARE IN PLACE AND CURED. STRUCTURAL LOADS - MASSACHUSETTS STATE BUILDING CODE (780 CMR) - NINTH EDITION C13. SET AND TIE ALL REINFORCING STEEL BEFORE PLACING CONCRETE. SETTING DOWELS AND REINFORCING STEEL INTO WET (I.B.C. 2015 WITH MASSACHUSETTS AMENDMENTS RC3. FOUNDATION SILLS SHALL BE PRESERVATIVE PRESSURE TREATED SOUTHERN PINE NO. 2 OR BETTER. ANCHOR BOLTS IN CONCRETE IS PROHIBITED. PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIP GALVANIZED. NAILS IN PRESERVATIVE PRESSURE TREATED **ABBREVIATIONS** L1. DEAD LOADS WOOD SHALL BE STAINLESS STEEL NO REINFORCING STEEL SHALL BE CUT OR OMITTED IN THE FIELD BECAUSE OF CONFLICT WITH SLEEVES, DUCT OPENINGS, OR A. WEIGHT OF BUILDING COMPONENTS RECESSES. REINFORCING STEEL MAY BE MOVED ASIDE WITHOUT CHANGE IN LEVEL, WITH THE APPROVAL OF THE ARCHITECT. TYPICAL FLOOR RC4. WOOD EXPOSED TO WEATHER SHALL BE PRESERVATIVE PRESSURE TREATED SOUTHERN PINE NO. 2 OR BETTER. BOLTS IN SUSPENDED TRACK IN GYMNASIUM 75 PSF AESS ARCHITECTURALLY EXPOSED STRUCTURAL STEEL LBS POUNDS PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIP GALVANIZED. NAILS IN PRESERVATIVE PRESSURE TREATED TYPICAL ROOF AREAS 25 PSF C15. NO CHASES, RECESS, OPENINGS, OR SLEEVES SHALL BE INSTALLED IN CONCRETE WITHOUT APPROVAL OF THE ARCHITECT. AFF LEFT END ABOVE FINISHED FLOOR WOOD SHALL BE STAINLESS STEEL. GYMNASIUM ROOF (INCLUDING FUTURE PV ALLOWANCE) 45 PSF ALTERNATE C16. NO CONDUITS SHALL BE PLACED IN CONCRETE SLABS ON METAL DECK B. TYPICAL PARTITIONS ALLOWANCE - (I.B.C. - SECTION 1607.5) 15 PSF LIVE LOAD ACI AMERICAN CONCRETE INSTITUTE RC5. LAMINATED VENEER LUMBER (LVL) SHALL BE MICRO-LAM, AS MANUFACTURED BY WEYERHAEUSER, OR EQUAL. AMERICAN FOREST & PAPER ASSOCIATION LONG LEG HORIZONTAL C17. KEYS SHALL BE A MINIMUM OF 2" x 4" WITH BEVELED SIDES, UNLESS NOTED OTHERWISE L2. SNOW LOADS AMERICAN INSTITUTE OF STEEL CONSTRUCTION LONG LEG VERTICAL RC6. PARALLEL STRAND LUMBER (PSL) SHALL BE PARALLAM, AS MANUFACTURED BY WEYERHAEUSER, OR EQUAL. GROUND SNOW LOAD - (MA TABLE 1604.11) = 30 PSEAISI AMERICAN IRON AND STEEL INSTITUTE LOCATION SNOW IMPORTANCE FACTOR - (ASCE 7-10 - TABLE 1.5-2) C18. DOWELS AND ANCHOR RODS SHALL BE SET BY TEMPLATE. SET EMBEDDED ITEMS FOR CONNECTION OF OTHER WORK = 1.0 AITC LONGITUDINAL AMERICAN INSTITUTE OF TIMBER CONSTRUCTION ENGINEERED I - JOISTS SHALL BE TJI JOISTS, AS MANUFACTURED BY WEYERHAEUSER, OR EQUAL. PROVIDE WEB ACCURATELY. EXPOSURE FACTOR - (ASCE 7-10 - TABLE 7-2) = 1.0 ASTM I OW POINT AMERICAN SOCIETY OF TESTING AND MATERIALS STIFFENERS/SQUASH BLOCKS PER MANUFACTURER'S RECOMMENDATIONS. THERMAL FACTOR - (ASCE 7-10 - TABLE 7-3) = 1 1 LOAD AND RESISTANCE FACTOR DESIGN AWS AMERICAN WELDING SOCIETY C19. HORIZONTAL CONSTRUCTION JOINTS SHALL BE AS INDICATED ON THE DRAWINGS. VERTICAL CONSTRUCTION JOINTS SHALL BE FLAT ROOF SNOW LOAD - (ASCE 7-10 - SECTION 7.3) = 30 PSF ANC ANCHOR LIGHT GAGE, GAUGE RC8. FLUSH FRAMED CONNECTIONS SHALL HAVE METAL BEAM OR JOIST HANGERS, MANUFACTURED BY SIMPSON STRONG-TIE APPROVED BY THE ARCHITECT. CONSTRUCTION JOINTS SHALL BE FORMED WITH A STANDARD KEY AND ALL REINFORCING STEEL ROOF SLOPE FACTOR - (ASCE 7-10 - FIGURE 7-2) = 1.0 & OR + I ONG WAY EXTENDED A MINIMUM OF 40 DIAMETERS, UNLESS NOTED. ALL CONSTRUCTION JOINTS BELOW GRADE SHALL HAVE CONTINUOUS SLOPED ROOF SNOW LOAD (ASCE 7-10 - SECTION 7.4 $P_s = 21 PSF$ **APPROX APPROXIMATELY** LWT LIGHTWEIGHT UNBALANCED SNOW LOAD (ASCE 7-10 - SECTION 7.6) 40 PSF OVER 9 FEET WIDTH ALONG RIDGE BENTONITE WATERSTOPS ARCH ARCHITECT OR ARCHITECTURAL LWC LIGHTWEIGHT CONCRETE RC9. ALL INDIVIDUAL POSTS SHALL HAVE METAL CAPS AND BASES, MANUFACTURED BY SIMPSON STRONG-TIE CO., INC., OR SLIDING SNOW LOAD ON LOWER ROOF AREAS (ASCE 7-10 - SECTION 7.9) 70 PSF AT RATE OF C20. CONSTRUCTION AND CONTROL JOINT LOCATIONS OTHER THAN THOSE SHOWN ON THE DRAWINGS MAY BE PERMITTED SUBJECT SNOW DRIFT - (ASCE 7-10 - FIGURES 7-7, 7-8 & 7-9) MAS MASONRY) THE PRIOR APPROVAL OF THE ARCHITECT. EXPANSION JOINT LOCATIONS ARE MANDATORY AS SHOWN. **MANUF** *MANUFACTURER* **BALANCE** RC10. ROOF SHEATHING SHALL BE A MINIMUM OF 5/8" NOMINAL (19/32" ACTUAL) EXTERIOR GRADE (EXPOSURE 1) APA RATED L3. LIVE LOADS MATLBLK MATERIAL STRUCTURAL 1 PLYWOOD SHEATHING WITH 10d NAILS 6" o.c. AT EDGES AND 12" o.c. AT INTERMEDIATE SUPPORTS. A. LOADS (I.B.C. - TABLE 1607.1) C21. SEE ARCHITECTURAL AND SITE DRAWINGS FOR FINISHES, DEPRESSIONS, REGLETS, NOTCHES, AND OTHER ARCHITECTURAL MAXMAXIMUM PROVIDE METAL "H" CLIPS AT PANEL EDGES. 65 PSF (INCLUDING PARTITION ALLOWANCE) OFFICES MECH MECHANICAL, ELECTRICAL, PLUMBING REARING CORRIDORS ABOVE THE FIRST FLOOR MEZZ **BOTH SIDES** MEZZANINE RC11. WALL SHEATHING SHALL BE A MINIMUM OF 1/2" NOMINAL (15/32" ACTUAL) EXTERIOR GRADE (EXPOSURE 1) APA RATED 100 PSF PROVIDE CONCRETE PADS FOR MECHANICAL EQUIPMENT ACCORDING TO THE REQUIREMENTS OF THE MANUFACTURER AND IN MID MIDDLE B OR BOT BOTTOM STRUCTURAL 1 PLYWOOD SHEATHING WITH 8d NAILS 6" o.c. AT EDGES AND 12" o.c. AT INTERMEDIATE SUPPORTS. ACCORDANCE WITH THE TYPICAL DETAILS. COORDINATE LOCATIONS WITH M.E.P. WORK. GYMNASIUM AND OPEN PLAN AREAS 100 PSF MID-PT MID-POINT BOTTOM OF BLOCK ALL EDGES OF PLYWOOD WALL SHEATHING. STORAGE MINIMUM BOTTOM EACH WAY C23. PROVIDE SEALANT FOR ALL EXPOSED-TO-VIEW CONSTRUCTION JOINTS, CONTROL JOINTS, AND SHEAR KEYS. 150 PSF (100PSF+50PSF EQUIPMENT PADS) MECHANICAL EQUIPMENT ROOMS **MISCELLANEOUS** BETWN BETWEEN RC12. FLOOR SHEATHING SHALL BE A MINIMUM OF 3/4" NOMINAL (23/32" ACTUAL) EXTERIOR GRADE (EXPOSURE 1) APA RATED LIVE LOAD REDUCTION (I.B.C. - SECTION 1607.10) MASONRY OPENING **BOTTOM LONG WAY** STRUCTURAL 1 PLYWOOD TONGUE AND GROOVE, GLUED AND NAILED WITH 10d NAILS AT 6" o.c. AT ENDS C24. EXPOSED EDGES OF CONCRETE ELEMENTS SHALL HAVE A 1-INCH CHAMFER UNLESS NOTED OTHERWISE CONSTRUCTION LOADS BUILDING OFFICIALS AND CODE ADMINISTRATORS AND 12" o.c. AT INTERMEDIATE SUPPORTS. MINIMUM WALL REINFORCING BASE PLATE C25. NOT ALL OPENINGS THROUGH CONCRETE SLABS AND WALLS ARE SHOWN ON STRUCTURAL DRAWINGS. OPENINGS INDICATEL 150 PLF (PERPENDICULAR TO DECK SPAN) CONCENTRATED LOAD (BARE STEEL DECK) BRACKET RC13. PLYWOOD SHALL HAVE STAGGERED JOINTS AND NAILS SHALL BE THREADED. ALL PLYWOOD SHEATHING SHALL BE CONCENTRATED LOAD (SLABS ON COMPOSITE DECK) OR ANY ADDITIONAL OPENINGS OR INSERTS REQUIRED, SHALL BE VERIFIED WITH RESPECTIVE TRADES PRIOR TO PLACING 1500 LBS. MAX WHEEL LOAD BSMT BASEMENT INSTALLED WITH THE FACE GRAIN PERPENDICULAR TO THE SUPPORTS. **NOT APPLICABLE** BSW **BOTTOM SHORT WAY** NATIONAL CONCRETE MASONRY WIND LOADS RC14. NAILING SHALL BE IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE, TABLE 2304.10.1, UNLESS OTHERWISE CENTER OF GRAVITY ASSOCIATION NOTE: PER ASCE 7-10 AND 2015 IBC, THE DESIGN WIND PRESSURES LISTED BELOW REFLECT ULTIMATE STRENGTH VALUES. **UNIT MASONRY (SECTION 042000)** NEAR FACE CAST-IN-PLACE PER IBC SECTION 1605.4, ASD LOAD COMBINATIONS NOW INCLUDE A 0.6 FACTOR FOR WIND LOADING NOT IN CONTRACT **CONSTRUCTION JOINT** RC15. FLOOR JOISTS SHALL BE BRIDGED AT 8'-0" o.c. MAX. NO OR# CONCRETE MASONRY CONSTRUCTION WORK SHALL CONFORM TO LATEST EDITION OF "BUILDING CODE REQUIREMENTS AND NUMBER CONTROL JOINT WIND LOAD PARAMETERS (ASCE 7-10 - CHAPTER 26) NTS NOT TO SCALE COMMENTARY FOR MASONRY STRUCTURES (ACI 530/530R)" AND "SPECIFICATIONS FOR MASONRY STRUCTURES AND RELATED CL OR € CENTER LINE RISK CATEGORY (IBC 2015 - TABLE 1604.5) CATEGORY II RC16. ALL WALL STUDS SHALL BE BLOCKED AT 4'-0" o.c. MAX. AND AT ALL PLYWOOD EDGES. NWT COMMENTARIES (ACI 530.1/530.1R)". CONCRETE MASONRY WALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH LOW LIFT NORMAL WEIGHT $V_{ult} = 134 MPH$ ULTIMATE DESIGN WIND SPEED (MA TABLE 1604.11 CLEAR, CLEARANCE $V_{asd} = 104 MPH$ NOMINAL DESIGN WIND SPEED (IBC 2015 - EQUATION 16-33) RC17. PROVIDE SOLID BLOCKING BETWEEN JOISTS AND RAFTERS OVER ALL STUD BEARING WALLS OR SUPPORTING BEAMS. ON CENTER CONCRETE MASONRY UNIT WIND DIRECTIONALITY FACTOR (ASCE 7-10 - TABLE 26.6-1 = 0.85 **OUTSIDE DIAMETER** CONCRETE MASONRY STRENGTH (f'm) SHALL NOT BE LESS THAN 1500 PSI WITH SPECIAL INSPECTION. CLEANOU WIND EXPOSURE CATEGORY (ASCE 7-10 - SECTION 26.7) EXPOSURE (OUTSIDE FACE RC18. ALL POSTS SHALL BE (VERTICALLY) BLOCKED THROUGH FLOOR CONSTRUCTION AT ALL LEVELS, TO THE TOP OF COL COLUMN TOPOGRAPHIC FACTOR (ASCE 7-10 - FIGURE 26.8-1) $K_{zt} = 1.0$ FOUNDATION WALL OR SUPPORTING BEAM. OPPOSITE HAND CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C90, NORMAL WEIGHT, TYPE 1 AND TO NCMA "REQUIREMENTS FOR COMPRESSION ENCLOSURE CLASSIFICATION (ASCE 7-10 - SECTION 26.10) ENCLOSED OPNG OPENING LOAD BEARING CONCRETE MASONRY". COMPRESSIVE STRENGTH SHALL BE AS REQUIRED FOR SPECIFIED CONCRETE MASONRY CONCRETE INTERNAL PRESSURE COEFFICIENT (ASCE 7-10 - TABLE 26.11-1 $GC_{pi} = \pm 0.18$ RC19. PROVIDE MINIMUM HEADERS AS REQUIRED BY TABLES 2308.4.1.1(1) AND 2308.4.1.1(2) OF THE MASSACHUSETTS STATE BUILDING OPPOSITE STRENGTH (fm), BUT NOT LESS THAN 2000 PSI FOR THE AVERAGE OF 3 UNITS OR 1700 PSI FOR AN INDIVIDUAL UNIT, BASED ON CONNECTION VELOCITY PRESSURE COEFFICIENT (ASCE 7-10 - TABLE 28.3-1) $K_h = 1.03$ **OCCUPATIONAL SAFETY &** CODE, UNLESS OTHERWISE NOTED. THE AVERAGE NET AREA. CONSTR CONSTRUCTION VELOCITY PRESSURE (ASCE 7-10 - EQUATION 28.3-1) = 40.11 PSF **HEALTH ADMINISTRATION** CONTINUE, CONTINUOUS RC20. PROVIDE MINIMUM BUILT-UP WALL STUDS AT JAMBS OF ALL WINDOW AND DOOR OPENINGS AS NOTED BELOW, UNLESS M4. MORTAR FOR REINFORCED CMU SHALL CONFORM TO ASTM C270, TYPE M OR S, AND HAVE A 28-DAY COMPRESSIVE STRENGTH CONTR CONTRACTOR COMPONENTS AND CLADDING (ASCE 7-10 - CHAPTER 30, PART 1: LOW-RISE BUILDINGS) MORE STRINGENT REQUIREMENTS ARE NOTED ON THE DRAWINGS. EQUAL TO THE SPECIFIED CONCRETE MASONRY STRENGTH (f'm), BUT NOT LESS THAN 1800 PSI. CRSI CONCRETE REINFORCING INSTITUTE EFFECTIVE WIND AREA (ASCE 7-10 - FIGURE 30.4-1) Aeff ≤ 10 SQ. FT PILE CAP OPENING SIZE JACK STUDS KING STUDS WIDTH OF SALIENT ZONE = 9 *FEET* PRECAST CONCRETE INSTITUTE DEAD LOAD UP TO 4'-0" GROUT SHALL CONFORM TO ASTM C476, FINE TYPE, AND HAVE A 28-DAY COMPRESSIVE STRENGTH EQUAL TO THE SPECIFIED NET DESIGN WIND PRESSURES (ASCE 7-10 - EQUATION 30.4-1) PENPENETRATION DEMOLISH, DEMOLITION 4'-0" TO 6'-0" CONCRETE MASONRY STRENGTH (f'm), BUT NOT LESS THAN 3000 PSI. WALL ELEMENTS PERIMETER DEPR DEPRESS, DEPRESSION 6-0" TO 8-0' AT A NON-SALIENT AREA = +43 PSF / -47 PSF PLATE DET 8'-0" TO 10'-0" GROUTING SHALL BE LIMITED TO A MAXIMUM WALL HEIGHT OF 5'-4" PER LIFT. AT A SALIENT CORNER = +43 PSF / -58 PSF POST TENSIONED DEV LNGTH DEVELOPMENT LENGTH ROOF ELEMENTS POUNDS PER SQUARE INCH ALL TRIMMERS AND JACK STUDS SHALL BE OF THE SAME MATERIAL AS THE TYPICAL WALL STUDS, UNLESS $p_{net} = +19 PSF / -47 PSF$ HORIZONTAL JOINT REINFORCEMENT SHALL CONFORM TO ASTM A82, LADDER TYPE, #9 WIRE. PROVIDE PREFABRICATED AT A NON-SALIENT AREA PSF POUNDS PER SQUARE FOOT OTHERWISE NOTED ON THE DRAWINGS. ALL TRIMMERS SHALL BE FULL HEIGHT, EXTENDING FROM THE SOLE PLATE DIA OR Ø DIAMETER CORNERS AND TEES. = +19 PSF / -79 PSF AT AN EDGE PRESSURE TREATED DIAMETERS TO THE CAP PLATE. JACK STUDS SHALL TERMINATE BELOW THE HEADER AND BE THOROUGHLY SPIKED TO THE = +19 PSF / -120 PSF AT A SALIENT CORNER POLYVINYL CHLORIDE RIMMERS. FRAMING SHOWN IN THE TABLE ABOVE IS FOR ONE OPENING - PROVIDE 2X THE NUMBER OF JACK STUDS DIM DIMENSIOI M8. MINIMUM HORIZONTAL JOINT REINFORCEMENT FOR WALLS AND PARTITIONS SHALL BE #9 WIRE SPACED VERTICALLY AT 16"o.c. OVERHANG ELEMENTS AND TRIMMERS FOR MULTIPLE OPENINGS IMMEDIATELY ADJACENT TO EACH OTHER. IN CASES WHERE THE DIRECTION AT A MINIMUM, PROVIDE A BOND BEAM, WITH 2-#5 HORIZONTAL AND CONTINUOUS BARS, AT EACH FLOOR LEVEL AND AT THE TOP = -75 PSF AT AN EDGE DISTANCE BETWEEN OPENINGS DOES NOT ACCOMMODATE THE TOTAL NUMBER OF JACK STUDS AND TRIMMERS, **ROOF DRAIN** AT A SALIENT CORNER = -120 PSF ELIMINATE THE JACK STUD(S) AND CONNECT THE HEADER TO THE TRIMMERS WITH A STANDARD METAL JOIST RIGHT END DRAWING HANGER (CONCEALED FLANGES). REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED BARS. LAP ALL CONTINUOUS BARS A MINIMUM OF REFERENCE L5. SEISMIC LOAD CRITERIA - GENERAL REINFORCING 48 DIAMETERS SEISMIC OCCUPANCY CATEGORY - (ASCE 7-10 - TABLE 1.5-1) RISK CATEGORY II RC21. PROVIDE A MINIMUM OF 3 - 2X CORNER POSTS AT ALL CORNERS AND WALL INTERSECTIONS. REMAINDER MAPPED SPECTRAL ACCELERATION FOR SHORT PERIODS - (MA TABLE 1604.11 **EACH END** = 0.195aRFT M10. ELEVATOR SHAFT WALLS SHALL BE FULLY GROUTED SOLID AND IN ADDITION TO HORIZONTAL REINFORCEMENT RFTURN DESIGN SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS = 0.208 EACH FAC RC22. PROVIDE DOUBLE JOISTS BELOW ALL PARTITIONS PARALLEL TO JOISTS. SHALL BE REINFORCED VERTICALLY WITH A MINIMUM OF #5 BARS AT 32" O.C., UNLESS NOTED. REQD REQUIRED MAPPED SPECTRAL ACCELERATION FOR 1-SECOND PERIOD - (MA TABLE 1604.11) $S_1 = 0.064$ **EACH SIDE** RETG RETAINING DESIGN SPECTRAL RESPONSE ACCELERATION FOR 1-SECOND PERIOD = 0.102**EACH WAY** RC23. PROVIDE METAL HURRICANE ANCHORS AT ALL ROOF RAFTERS AND ROOF TRUSSES TO PLATE CONNECTIONS. M11. THE TOP OF CMU WALLS AND PARTITIONS SHALL BE ANCHORED AS SHOWN IN THE TYPICAL DETAILS AND THE SECTIONS. REV REVISION SITE CLASS - REFER TO GEOTECHNICAL REPORT SITE CLASS D **ELEVATION** SEISMIC DESIGN CATEGORY - (ASCE 7-10 - TABLES 11.6-1 & 11.6-2) CATEGORY B FI FVATOR SECTION MODULUS RC24. FRAME ALL OPENINGS IN FLOOR AND ROOF CONSTRUCTION WITH MINIMUM OF 2 - 2X HEADERS AND TRIMMERS (DEPTH TO M12. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR REQUIRED FIRE RATINGS. SEISMIC IMPORTANCE FACE - (ASCE 7-10 - TABLE 11.5-1) EMBED, EMBEDMENT MATCH THE ADJACENT FRAMING) WITH METAL JOIST/BEAM HANGERS, UNLESS OTHERWISE NOTED. SHEAR CONNECTOR STEEL DECK INSTITUTE SEISMIC LOADS - GYMNASIUM **EQUIPMEN** SECT SECTION RC25. NOTCHING OF JOISTS, BEAMS, STUDS OR PLATES SHALL NOT BE PERMITTED. STRUCTURAL STEEL FRAMING (SECTION 051200) A. BASIC SEISMIC-FORCE-RESISTING SYSTEM - (ASCE 7-10 - TABLE 12.2-1) STRUCTURAL STEEL SYSTEMS NOT EQUIVALEN[.] SQUARE FOOT SPECIFICALLY DETAILED FOR ETCETERA SIMILAR STRUCTURAL STEEL WORK SHALL CONFORM TO THE LASTEST EDITIONS OF AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL SEISMIC RESISTANCE EXIST OR (E **EXISTING** STEEL JOIST INSTITUTE METAL PLATE CONNECTED WOOD TRUSSES (SECTION 061753) BUILDINGS" AND AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", AS MODIFIED BY THE SPECIFICATIONS. RESPONSE MODIFICATION COEFFICIENT - (ASCE 7-10 - TABLE 12.2-1) = 3.0 EXP BLT EXPANSION BOL SPLICE LENGTH DEFLECTION AMPLIFICATION FACTOR - (ASCE 7-10 - TABLE 12.2-1) $_{-}$ = 3.0 EXP JT E XPANSION JOIN SOG SLAB ON GRADE WT1. ALL ROOF TRUSSES SHALL CONFORM TO THE TRUSS PLATE INSTITUTE, "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE WELDING SHALL BE IN ACCORDANCE WITH THE LATEST AWS "D1.1-STRUCTURAL WELDING CODE-STEEL" SYSTEM OVERSTRENGTH FACTOR - (ASCE 7-10 -TABLE 12.2-1) Ω_0 $\neq 3.0$ **EXTERIOR** SPACES CONNECTED WOOD TRUSSES" LATEST EDITION AND SHALL BE DESIGNED FOR THE LOADINGS INDICATED ON THE DRAWINGS. = 0.069 SEISMIC RESPONSE COEFFICENT **SPECS** FLOOR DRAIN **SPECIFICATIONS** TOTAL AND LIVE LOAD DEFLECTIONS SHALL BE LIMITED AS FOLLOWS (UNLESS OTHERWISE INDICATED ON THE DRAWINGS): STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING, UNLESS NOTED: DESIGN BASE SHEAR V = 70 KIPS SQUARE *FAR FACE* PLATES AND OTHER SHAPES (OTHER THAN W & WT) ASTM A36 Fv = 36KSIANALYSIS PROCEDURE USED EQUIVALENT LATERAL FORCE **FULL PENETRATION WELD** STAINLESS STEEL LIVE LOAD DEFLECTION: < L/480 Fy = 50KSI (SQUARE & RECT.) HOLLOW STRUCTURAL SECTIONS ASTM A500 GRADE (STIFF STIFFENER FAR SIDE TOTAL LOAD DEFLECTION: < L/360 ASTM A500 GRADE B Fy = 42KSI (ROUND)SEISMIC LOADS - COMMUNITY CENTER STL FOOT OR FFF STEEL STRUCTURAL PIPE ASTM A53 GRADE B Fv = 35 KSIA. BASIC SEISMIC-FORCE-RESISTING SYSTEM - (ASCE 7-10 - TABLE 12.2-1) LIGHT-FRAME WOOD WALLS SHEATHED SUPP SUPPORT FIELD VERIFY WT2. SUBMIT COMPLETE SHOP DRAWINGS (ERECTION PLANS, MEMBER DETAILS, MEMBER CONNECTIONS, CONNECTIONS BETWEEN $F_V = 50KSI$ W & WT SHAPES *ASTM A992 OR A572* WITH WOOD STRUCTURAL PANELS SYMM SYMMETRICAL MEMBERS AND SUPPORTS, BRACING, ETC.) AND CALCULATIONS FOR ALL TRUSSES AND CONNECTIONS. ALL CALCULATIONS AND RATED FOR SHEAR RESISTANCE FIN FLR FINISHED FLOOR SHOP DRAWINGS SHALL BE PREPARED BY AND BEAR THE STAMP OF A PROFESSIONAL ENGINEER. REGISTERED IN THE CONNECTIONS MAY BE BOLTED OR WELDED, UNLESS SPECIFICALLY NOTED OTHERWISE. CONNECTIONS SHALL BE DESIGNED RESPONSE MODIFICATION COEFFICIENT - (ASCE 7-10 - TABLE 12.2-1) R = 6.5*FIREPROOF* STATE OF MASSACHUSETTS. FINAL DESIGN OF TRUSSES SHALL BE PROVIDED BY THE DESIGNER/FABRICATOR. AND DETAILED IN ACCORDANCE WITH AISC STANDARDS, USING THE ASD METHOD. DEFLECTION AMPLIFICATION FACTOR - (ASCE 7-10 - TABLE 12.2-1) $C_d = 4.0$ TIE BEAM ¥3.0 \~ TIE JOIST SYSTEM OVERSTRENGTH FACTOR - (ASCE 7-10 -TABLE 12.2-1) **FOUNDATION** WT3. IN ADDITION TO THE SLOPING AND UNBALANCED SNOW LOADS AND WIND LOADS LISTED UNDER STRUCTURAL LOADS (THIS SHEET). CONNECTIONS SHALL BE WELDED TO CONFORM TO ASTM A233, E70 SERIES, OR BOLTED TO CONFORM TO ASTM A325, TYPE N TOP AND BOTTOM SEISMIC RESPONSE COEFFICENT = 0.032 **FOOTING** ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING DEAD AND LIVE LOADS APPLIED TO THE TOP CHORD (TC) AND BOTTOM T&G TONGUE AND GROOVE DESIGN BASE SHEAR $2 \vee V = 15 \text{ KIPS}$ GAGE OR GAUGE EQUIVALENT LATERAL FORCE TEMP TEMPERATURE ANALYSIS PROCEDURE USED $LIVE\ LOAD = 46\ PSF\ (TC),\ 20\ PSF\ (BC)$ PROVIDE 3/4" DIAMETER MINIMUM HEADED TYPE ANCHOR RODS AT COLUMNS AND POSTS, UNLESS NOTED OTHERWISE GRADE BEAM TENSION $DEAD\ LOAD = 15\ PSF\ (TC),\ 10\ PSF\ (BC)$ HOT-DIPPED GALVANIZED TEW TOP EACH WAY INCLUDE DETAILED LOAD COMBINATIONS FOR ALL TRUSS CALCULATIONS. FURNISH AND INSTALL ONE WASHER AND ONE HEAVY HEX NUT WITH ALL ANCHOR RODS, UNLESS NOTED OTHERWISE GENERAL CONTRACTOR TFE TETRAFLUORETHYLENE THICK, THICKNESS THK WT4 TRUSS WEB CONFIGURATIONS SHOWN ON THE TRUSS ELEVATIONS ARE SCHEMATIC. FINAL WEB CONFIGURATIONS SHALL BE SIMPLY SUPPORTED BEAM-TO-BEAM CONNECTIONS SHALL BE DOUBLE ANGLE TYPE IN CONFORMANCE WITH THE AISC "MANUAL **THRD** GRADE THREADED DETERMINED BY THE TRUSS DESIGNER / FABRICATOR. COORDINATE WEB MEMBER LOCATIONS WITH M.E.P. AND ACCESS OF STEEL CONSTRUCTION", UNLESS SPECIFICALLY NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS. THROUGH REQUIREMENTS SHOWN ON THE DRAWINGS. FIELD MODIFICATION OF TRUSSES TO FACILITATE THE INSTALLATION OF MECHANICAL H OR HOR TIM HORIZONTAL TIMRER UNITS OR PIPING IS NOT PERMITTED. PROVIDE A 1/4" THICK LEVELING PLATE UNDER EACH COLUMN BASE PLATE FOR USE IN ALIGNING ANCHOR RODS AND BASE TLWTOP LONG WAY HORIZONTAL EACH FACE PLATES. LEVELING PLATE SHALL BE SET AND GROUTED WITH AN APPROVED NON-SHRINK, NON-METALLIC GROUT. GROUT SHALL TOC HORIZONTAL INSIDE FACE TOP OF CONCRETE WT5. MINIMUM CHORD SIZES SHALL BE AS SHOWN ON THE TRUSS ELEVATIONS ON DRAWING S304. MINIMUM WEB MEMBER SIZES SHALL HAVE ATTAINED DESIGN STRENGTH BEFORE ERECTION OF COLUMN. HORIZONTAL OUTSIDE FACE TOS TOP OF STEEL BE 2x4. DESIGN ALL WEB MEMBERS HAVING A LENGTH LESS THAN 8'-0" (FROM WORKING POINT TO WORKING POINT) SO THAT THE TOP OF WALL HIGH POINT WEB MEMBER DOES NOT REQUIRE LATERAL RESTRAINT OR BRACING. S10. PROVIDE A 1/4" THICK MINIMUM CAP PLATE WELDED AT TOP OF HSS COLUMNS, UNLESS NOTED OTHERWISE. HIGH STRENGTH TRANSVERSE TSW HOLLOW STRUCTURAL SECTION TOP SHORT WAY WT6. TRUSS CHORDS AND TRUSS WEB MEMBERS SHALL BE SOUTHERN PINE NO. 2 OR BETTER. ALL LUMBER SHALL BE 19% MAXIMUM S12. SPLICING STRUCTURAL MEMBERS WHERE NOT DETAILED ON DRAWINGS IS PROHIBITED WITHOUT PRIOR APPROVAL OF TYPICAL MOISTURE CONTENT. HEATING, VENTILATION AND AIR CONDITIONING UNIFORM BUILDING CODE WT7. ALL TRUSSES SHALL BE SECURELY BRACED BOTH DURING ERECTION AND AS REQUIRED AFTER PERMANENT INSTALLATION AS UNDERWRITERS LABORATORY S13. STRUCTURAL STEEL EXPOSED TO THE WEATHER IN THE FINISHED PROJECT SHALL BE HOT DIP GALVANIZED TO CONFORM TO REQUIRED ON THE STRUCTURAL DRAWINGS, THE SHOP DRAWINGS, BY THE TRUSS PLATE INSTITUTE SPECIFICATIONS, TPI/WTCA ASTM A123. UNLESS NOTED OTHERWISE INCLUSIVE, INCLUDING PUBLICATION BCSI 1-03, OR AS DIRECTED IN THE FIELD BY THE ARCHITECT/STRUCTURAL ENGINEER. INFORMATION S15. STRUCTURAL STEEL EXPOSED TO VIEW IN THE COMPLETED PROJECT SHALL BE EXPOSED TO VIEW STRUCTURAL STEEL (E.V.S.S.). V OR VERT VERTICAL INSIDE DIAMETER WT8. WEB TRUSS-TO-TRUSS CONTINUOUS LATERAL BRACING (CLB) SHALL BE DETERMINED BY THE TRUSS DESIGNER. SEE TRUSS SHOPS VERTICAL EACH FACE REFER TO THE SPECIFICATIONS FOR SPECIFIC REQUIREMENTS. INSIDE FACE TO DETERMINE FINAL WEB LAYOUT (SEE NOTE #4) AND PROVIDE CLB AS REQUIRED AND SHOWN ON THE TRUSS SHOPS. VERTICAL INSIDE FACE INSULATION VOF VERTICAL OUTSIDE FACE S16. REFER TO THE SPECIFICATION FOR PAINTING AND SURFACE PREPARATION REQUIREMENTS. INTERIOR WT9. CONTINUOUS LATERAL BRACING LINES (CLB) SHALL BE RESTRAINED WITH DIAGONAL. ARROWHEAD OR 'X' BRACING AT EACH END OF INVERT CLB LINE AND AT 20'-0" O.C. MAX, PER BCSI 2013 AND THE TYPICAL DETAILS ON THE CONTRACT DRAWINGS. SEE TYPICAL DETAIL ON S17. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE W/O WITHOUT THIS DRAWING. ALL WEB MEMBERS REQUIRING CLB AS SHOWN ON THE TRUSS SHOPS SHALL RECIEVE ARROWHEAD OR 'X' NEW STRUCTURE FOR WIND AND CONSTRUCTION LOADS. TEMPORARY SUPPORTS SHALL REMAIN IN PLACE UNTIL ALL ELEMENTS JOIST WOOD REQUIRED FOR STABILITY OF THE STEEL FRAME ARE COMPLETED. **WORKING POINT** WATERPROOFING **WPG** WT10. CONTINUOUS LATERAL BRACING REQUIREMENTS: KIP (1000 POUNDS WS WATER STOP a) TOP CHORD: SEE BOTH STRUCTURAL DRAWINGS AND SHOP DRAWINGS FOR UNSHEATHED PORTIONS OF TOP CHORD. USE THE KIPS PER SQUARE INCH **SHEAR CONNECTORS (SECTION 051226)** WEIGHT MORE STRINGENT REQUIREMENT. KNOCKOUT

SC1. SHEAR CONNECTORS SHALL BE HEADED STUDS CONFORMING TO ASTM A108, GRADES 1010, 1015, OR 1020.

FOOT OF BEAM LENGTH ON ALL BEAMS SUPPORTING CONCRETE SLABS OVER COMPOSITE DECK.

SC2. SHEAR CONNECTORS SHALL BE 3/4" DIAMETER WITH A LENGTH EQUAL TO TOTAL SLAB THICKNESS MINUS 1 1/2 INCHES UNLESS

NOTED OTHERWISE THE NUMBER OF SHEAR CONNECTORS REQUIRED PER BEAM IS INDICATED THUS (X) ON THE DRAWINGS.

INSTALL CONNECTORS UNIFORMLY ALONG THE LENGTH OF THE BEAM UNLESS NOTED. INSTALL AT LEAST ONE CONNECTOR PER

b) BOTTOM CHORD: PROVIDE CONTINUOUS LATERAL BRACING OF THE BOTTOM CHORDS OF ALL TRUSSES WITH 2x4 (MIN.) AT

6'-0" o.c. MAX. SPACING. IF GYPSUM SHEATHING IS APPLIED DIRECTLY TO THE BOTTOM CHORD, BOTTOM CHORD CONTINUOUS

BEARING POINTS). TRUSS DESIGNER SHALL SELECT AND SPECIFY ON THE TRUSS ERECTION DRAWINGS A HOLDDOWN THAT

WT11. PROVIDE SIMPSON "H" SERIES HOLDDOWN CONNECTORS (OR EQUAL) AT ALL ROOF TRUSS BEARINGS (INTERIOR AND EXTERIOR

c) WEB MEMBERS: SEE THE SHOP DRAWINGS TO IDENTIFY WEB MEMBERS THAT REQUIRE BRACING.

EXCEEDS THE UPLIFT FORCE PROVIDED IN THE TRUSS SHOP DRAWINGS.

LATERAL BRACING MAY BE OMITTED.

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2 Addendum #2

DRAWING TITLE

General Notes and

Abbreviations

DRAWING INFORMATION

DATE OF ISSUE

WELDED WIRE FABRIC

WESTERN WOOD PRODUCTS ASSOCIATION

WWF

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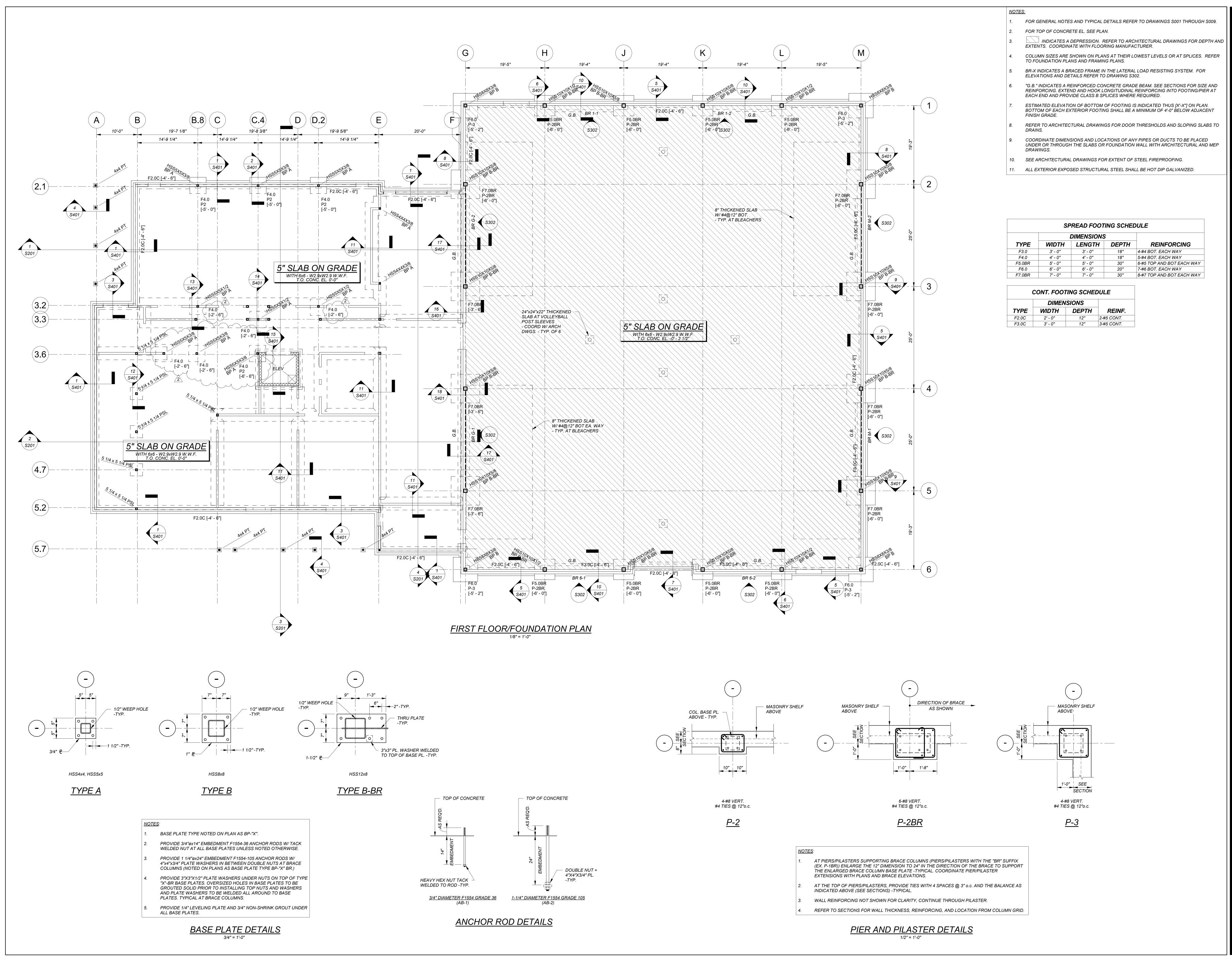
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2 Addendum #2 08/03/2022

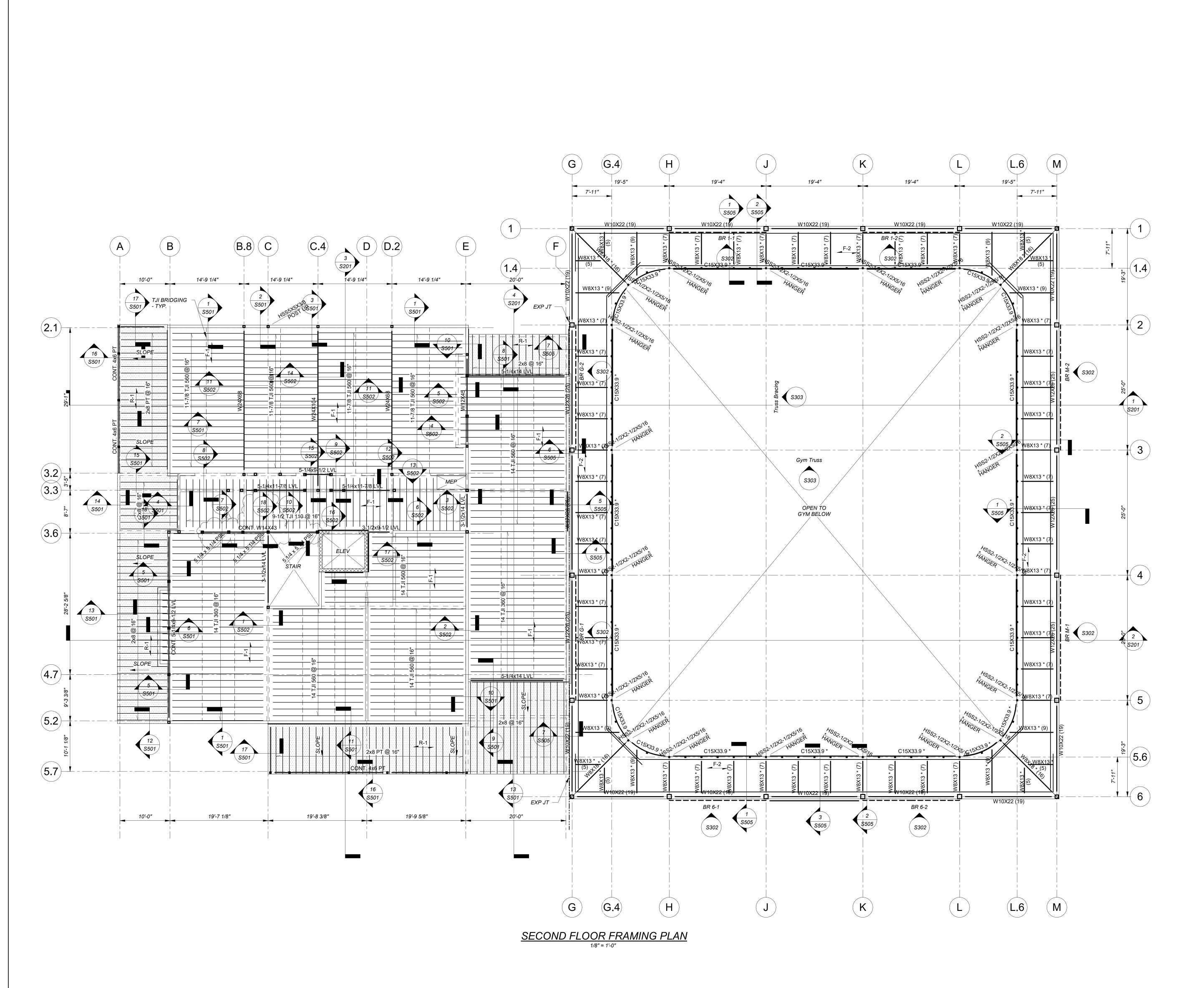
DRAWING TITLE First Floor/Foundation

DRAWING INFORMATION

Construction Documents

DRAWING NUMBER

PROJECT #



NOTES:

FOR GENERAL NOTES AND TYPICAL DETAILS REFER TO DRAWINGS S001 THROUGH S009.
 F-1 INDICATES SPAN DIRECTION OF FLOOR CONSTRUCTION CONSISTING OF 3/4" PLYWOOD FLOOR SHEATHING GLUED AND NAILED TO FRAMING MEMBERS. PROVIDE SOUND MAT AND GYPSUM

TOPPING PER THE ARCHITECTURAL DRAWINGS.

1. F-2 INDICATES SPAN DIRECTION OF FLOOR CONSTRUCTION CONSISTING OF 3" MINIMUM NORMAL WEIGHT CONCRETE TOPPING WITH 6x6-W2.9xW2.9 W.W.F. SUPPORTED WITH SLAB

BOLSTERS, AND 2" DEEP x 18 GAUGE GALVANIZED COMPOSITE METAL FLOOR DECK. MINIMUM TOTAL THICKNESS = 5" TOTAL SLAB THICKNESS.

(XX) INDICATES THE NUMBER OF EQUALLY SPACED 3/4" DIAMETER HEADED SHEAR CONNECTOR STUDS WELDED TO THE CENTER OF THE TOP FLANGE OF THE BEAM OR GIRDER. PROVIDE A MINIMUM OF ONE STUD PER FOOT LENGTH AT ALL BEAMS SUPPORTING COMPOSITE FLOOR DECK.

MINIMUM OF ONE STUD PER FOOT LENGTH AT ALL BEAMS SUPPORTING COMPOSITE FLOOR DECI 5. SECOND FLOOR TOP OF STEEL ELEVATION = 13'-7". UNLESS NOTED OTHERWISE ON PLAN OR IN

6. COLUMN SIZES ARE SHOWN ON PLANS AT THEIR LOWEST LEVELS OR AT SPLICES. REFER TO FOUNDATION PLANS AND FRAMING PLANS.

7. WOOD HEADERS AND BEAMS SHALL BE SUPPORTED BY BUILT-UP JAMBS (JACK AND KING STUDS) AT THE BOTH ENDS, PER THE GENERAL NOTES, UNLESS NOTED OTHERWISE.

BR-X INDICATES A BRACED FRAME IN THE LATERAL LOAD RESISTING SYSTEM. FOR ELEVATIONS AND

9. "D.B." INDICATES 2-#5 DRAG BARS IN CONCRETE SLAB. PLACE ONE BAR ON EACH SIDE OF THE BRACED FRAME BEAM AND BOTH BARS ON SAME SIDE WHERE THE FRAME IS NEAR THE EDGE OF SLAB. EXTEND BARS ONE BAY (MIN.) EACH SIDE OF BRACE BAY. PROVIDE CLASS B SPLICES WHERE

10. *INDICATES EXPOSED TO VIEW STRUCTURAL STEEL. REFER TO SPECIFICATIONS FOR SURFACE PREPARATION AND FINISH REQUIREMENTS. COORDINATE LOCATIONS OF E.V.S.S. WITH ARCHITECTURAL DRAWINGS.

11. R-1 INDICATES SPAN DIRECTION OF ROOF CONSTRUCTION CONSISTING OF 5/8" DEEP 40/20 APA RATED SHEATHING WITH EXTERIOR GRADE AND STRUCTURAL 1 RATING. PROVIDE "H" CLIPS PER TYPICAL DETAILS.

. INDICATES A ROOF AREA.

13. COORDINATE AND VERIFY THE SIZE, QUANTITY AND LOCATION OF ALL VENTS, PIPES, DUCTS SHAFTS, AND OTHER FLOOR PENETRATIONS WITH ARCHITECTURAL AND M.E.P. DRAWINGS AND PROVIDE STEEL FRAMES AS REQUIRED PER THE TYPICAL DETAILS. ALL REQUIRED PENETRATIONS MIGHT NOT BE SHOWN ON THE STRUCTURAL DRAWINGS.

14. ALL EXTERIOR EXPOSED STRUCTURAL STEEL SHALL BE HOT DIP GALVANIZED.

SEE ARCHITECTURAL DRAWINGS FOR EXTENT OF STEEL FIREPROOFING.

thitect

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REVISIONS2 Addendum #2 08/03/2022

DRAWING TITLE

Second Floor Framing Plan

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07/22/22
DATE OF ISSUE

struction Documents

CRIPTION

Indicated Author

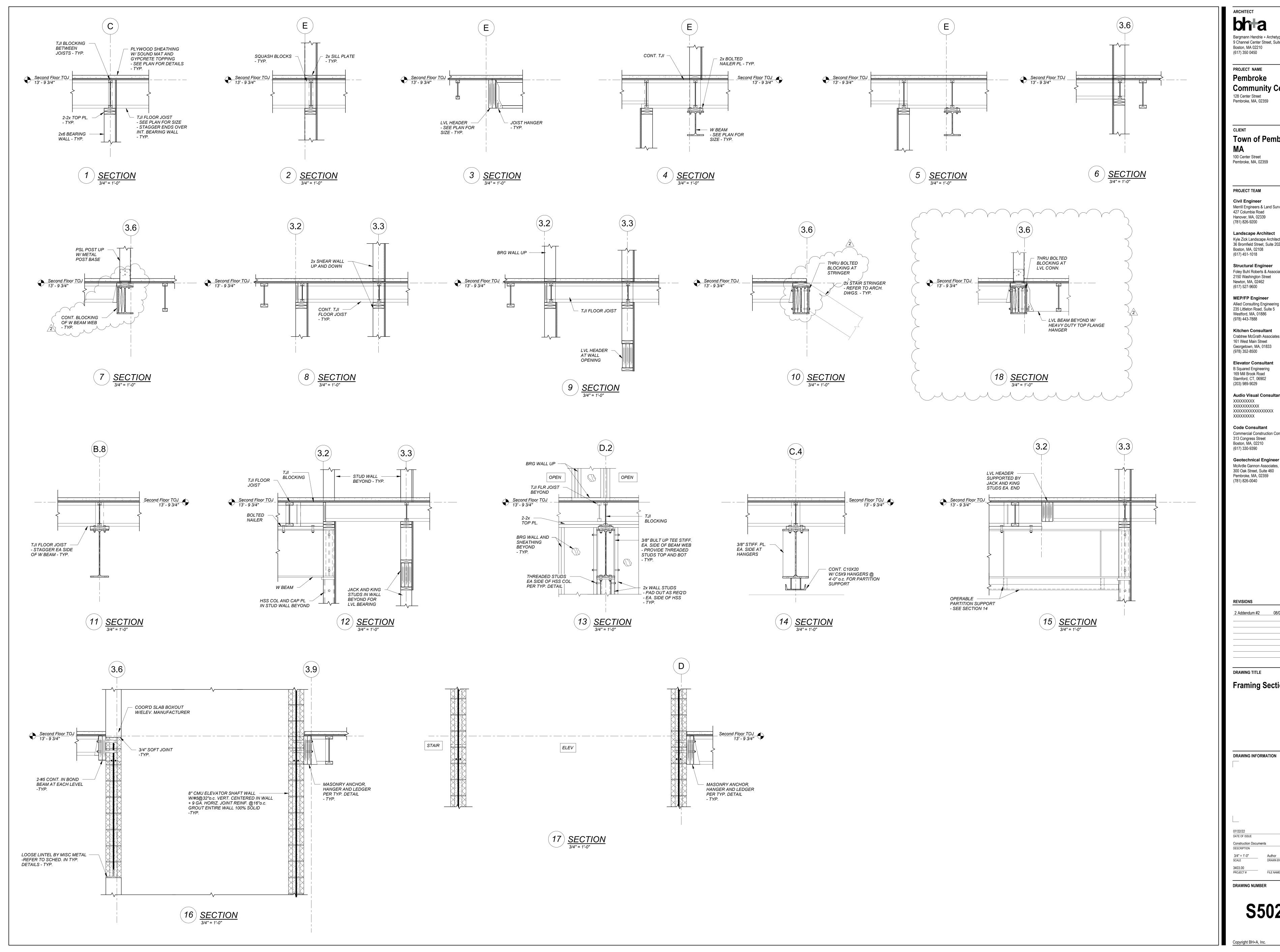
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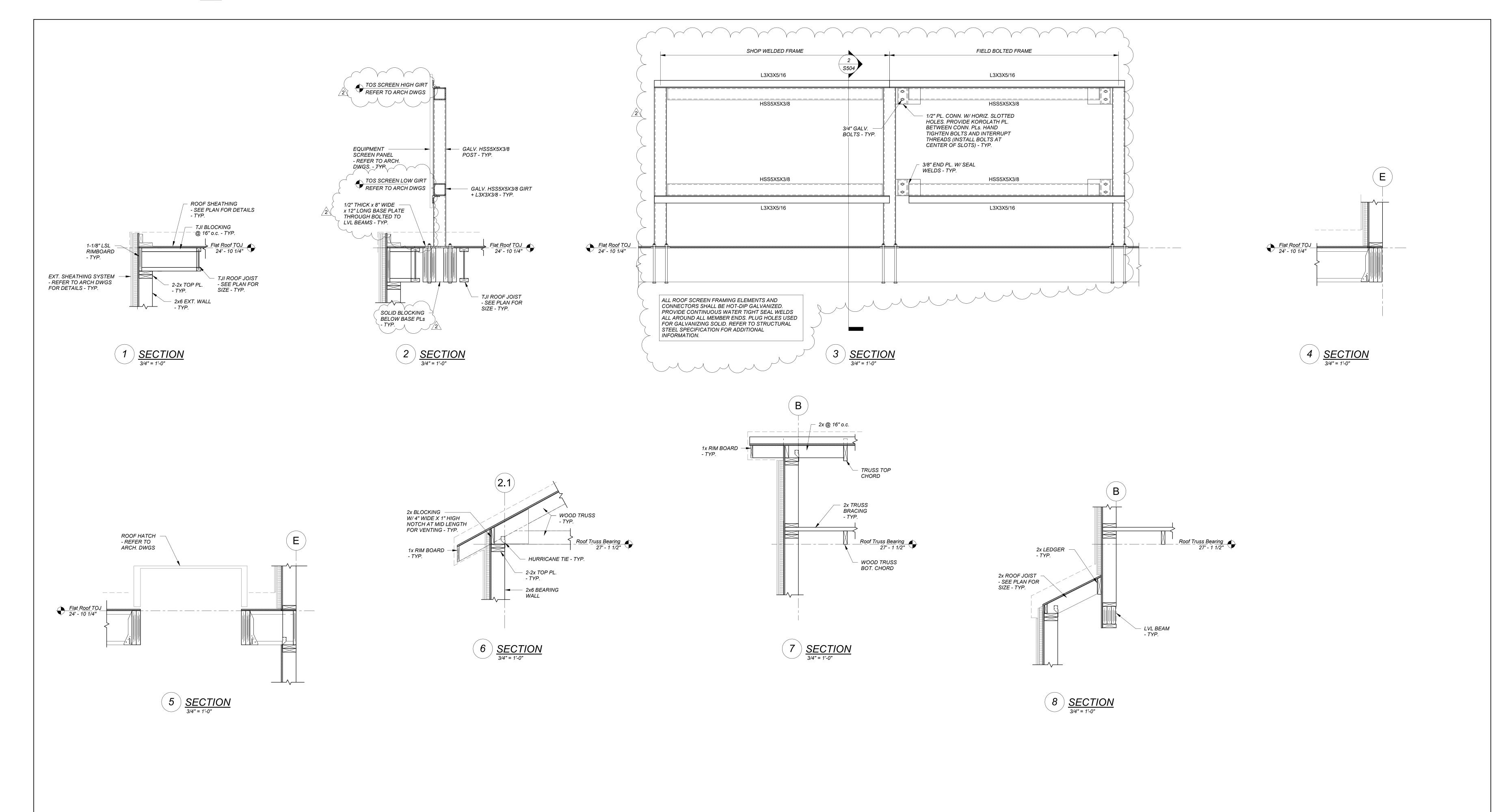
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2 Addendum #2 08/03/2022

Framing Sections II

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S502



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2 Addendum #2 08/03/2022

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Framing Sections
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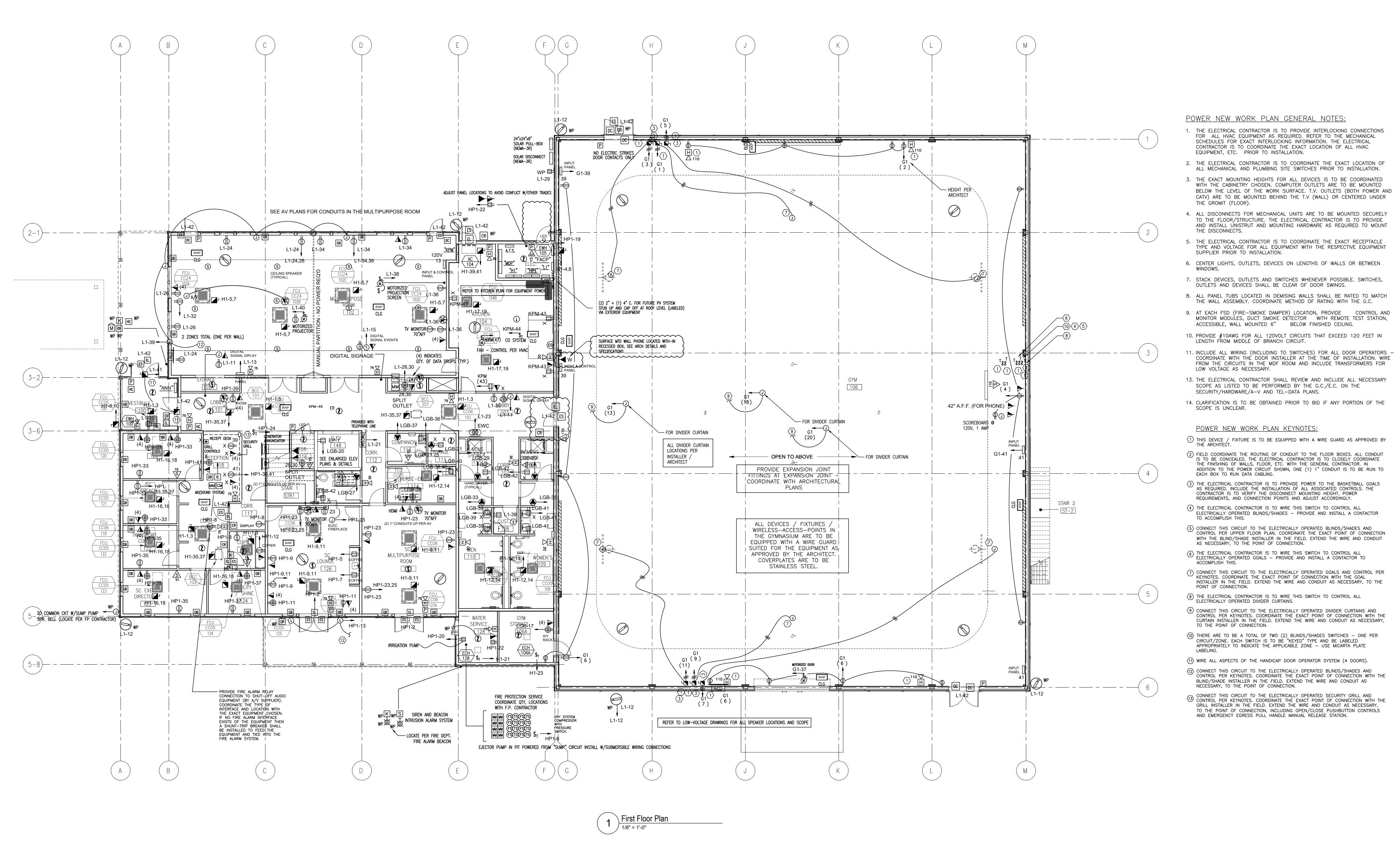
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S50*4*



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Hanover, MA, 02339 TO THE FLOOR/STRUCTURE. THE ELECTRICAL CONTRACTOR IS TO PROVIDE (781) 826-9200 AND INSTALL UNISTRUT AND MOUNTING HARDWARE AS REQUIRED TO MOUNT Landscape Architect

Kyle Zick Landscape Architecture, Inc. TYPE AND VOLTAGE FOR ALL EQUIPMENT WITH THE RESPECTIVE EQUIPMENT 36 Bromfield Street, Suite 202 Boston, MA, 02108 (617) 451-1018

OUTLETS AND DEVICES SHALL BE CLEAR OF DOOR SWINGS. 8. ALL PANEL TUBS LOCATED IN DEMISING WALLS SHALL BE RATED TO MATCH THE WALL ASSEMBLY. COORDINATE METHOD OF RATING WITH THE G.C.

9. AT EACH FSD (FIRE-SMOKE DAMPER) LOCATION, PROVIDE CONTROL AND MONITOR MODULES, DUCT SMOKE DETECTOR WITH REMOTE TEST STATION, ACCESSIBLE, WALL MOUNTED 6" BELOW FINISHED CEILING.

10. PROVIDE #10AWG FOR ALL 120VOLT CIRCUITS THAT EXCEED 120 FEET IN LENGTH FROM MIDDLE OF BRANCH CIRCUIT.

FROM THE CIRCUITS IN THE MDF ROOM AND INCLUDE TRANSFORMERS FOR LOW VOLTAGE AS NECESSARY. 13. THE ELECTRICAL CONTRACTOR SHALL REVIEW AND INCLUDE ALL NECESSARY SCOPE AS LISTED TO BE PERFORMED BY THE G.C./E.C. ON THE

SECURITY/HARDWARE/A-V AND TEL-DATA PLANS. 14. CLARIFICATION IS TO BE OBTAINED PRIOR TO BID IF ANY PORTION OF THE

SCOPE IS UNCLEAR.

POWER NEW WORK PLAN KEYNOTES:

1) THIS DEVICE / FIXTURE IS TO BE EQUIPPED WITH A WIRE GUARD AS APPROVED BY

(2) FIELD COORDINATE THE ROUTING OF CONDUIT TO THE FLOOR BOXES. ALL CONDUIT IS TO BE CONCEALED. THE ELECTRICAL CONTRACTOR IS TO CLOSELY COORDINATE THE FINISHING OF WALLS, FLOOR, ETC. WITH THE GENERAL CONTRACTOR. IN ADDITION TO THE POWER CIRCUIT SHOWN, ONE (1) 1" CONDUIT IS TO BE RUN TO EACH BOX TO RUN DATA CABLING.

(3) THE ELECTRICAL CONTRACTOR IS TO PROVIDE POWER TO THE BASKETBALL GOALS AS REQUIRED. INCLUDE THE INSTALLATION OF ALL ASSOCIATED CONTROLS. THE CONTRACTOR IS TO VERIFY THE DISCONNECT MOUNTING HEIGHT. POWER REQUIREMENTS, AND CONNECTION POINTS AND ADJUST ACCORDINGLY.

(4) THE ELECTRICAL CONTRACTOR IS TO WIRE THIS SWITCH TO CONTROL ALL ELECTRICALLY OPERATED BLINDS/SHADES — PROVIDE AND INSTALL A CONTACTOR TO ACCOMPLISH THIS.

(5) CONNECT THIS CIRCUIT TO THE ELECTRICALLY OPERATED BLINDS/SHADES AND CONTROL PER UPPER FLOOR PLAN. COORDINATE THE EXACT POINT OF CONNECTION WITH THE BLIND/SHADE INSTALLER IN THE FIELD. EXTEND THE WIRE AND CONDUIT AS NECESSARY, TO THE POINT OF CONNECTION.

(6) THE ELECTRICAL CONTRACTOR IS TO WIRE THIS SWITCH TO CONTROL ALL ELECTRICALLY OPERATED GOALS — PROVIDE AND INSTALL A CONTACTOR TO ACCOMPLISH THIS.

(7) CONNECT THIS CIRCUIT TO THE ELECTRICALLY OPERATED GOALS AND CONTROL PER KEYNOTES. COORDINATE THE EXACT POINT OF CONNECTION WITH THE GOAL INSTALLER IN THE FIELD. EXTEND THE WIRE AND CONDUIT AS NECESSARY, TO THE

(8) THE ELECTRICAL CONTRACTOR IS TO WIRE THIS SWITCH TO CONTROL ALL ELECTRICALLY OPERATED DIVIDER CURTAINS.

(9) CONNECT THIS CIRCUIT TO THE ELECTRICALLY OPERATED DIVIDER CURTAINS AND CONTROL PER KEYNOTES. COORDINATE THE EXACT POINT OF CONNECTION WITH THE CURTAIN INSTALLER IN THE FIELD. EXTEND THE WIRE AND CONDUIT AS NECESSARY,

TO THE POINT OF CONNECTION. (10) THERE ARE TO BE A TOTAL OF TWO (2) BLINDS/SHADES SWITCHES - ONE PER CIRCUIT/ZONE. EACH SWITCH IS TO BE "KEYED" TYPE AND BE LABELED

APPROPRIATELY TO INDICATE THE APPLICABLE ZONE - USE MICARTA PLATE (11) WIRE ALL ASPECTS OF THE HANDICAP DOOR OPERATOR SYSTEM (4 DOORS).

CONNECT THIS CIRCUIT TO THE ELECTRICALLY OPERATED BLINDS/SHADES AND CONTROL PER KEYNOTES. COORDINATE THE EXACT POINT OF CONNECTION WITH THE BLIND/SHADE INSTALLER IN THE FIELD. EXTEND THE WIRE AND CONDUIT AS NECESSARY, TO THE POINT OF CONNECTION.

CONNECT THIS CIRCUIT TO THE ELECTRICALLY OPERATED SECURITY GRILL AND CONTROL PER KEYNOTES. COORDINATE THE EXACT POINT OF CONNECTION WITH THE GRILL INSTALLER IN THE FIELD. EXTEND THE WIRE AND CONDUIT AS NECESSARY, TO THE POINT OF CONNECTION, INCLUDING OPEN/CLOSE PUSHBUTTON CONTROLS AND EMERGENCY EGRESS PULL HANDLE MANUAL RELEASE STATION.

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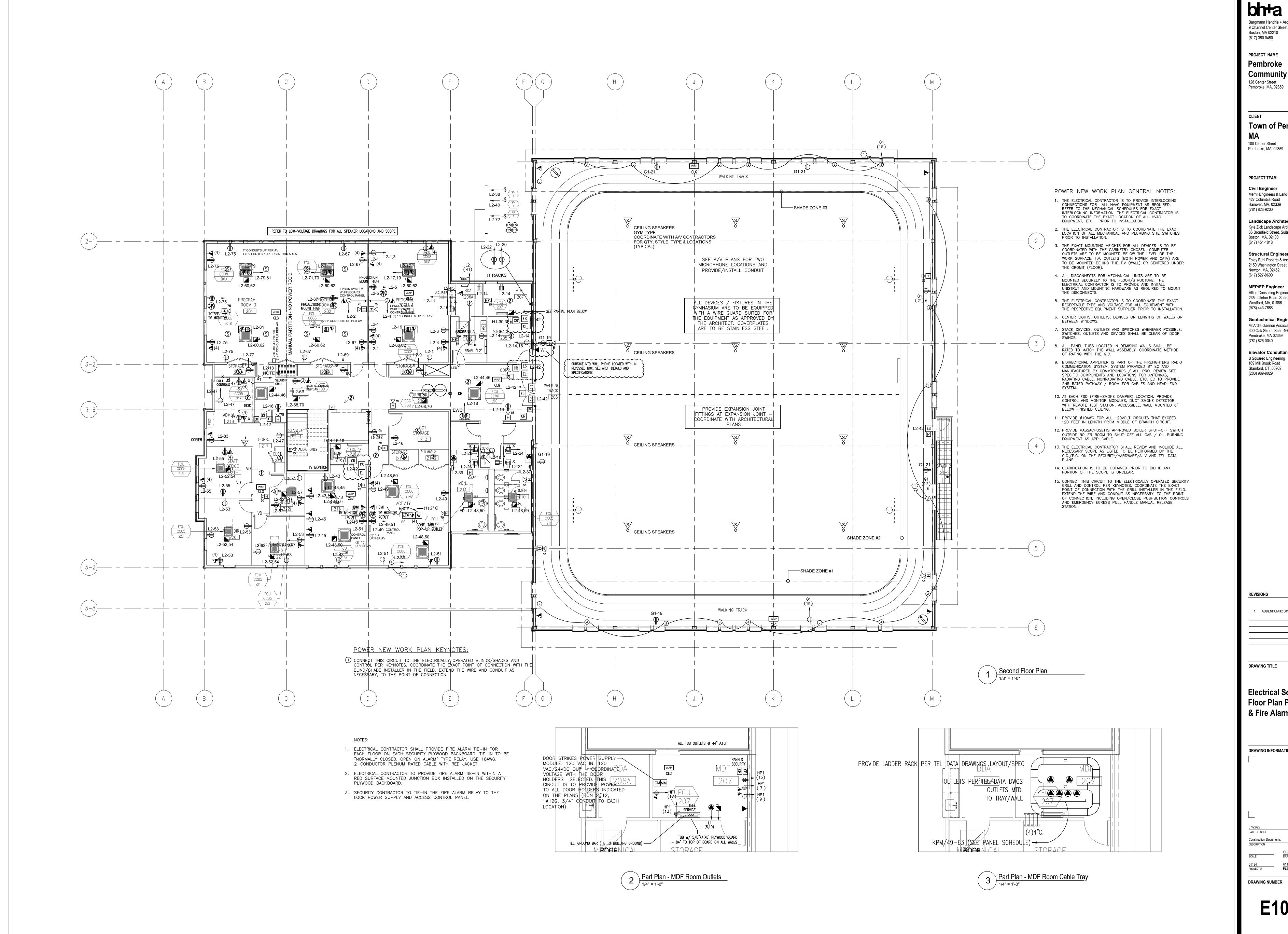
REVISIONS

DRAWING TITLE

Electrical First Floor Plan Power & Fire

DRAWING INFORMATION

E100



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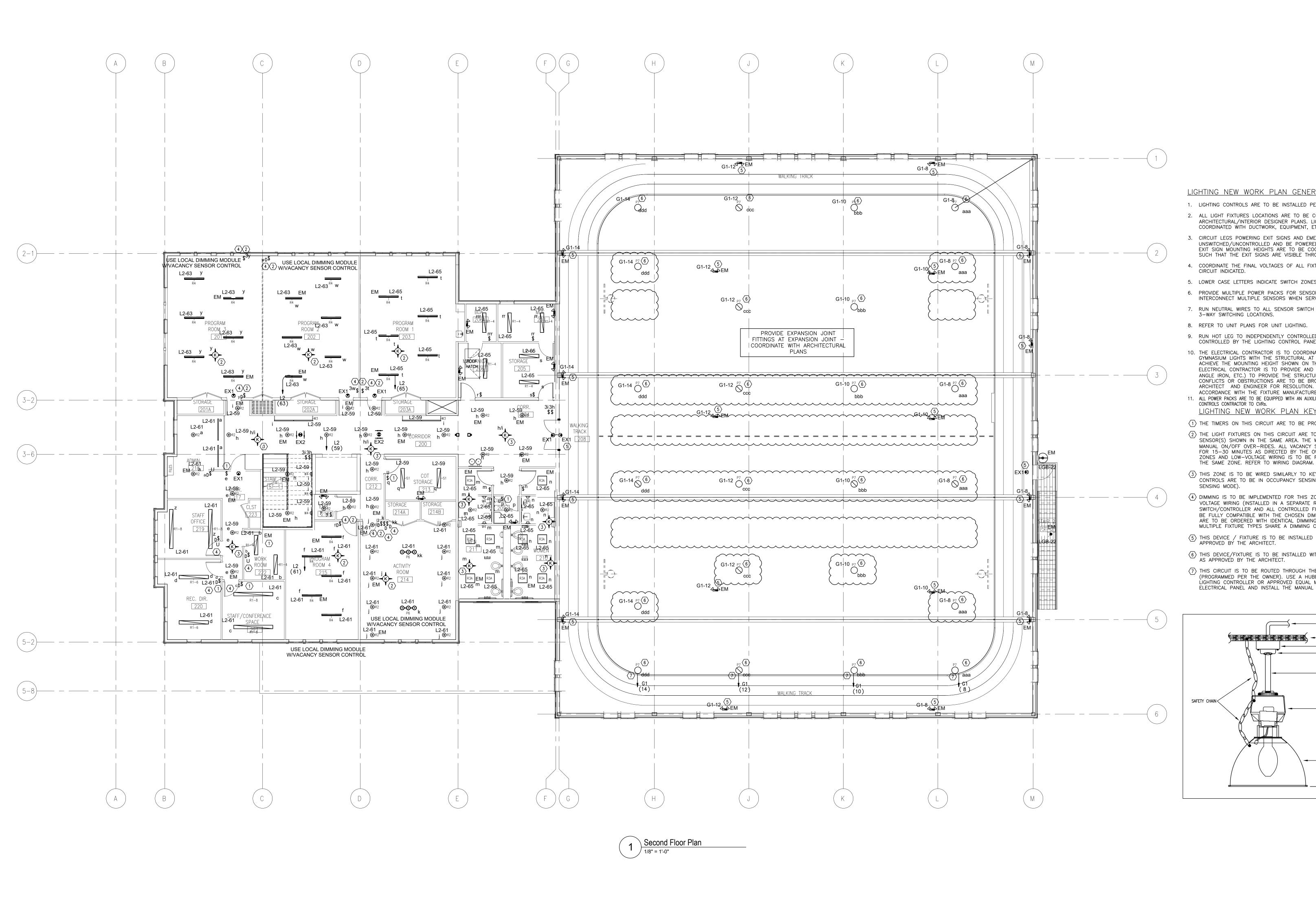
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Electrical Second Floor Plan Power & Fire Alarm

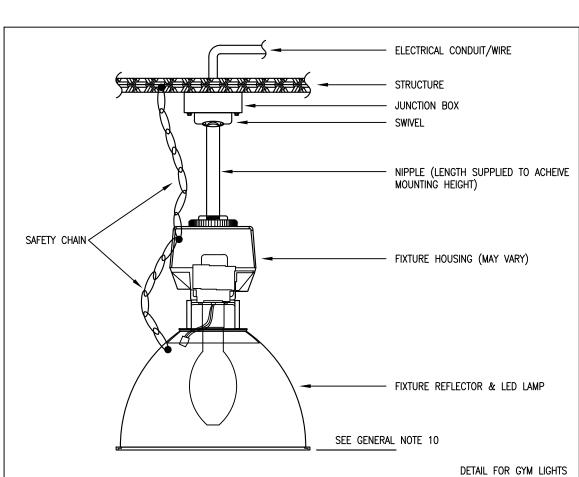
DRAWING INFORMATION

E101



<u>LIGHTING NEW WORK PLAN GENERAL NOTES:</u>

- 1. LIGHTING CONTROLS ARE TO BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
- 2. ALL LIGHT FIXTURES LOCATIONS ARE TO BE COORDINATED WITH ARCHITECTURAL/INTERIOR DESIGNER PLANS. LIGHT FIXTURE LOCATIONS ARE TO BE COORDINATED WITH DUCTWORK, EQUIPMENT, ETC.
- 3. CIRCUIT LEGS POWERING EXIT SIGNS AND EMERGENCY FIXTURES SHALL BE UNSWITCHED/UNCONTROLLED AND BE POWERED FROM THE LOCAL LIGHTING CIRCUIT. EXIT SIGN MOUNTING HEIGHTS ARE TO BE COORDINATED WITH THE CEILING HEIGHT SUCH THAT THE EXIT SIGNS ARE VISIBLE THROUGHOUT THE AREAS SERVED.
- 4. COORDINATE THE FINAL VOLTAGES OF ALL FIXTURES WITH THE VOLTAGE OF THE CIRCUIT INDICATED.
- 5. LOWER CASE LETTERS INDICATE SWITCH ZONES. WIRE CONTROLS ACCORDINGLY.
- 6. PROVIDE MULTIPLE POWER PACKS FOR SENSORS SERVING MULTIPLE ZONES. INTERCONNECT MULTIPLE SENSORS WHEN SERVING THE SAME ZONE.
- 7. RUN NEUTRAL WIRES TO ALL SENSOR SWITCH LOCATIONS. PROVIDE EXTRA LEGS FOR 3-WAY SWITCHING LOCATIONS.
- 8. REFER TO UNIT PLANS FOR UNIT LIGHTING.
- 9. RUN HOT LEG TO INDEPENDENTLY CONTROLLED ZONES FOR FOR AREAS NOT CONTROLLED BY THE LIGHTING CONTROL PANEL.
- 10. THE ELECTRICAL CONTRACTOR IS TO COORDINATE THE INSTALLATION OF THE GYMNASIUM LIGHTS WITH THE STRUCTURAL AT THE FIXTURE LOCATIONS SHOWN TO ACHIEVE THE MOUNTING HEIGHT SHOWN ON THE ARCHITECTURAL ELEVATIONS. THE ELECTRICAL CONTRACTOR IS TO PROVIDE AND INSTALL ANY HARDWARE (I.E. UNISTRUT, ANGLE IRON, ETC.) TO PROVIDE THE STRUCTURAL SUPPORT FOR THE FIXTURE. ANY ARCHITECT AND ENGINEER FOR RESOLUTION. ALL WORK IS TO BE PERFORMED IN ACCORDANCE WITH THE FIXTURE MANUFACTURER'S SPECIFICATIONS.
- 11. ALL POWER PACKS ARE TO BE EQUIPPED WITH AN AUXILIARY RELAY FOR CONNECTION BY THE HVAC / CONTROLS CONTRACTOR TO CVRs. LIGHTING NEW WORK PLAN KEYNOTES:
- (1) THE TIMERS ON THIS CIRCUIT ARE TO BE PROGRAMMED FOR 30 MINUTES. (2) THE LIGHT FIXTURES ON THIS CIRCUIT ARE TO BE CONTROLLED BY THE VACANCY SENSOR(S) SHOWN IN THE SAME AREA. THE WALL SWITCHES ARE TO SERVE AS MANUAL ON/OFF OVER-RIDES. ALL VACANCY SENSORS ARE TO BE PROGRAMMED FOR 15-30 MINUTES AS DIRECTED BY THE OWNER. LOWER CASE LETTERS INDICATE ZONES AND LOW-VOLTAGE WIRING IS TO BE RUN BETWEEN SENSORS THAT SERVE
- (3) THIS ZONE IS TO BE WIRED SIMILARLY TO KEYNOTE #2 EXCEPT THAT THE CONTROLS ARE TO BE IN OCCUPANCY SENSING MODE (IN LIEU OF VACANCY SENSING MODE).
- 4 DIMMING IS TO BE IMPLEMENTED FOR THIS ZONE INCLUDE ALL LOW & LINE VOLTAGE WIRING (INSTALLED IN A SEPARATE RACEWAY) AS REQUIRED BETWEEN THE SWITCH/CONTROLLER AND ALL CONTROLLED FIXTURES. THE DIMMER SWITCH IS TO BE FULLY COMPATIBLE WITH THE CHOSEN DIMMING BALLAST/FIXTURE. THE FIXTURES ARE TO BE ORDERED WITH IDENTICAL DIMMING SYSTEM CHARACTERISTICS WHERE MULTIPLE FIXTURE TYPES SHARE A DIMMING CONTROL SWITCH.
- 5 THIS DEVICE / FIXTURE IS TO BE INSTALLED WITH A WIRE GUARD COVER AS APPROVED BY THE ARCHITECT.
- (6) THIS DEVICE/FIXTURE IS TO BE INSTALLED WITH A WIRE GUARD AND GLASS LENS AS APPROVED BY THE ARCHITECT.
- $\langle 7 \rangle$ THIS CIRCUIT IS TO BE ROUTED THROUGH THE GYM LIGHTING CONTROLLER (PROGRAMMED PER THE OWNER). USE A HUBBELL #CX SERIES 8-RELAY DIGITAL LIGHTING CONTROLLER OR APPROVED EQUAL MOUNTED ADJACENT TO THE ELECTRICAL PANEL AND INSTALL THE MANUAL SWITCHES AS SHOWN.



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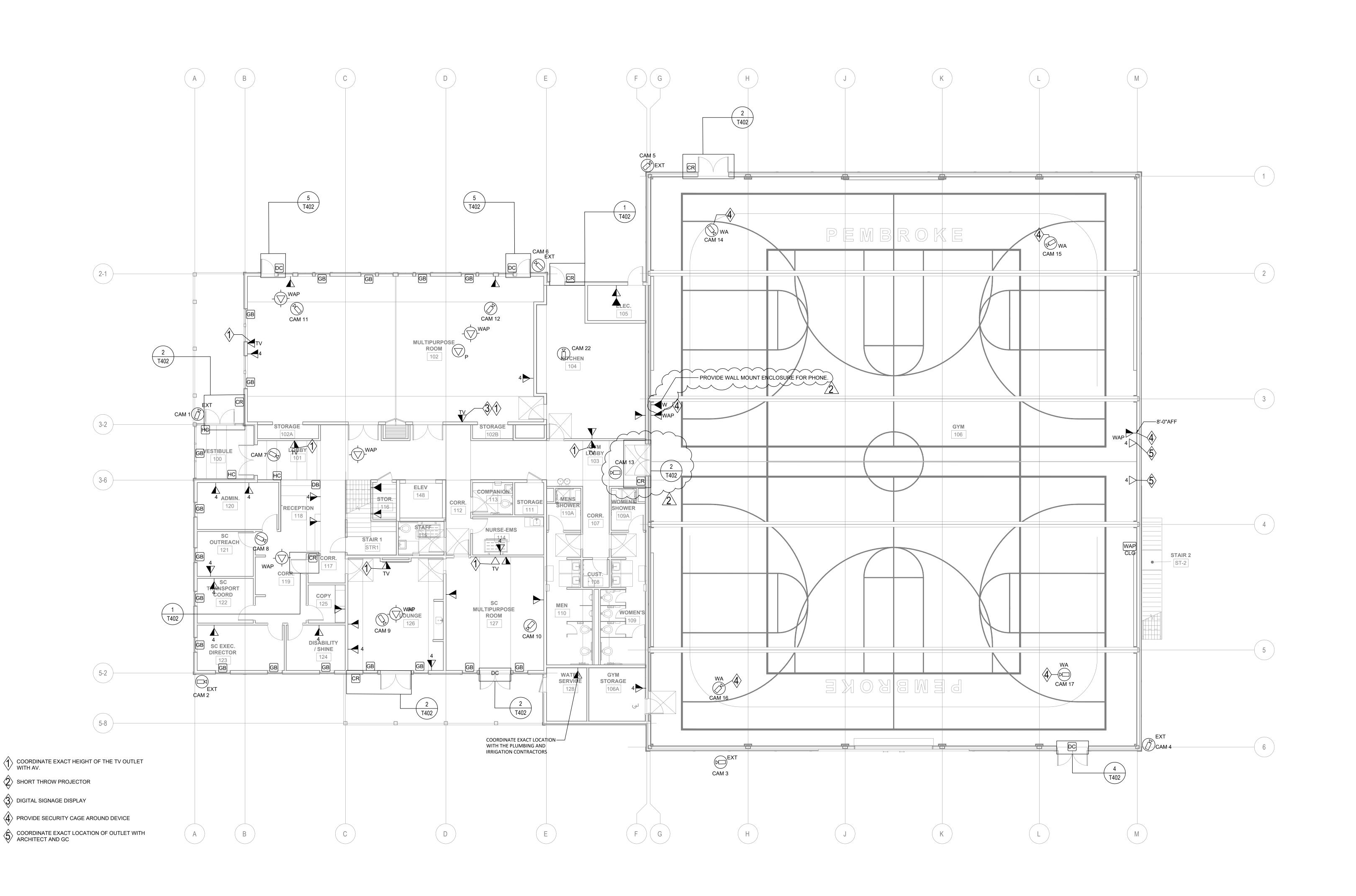
REVISIONS

1. ADDENDUM #2 08/03/2022

DRAWING TITLE

Second Floor Plan

DRAWING INFORMATION



architect bhta

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REVISIONS

2\ ADDENDUM 2

Technology First Floor Plan

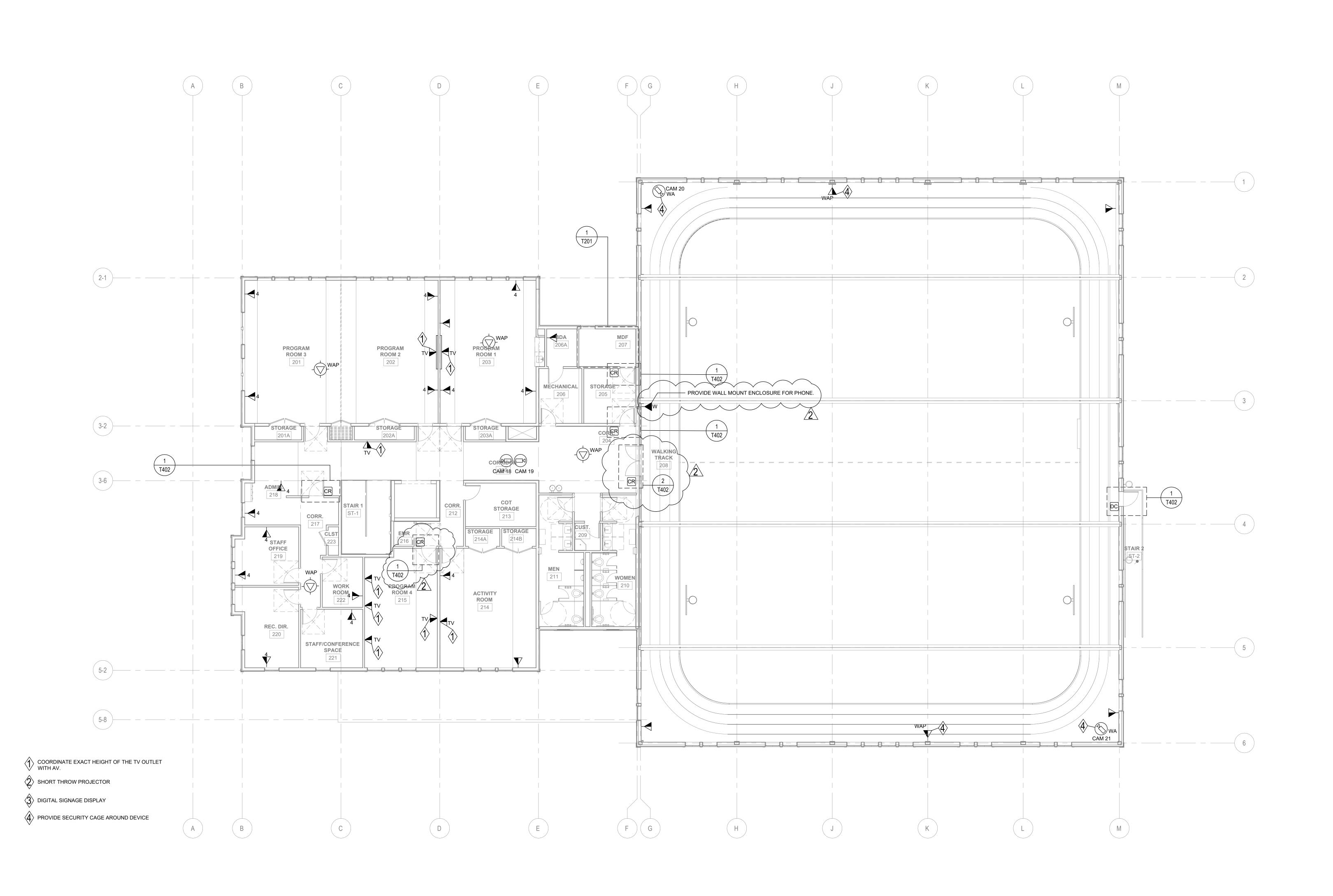
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DRAWING INFORMATION

7/22/22
DATE OF ISSUE
Site Plan Review
DESCRIPTION

DRAWING NUMBER

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architect bha

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EVISIONS

2\ ADDENDUM 2

DRAWING TITLE

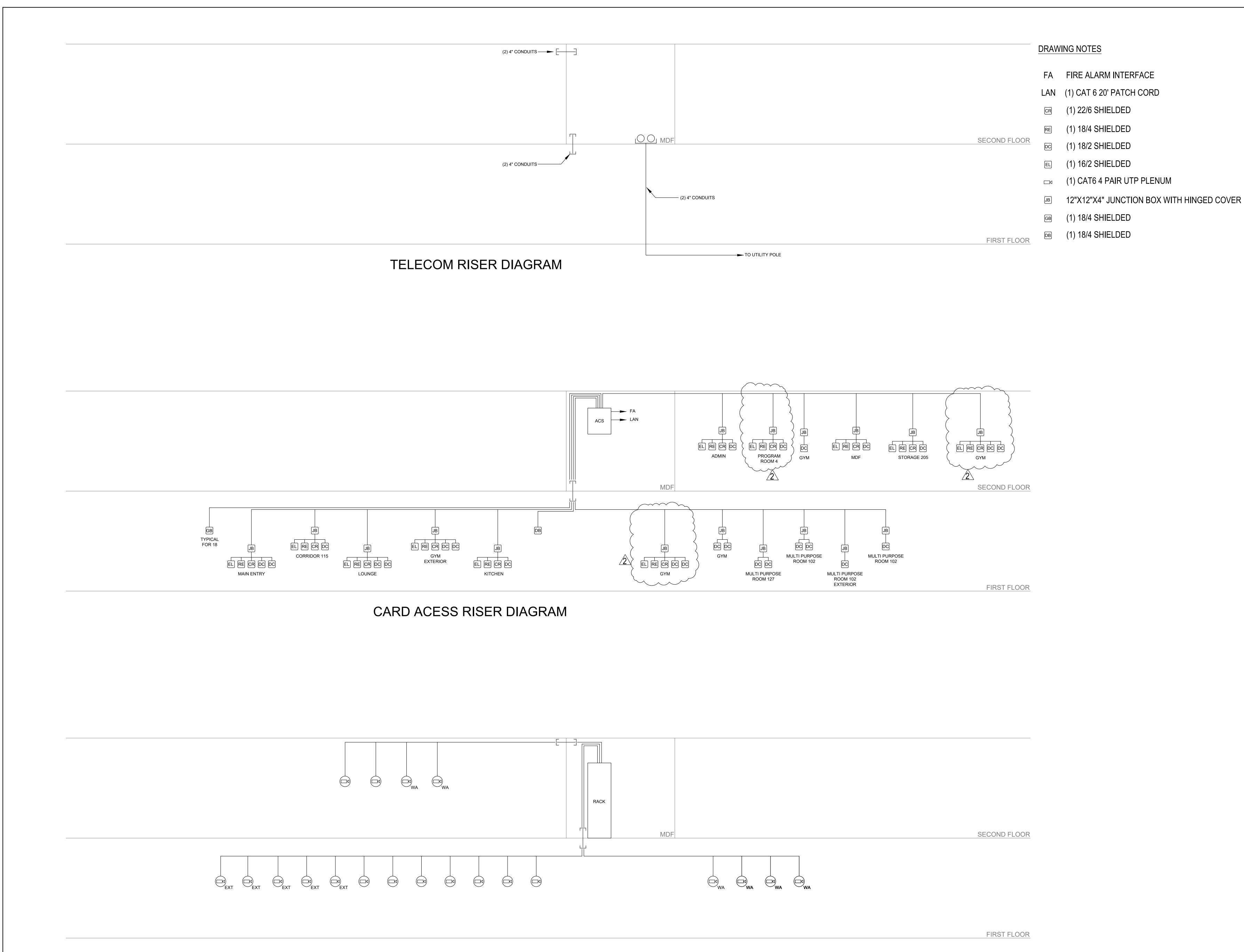
Technology Second Floor Plan

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DATE OF ISSUE

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EVISIONS

2\ ADDENDUM 2

DRAWING TITLE

Technology Risers

DRAWING INFORMATION

7/22/22

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DATE OF ISSUE

Site Plan Review

DESCRIPTION

1/8" = 1'-0"

SCALE

DRAM

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DJECT# FILE NAME

T204

T301