<b>3</b> 1.	THE GENERAL NOTES APPLY UNLESS NOTED OTHERWISE ON THE DRAWINGS OR IN THE S	SPECIFICATIONS.	<u>CAST</u> C1	-IN-PLACE CONC
<del>3</del> 2.	STRUCTURAL WORK SHALL CONFORM TO REQUIREMENTS OF THE "MASSACHUSETTS STA	ATE BUILDING CODE" 780 CMR, NINTH	C2.	CONCRETE SH
<del>3</del> 3.	THE INTENT OF THE STRUCTURAL DRAWINGS IS TO SHOW THE MAIN STRUCTURAL FEATU COMPLETED PROJECT. ARCHITECTURAL DETAILS AND OTHER COMPONENTS THAT MAY B PROJECT ARE SHOWN INCIDENTALLY ONLY AND NOT COMPLETELY.	RES AND DESIGN FOR THE E NECESSARY TO CONSTRUCT THE	C3.	CONCRETE QL THE STRUCTU SPECIFICATIO
64.	STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, S PLUMBING, AND FIRE PROTECTION DRAWINGS, APPROVED SHOP DRAWINGS, AND SPECIF	TTE, MECHANICAL, ELECTRICAL, FICATIONS.	C4. C5.	NORMAL WEIG
5.	REFER TO ARCHITECTURAL, SITE, MECHANICAL, ELECTRICAL, PLUMBING, AND FIRE PROT VERIFICATION OF LOCATIONS AND DIMENSIONS OF ALL SHAFTS, INSERTS, CURBS, OPEN FLOOR PITCHES, ANGLE FRAMES, AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON	ECTION DRAWINGS FOR INGS, SLEEVES, ANCHOR BOLTS, I STRUCTURAL DRAWINGS.	C6.	CONCRETE MI A. FOOTIN B. SLABS C. SITE P
6.	THE CONTRACTOR SHALL INFORM THE ARCHITECT OF ALL DISCREPANCIES BETWEEN DR. PRIOR TO INITIATION OF ANY WORK.	AWINGS OF DIFFERENT TRADES	C7.	REINFORCING 40 DIAMETERS
7.	EXISTING DIMENSIONS AND CONDITIONS MUST BE VERIFIED OR DETERMINED IN THE FIEL BROUGHT TO THE ATTENTION OF THE ARCHITECT BEFORE PROCEEDING WITH THE AFFE	D AND ANY DISCREPANCIES SHALL BE CTED PORTION OF THE WORK.	C8.	PROVIDE A MII OTHERWISE.
3. 9.	THE CONTRACTOR SHALL PROVIDE ALL THE NECESSARY ENGINEERED TEMPORARY BRAC SUPPORT THE NEW AND EXISTING WORK AND THE APPLIED LOADS UNTIL THE PERMANEN AND AT FULL STRENGTH. SHOP DRAWINGS FOR REINFORCING STEEL, STRUCTURAL STEEL, STEEL DECK AND PREF	CING AND SHORING TO SAFELY IT STRUCTURE IS FULLY INSTALLED ABRICATED WOOD TRUSSES SHALL	C9.	WELDED WIRE TIE AT 3'-0" o.c. EACH WAY.
10.	BE SUBMITTED TO THE ARCHITECT AND A STAMPED APPROVAL RECEIVED BEFORE FABRI AND ERECTION SHALL PROCEED FROM APPROVED SHOP DRAWINGS ONLY. NOTES AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TO UNLESS NOTED	CATION MAY PROCEED. FABRICATION YPICAL FOR ALL SIMILAR CONDITIONS,	C10.	PROVIDE REIN RECOMMENDE SLAB CHAIRS,
11.	REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.		C12.	
912. 913.	THE CONTRACTOR SHALL SUBMIT LOADING DATA FOR ANY EQUIPMENT SUCH AS LIFTS, E BUILDING DURING CONSTRUCTION. BUILDING CONSTRUCTION TYPE IS IIIB, FLOOR AND ROOF CONSTRUCTION ARE TO BE COI	TC. PROPOSED FOR USE ON THE NSIDERED RESTRAINED.		B. FOUND C. PILAST D. INTERI E. EXTER
<u>STRUC</u> I.B.C.	<u> CTURAL LOADS - MASSACHUSETTS STATE BUILDING CODE (780 CMR) - NINTH EDITION</u> 2015 WITH MASSACHUSETTS AMENDMENTS)		C13.	SET AND TIE A
1.	DEAD LOADS A. WEIGHT OF BUILDING COMPONENTS		C14.	CONCRETE IS
	1.TYPICAL FLOOR30 PSF2.SUSPENDED TRACK IN GYMNASIUM75 PSF3.TYPICAL BOOF AREAS25 PSF	= = =	C15.	RECESSES. RE
	4.GYMNASIUM ROOF (INCLUDING FUTURE PV ALLOWANCE)45 PSFB.TYPICAL PARTITIONS ALLOWANCE - (I.B.C SECTION 1607.5)15 PSF	= =	C16.	NO CONDUITS
2.	SNOW LOADS A. GROUND SNOW LOAD - (MA TABLE 1604.11) Pg	= 30 PSF	C17.	KEYS SHALL BI
	B.SNOW IMPORTANCE FACTOR - (ASCE 7-10 - TABLE 1.5-2)IsC.EXPOSURE FACTOR - (ASCE 7-10 - TABLE 7-2)CeD.THERMAL FACTOR - (ASCE 7-10 - TABLE 7-3)Ct	= 1.0 = 1.0 = 1.1	C18.	DOWELS AND ACCURATELY.
	E.       FLAT ROOF SNOW LOAD - (ASCE 7-10 - SECTION 7.3)       Pf         F.       ROOF SLOPE FACTOR - (ASCE 7-10 - FIGURE 7-2)       Cs         G.       SLOPED ROOF SNOW LOAD (ASCE 7-10 - SECTION 7.4)       P	= 30 PSF = 1.0 = 21 PSE	C19.	HORIZONTAL C APPROVED BY
	<ul> <li>G. SLOF LD ROOF SNOW LOAD (ASCE 7-10 - SECTION 7.4)</li> <li>H. UNBALANCED SNOW LOAD (ASCE 7-10 - SECTION 7.6)</li> <li>I. SLIDING SNOW LOAD ON LOWER ROOF AREAS (ASCE 7-10 - SECTION 7.9)</li> <li>J. SNOW DRIFT - (ASCE 7-10 - FIGURES 7-7, 7-8 &amp; 7-9)</li> </ul>	= OVER 9 FEET WIDTH ALONG RIDGE =	C20.	CONSTRUCTIC
i.	LIVE LOADS A. LOADS (I.B.C TABLE 1607.1) 1. OFFICES 65 PS	F (INCLUDING PARTITION ALLOWANCE)	C21.	SEE ARCHITEC FEATURES.
	2.CORRIDORS ABOVE THE FIRST FLOOR80 PS3.STAIRS100 PS4.GYMNASIUM AND OPEN PLAN AREAS100 PS	F SF SF	C22.	PROVIDE CON ACCORDANCE
	5. STORAGE 125 PS 6. MECHANICAL EQUIPMENT ROOMS 150 PS B LIVE LOAD REDUCTION (LB C - SECTION 1607 10)	SF SF (100PSF+50PSF EQUIPMENT PADS)	C23.	PROVIDE SEAL
	C. CONSTRUCTION LOADS 1. UNIFORM LOAD (STEEL DECK) 2. CONCENTRATED LOAD (BARE STEEL DECK) 3. CONCENTRATED LOAD (SLABS ON COMPOSITE DECK) 1500 L	F PLF (PERPENDICULAR TO DECK SPAN) BS_MAX WHEEL LOAD	C24. C25.	EXPOSED EDG NOT ALL OPEN OR ANY ADDIT
4.	WIND LOADS <u>NOTE:</u> PER ASCE 7-10 AND 2015 IBC, THE DESIGN WIND PRESSURES LISTED BELOW REFLE <u>DEFINITIONS NOW INCLUDE A 0.6 FACTOR FOR WIN</u>	ECT ULTIMATE STRENGTH VALUES.	UNIT	CONCRETE.
	A. WIND LOAD PARAMETERS (ASCE 7-10 - CHAPTER 26) 1. RISK CATEGORY (IBC 2015 - TABLE 1604.5) 2. ULTIMATE DESIGN WIND SPEED (MA TABLE 1604.11)	$CATEGORY II$ $V_{ult} = 134 MPH$	<u>онн 1</u> M1.	CONCRETE MA COMMENTARY COMMENTARIE
	<ol> <li>NOMINAL DESIGN WIND SPEED (IBC 2015 - EQUATION 16-33)</li> <li>WIND DIRECTIONALITY FACTOR (ASCE 7-10 - TABLE 26.6-1</li> <li>WIND EXPOSURE CATEGORY (ASCE 7-10 - SECTION 26.7)</li> </ol>	$V_{asd} = 104 MPH$ $K_d = 0.85$ EXPOSURE C	М2.	CONCRETE MA
	<ol> <li>TOPOGRAPHIC FACTOR (ASCE 7-10 - FIGURE 26.8-1)</li> <li>ENCLOSURE CLASSIFICATION (ASCE 7-10 - SECTION 26.10)</li> <li>INTERNAL PRESSURE COEFFICIENT (ASCE 7-10 - TABLE 26.11-1)</li> <li>VELOCITY PRESSURE COEFFICIENT (ASCE 7-10 - TABLE 28.3-1)</li> </ol>	$K_{zt} = 1.0$ ENCLOSED $GC_{pi} = \pm 0.18$ $K_h = 1.03$ c = 40.11 PSE	МЗ.	CONCRETE MA LOAD BEARING STRENGTH (fm THE AVERAGE
	<ul> <li>B. COMPONENTS AND CLADDING (ASCE 7-10 - CHAPTER 30, PART 1: LOW-RISE BUILDI</li> </ul>	$q_h = 40.11 PSP$	M4.	MORTAR FOR
	<ol> <li>EFFECTIVE WIND AREA (ASCE 7-10 - FIGURE 30.4-1)</li> <li>WIDTH OF SALIENT ZONE</li> <li>NET DESIGN WIND PRESSURES (ASCE 7-10 - EQUATION 30.4-1)</li> </ol>	Aeff ≤ 10 SQ. FT. a = 9 FEET	M5.	GROUT SHALL
	a. WALL ELEMENTS i. AT A NON-SALIENT AREA ii. AT A SALIENT CORNER	p <sub>net</sub> = +43 PSF / -47 PSF p <sub>net</sub> = +43 PSF / -58 PSF	M6.	CONCRETE MA
	b. ROOF ELEMENTS i. AT A NON-SALIENT AREA ii. AT AN EDGE	$p_{net} = +19 PSF / -47 PSF$ $p_{net} = +19 PSF / -79 PSF$	M7.	HORIZONTAL J CORNERS AND
	iii. AT A SALIENT CORNER c. OVERHANG ELEMENTS i. AT AN EDGE ii. AT A SALIENT CORNER	$p_{net} = +19 PSF / -120 PSF$ $p_{net} = -75 PSF$ $p_{net} = -120 PSF$	M8.	MINIMUM HOR AT A MINIMUM, OF WALL.
5.	SEISMIC LOAD CRITERIA - GENERAL A. SEISMIC OCCUPANCY CATEGORY - (ASCE 7-10 - TABLE 1.5-1)	RISK CATEGORY II	M9.	REINFORCING 48 DIAMETERS
	B.       MAPPED SPECTRAL ACCELERATION FOR SHORT PERIODS - (MA TABLE 1604.11)         C.       DESIGN SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS         D.       MAPPED SPECTRAL ACCELERATION FOR 1-SECOND PERIOD - (MA TABLE 1604.11)	$S_S = 0.195g$ $S_{DS} = 0.208$ $S_1 = 0.064$	M10.	ELEVATOR SH, SHALL BE REIN
	E. DESIGN SPECTRAL RESPONSE ACCELERATION FOR 1-SECOND PERIOD F. SITE CLASS - REFER TO GEOTECHNICAL REPORT G. SEISMIC DESIGN CATEGORY - (ASCE 7-10 - TABLES 11.6-1 & 11.6-2) H. SEISMIC IMPORTANCE FACE - (ASCE 7-10 - TABLE 11.5-1)	$S_{D1} = 0.102$ SITE CLASS D CATEGORY B $I_e = 1.0$	М11. M12.	THE TOP OF C
	SEISMIC LOADS - GYMNASIUM A. BASIC SEISMIC-FORCE-RESISTING SYSTEM - (ASCE 7-10 - TABLE 12.2-1)	STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE	<u>STRU</u> S1.	CTURAL STEEL F
	<ul> <li>B. RESPONSE MODIFICATION COEFFICIENT - (ASCE 7-10 - TABLE 12.2-1)</li> <li>C. DEFLECTION AMPLIFICATION FACTOR - (ASCE 7-10 - TABLE 12.2-1)</li> <li>D. SYSTEM OVERSTRENGTH FACTOR - (ASCE 7-10 - TABLE 12.2-1)</li> </ul>	$ \begin{array}{rcl} R &= 3.0 \\ C_d &= 3.0 \\ \Omega_0 &= 3.0 \end{array} $	S2.	BUILDINGS" AN WELDING SHA
	E. SEISMIC RESPONSE COEFFICENT F. DESIGN BASE SHEAR G. ANALYSIS PROCEDURE USED SEISMIC LOADS - COMMUNITY CENTER	Cs = 0.069 V = 70 KIPS EQUIVALENT LATERAL FORCE	S3.	STRUCTURAL A. PLATES B. HOLLO
	A. BASIC SEISMIC-FORCE-RESISTING SYSTEM - (ASCE 7-10 - TABLE 12.2-1)	LIGHT-FRAME WOOD WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE	S4.	C. STRUC D. W & W CONNECTIONS
	<ul> <li>Response Mobilication Coefficient - (ASCE 1-10 - TABLE 12.2-1)</li> <li>B. DEFLECTION AMPLIFICATION FACTOR - (ASCE 7-10 - TABLE 12.2-1)</li> <li>C. SYSTEM OVERSTRENGTH FACTOR - (ASCE 7-10 - TABLE 12.2-1)</li> <li>D. SEISMIC RESPONSE COEFFICENT</li> <li>E. DESIGN BASE SHEAR</li> </ul>	$ \begin{array}{l}                                     $	S5.	AND DETAILED CONNECTIONS BOLTS.
	F. ANALYSIS PROCEDURE USED	EQUIVALENT LATERAL FORCE	S6.	PROVIDE 3/4" [
			S7.	FURNISH AND
			S9.	OF STEEL CON PROVIDE A 1/4 PLATES. LEVEL
			S10.	HAVE ATTAINE PROVIDE A 1/4
			S12.	SPLICING STR
			S13.	STRUCTURAL
			S15.	STRUCTURAL
			S16.	REFER TO THE
			S17.	THE CONTRAC NEW STRUCTL REQUIRED FO
			<u>SHEA</u>	R CONNECTORS
			SUT	SHEAR
			SC1. SC2.	SHEAR CONNE

NCRETE (SECTION 033000) WORK SHALL CONFORM TO THE LATEST "BUILDING	CODE REQUIREMENTS FOR S	TRUCTURAL CONCRETE (ACI 318)".	<u>STEEL</u> D1.	<u>. DECK (SECTION 053100)</u> STEEL DECK WORK SHALL CONFORM TO "SPECIFICATION FOR
SHALL BE PLACED IN THE PRESENCE OF THE APPRO QUALITY IN ACCORDANCE WITH THE REQUIREMENT TURAL PERFORMANCE OF THE BUILDING, CONCRET	OVED TESTING AGENCY. IS OF THESE DRAWINGS AND S E THAT IS NOT IN ACCORDANC	SPECIFICATIONS IS ESSENTIAL TO	D2.	COMPOSITE STEEL FLOOR DECK SHALL BE MADE FROM STEEL
IONS WILL NOT BE ACCEPTED. EXPOSED TO WEATHER SHALL CONTAIN AN AIR ENT	RAINMENT ADMIXTURE.		D3.	GAUGE SHALL BE AS NOTED ON THE DRAWINGS. STEEL ROOF DECK SHALL BE MADE FROM STEEL CONFORMIN
EIGHT CONCRETE SHALL HAVE AN AIR-DRY UNIT WEI MINIMUM 28-DAY STRENGTH, UNLESS NOTED OTHEF	IGHT OF 145 PCF. RWISE, SHALL CONFORM TO F	OLLOWING:	D4.	ACCORDANCE WITH ASTM A653, COATING CLASS G60. STEEL DRAWINGS. PROVIDE CONTINUOUS POUR STOP ANGLES WITH RETURN LII
TINGS, PIERS, FOUNDATION WALLS, + GRADE BEAMS 3S ON GRADE + SLABS ON METAL DECK: PAVING:	S: 3000 PSI (NORMAI 4000 PSI (NORMAI 4500 PSI (NORMAI	_ WEIGHT) _ WEIGHT) _ WEIGHT)	D5.	TO THE SCHEDULE FOR GAUGE. COMPOSITE STEEL FLOOR DECK SHALL BE ATTACHED TO THE
IG STEEL SHALL CONFORM TO ASTM A615, GRADE 6 RS, UNLESS NOTED. PROVIDE MATCHING CORNER A	0, DEFORMED BARS. LAP ALL AND INTERSECTION BARS.	CONTINUOUS BARS A MINIMUM OF		REQUIREMENTS:         A.       PANEL ENDS AND END LAPS       5         B.       INTERMEDIATE SUPPORTS       5         O       LONOTUDINAL EDODO AT OUE OUPPORTO       5
MINIMUM OF #4 AT 12" EACH WAY, EACH FACE, FOR A	ALL WALLS, FOOTINGS, PITS, (	DR PADS, UNLESS NOTED	D6	C. LONGITUDINAL EDGES AT SIDE SUPPORTS 5 D. SIDE LAPS OF ADJACENT UNITS E
RE FABRIC SHALL CONFORM TO ASTM A 1064 IN FLA b.c. AT SLAB ON GRADE, PLACE WELDED WIRE FABRI	T SHEETS. LAP ONE AND ONE IC ON SLAB BOLSTERS WITH S	-HALF SQUARES AT ALL JOINTS AND AND PLATES SPACED AT 3'-0" o.c.	<i>D</i> 0.	REQUIREMENTS:A.PANEL ENDS AND END LAPSB.INTERMEDIATE SUPPORTSC.LONGITUDINAL EDGES AT SIDE SUPPORTS
INFORCING STEEL DETAILING, LAP SPLICES, EMBED DED IN THE LATEST EDITION OF THE "ACI DETAILING S, IN CONTACT WITH EXPOSED SURFACES, SHALL B	MENTS, BAR SUPPORTS, SPA MANUAL". ACCESSORIES, SU E ZINC COATED AND PLASTIC	CERS, AND ACCESSORIES AS CH AS SLAB BOLSTERS AND BEAM AND TIPPED.	D7.	D. SIDE LAPS OF ADJACENT UNITS 1 DO NOT HANG M.E.P., F.P. OR ARCHITECTURAL COMPONENTS FRAMING BY THE COMPONENT CONTRACTOR TO SPAN BETW
IG STEEL DETAILS NOT SHOWN ON THE DRAWINGS CRETE COVER FOR REINFORCING BARS OR WELDEL	SHALL BE IN ACCORDANCE W D WIRE FABRIC SHALL CONFO	ITH THE "ACI DETAILING MANUAL 2004". RM TO THE FOLLOWING, UNLESS NOTED:	ROUG	H CARPENTRY (SECTION 061000)
TINGS:     3"       NDATION WALLS:     1-1/2"       STERS/PIERS:     1-1/2" TO TIES       STOR OF ADD STATES     1.1/2" TO TIES			RC1.	STRUCTURAL LUMBER SHALL CONFORM TO THE AF&PA, "NAT SUPPLEMENT, "DESIGN VALUES FOR WOOD CONSTRUCTION"
RIOR SLABS ON GRADE: 1-1/2" FROM TOP ERIOR SLABS ON GRADE: MID-DEPTH PINGS ON METAL DECK: 1" FROM TOP			RC2.	SAWN LUMBER SHALL BE SPRUCE-PINE-FIR NO. 2 OR BETTER, PLATES
ALL REINFORCING STEEL BEFORE PLACING CONCE IS PROHIBITED.	RETE. SETTING DOWELS AND I	REINFORCING STEEL INTO WET	RC3.	FOUNDATION SILLS SHALL BE PRESERVATIVE PRESSURE TRE PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DI
RCING STEEL SHALL BE CUT OR OMITTED IN THE FIE REINFORCING STEEL MAY BE MOVED ASIDE WITHOU RECESS, OPENINGS, OR SLEEVES SHALL BE INSTA	LD BECAUSE OF CONFLICT WI JT CHANGE IN LEVEL, WITH TH LLED IN CONCRETE WITHOUT	TH SLEEVES, DUCT OPENINGS, OR HE APPROVAL OF THE ARCHITECT. APPROVAL OF THE ARCHITECT.	RC4.	WOOD SHALL BE STAINLESS STEEL. WOOD EXPOSED TO WEATHER SHALL BE PRESERVATIVE PRE PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DI WOOD SHALL BE STAINLESS STEEL.
TS SHALL BE PLACED IN CONCRETE SLABS ON META	NL DECK. ESS NOTED OTHERWISE		RC5.	LAMINATED VENEER LUMBER (LVL) SHALL BE MICRO-LAM, AS
D ANCHOR RODS SHALL BE SET BY TEMPLATE. SET Y.	EMBEDDED ITEMS FOR CONN	ECTION OF OTHER WORK	RC6. RC7.	PARALLEL STRAND LUMBER (PSL) SHALL BE PARALLAM, AS MA ENGINEERED I - JOISTS SHALL BE TJI JOISTS, AS MANUFACTUR
L CONSTRUCTION JOINTS SHALL BE AS INDICATED O BY THE ARCHITECT. CONSTRUCTION JOINTS SHALL	ON THE DRAWINGS. VERTICAL BE FORMED WITH A STANDAR	CONSTRUCTION JOINTS SHALL BE	RC8.	STIFFENERS/SQUASH BLOCKS PER MANUFACTURER'S RECOM FLUSH FRAMED CONNECTIONS SHALL HAVE METAL BEAM OR CO., INC., OR EQUAL
A MINIMUM OF 40 DIAMETERS, UNLESS NOTED. ALL C WATERSTOPS.	CONSTRUCTION JOINTS BELO	W GRADE SHALL HAVE CONTINUOUS	RC9.	ALL INDIVIDUAL POSTS SHALL HAVE METAL CAPS AND BASES, EQUAL.
ECTURAL AND SITE DRAWINGS FOR FINISHES. DEPR	THOSE SHOWN ON THE DRAW T LOCATIONS ARE MANDATOR	RY AS SHOWN.	RC10.	ROOF SHEATHING SHALL BE A MINIMUM OF 5/8" NOMINAL (19/3 STRUCTURAL 1 PLYWOOD SHEATHING WITH 10d NAILS 6" o.c. A
DNCRETE PADS FOR MECHANICAL EQUIPMENT ACCO	DRDING TO THE REQUIREMEN	TS OF THE MANUFACTURER AND IN	RC11.	PROVIDE METAL "H" CLIPS AT PANEL EDGES. WALL SHEATHING SHALL BE A MINIMUM OF 1/2" NOMINAL (15/3 STRUCTURAL 1 PLYMOOD SHEATHING WITH 8d NAU S 6" o c. A
CE WITH THE TYPICAL DETAILS. COORDINATE LOCAT ALANT FOR ALL EXPOSED-TO-VIEW CONSTRUCTION	TIONS WITH M.E.P. WORK. I JOINTS, CONTROL JOINTS, A	ND SHEAR KEYS.	RC12.	BLOCK ALL EDGES OF PLYWOOD WALL SHEATHING. FLOOR SHEATHING SHALL BE A MINIMUM OF 3/4" NOMINAL (23.
DGES OF CONCRETE ELEMENTS SHALL HAVE A 1-INC	CH CHAMFER UNLESS NOTED RE SHOWN ON STRUCTURAL I	OTHERWISE. DRAWINGS. OPENINGS INDICATED.		STRUCTURAL 1 PLYWOOD TONGUE AND GROOVE, GLUED ÀNE AND 12" o.c. AT INTERMEDIATE SUPPORTS.
DITIONAL OPENINGS OR INSERTS REQUIRED, SHALL	BE VERIFIED WITH RESPECTIN	E TRADES PRIOR TO PLACING	RC13. RC14.	PLYWOOD SHALL HAVE STAGGERED JOINTS AND NAILS SHALL INSTALLED WITH THE FACE GRAIN PERPENDICULAR TO THE S NAILING SHALL BE IN ACCORDANCE WITH THE 2015 INTERNAT
<u>CTION 042000)</u> MASONRY CONSTRUCTION WORK SHALL CONFORM BY FOR MASONRY STRUCTURES (ACL 530/530R)" AND	TO LATEST EDITION OF "BUIL	DING CODE REQUIREMENTS AND	RC15.	FLOOR JOISTS SHALL BE BRIDGED AT 8'-0" o.c. MAX.
RIES (ACI 530.1/530.1R)". CONCRETE MASONRY WALL METHOD.	CONSTRUCTION SHALL BE IN	I ACCORDANCE WITH LOW LIFT	RC16. RC17.	ALL WALL STUDS SHALL BE BLOCKED AT 4'-0" o.c. MAX. AND AT PROVIDE SOLID BLOCKING BETWEEN JOISTS AND RAFTERS O
MASONRY STRENGTH (fm) SHALL NOT BE LESS THAI MASONRY UNITS (CMU) SHALL CONFORM TO ASTM (	N 1500 PSI WITH SPECIAL INSF C90, NORMAL WEIGHT, TYPE 1	PECTION. AND TO NCMA "REQUIREMENTS FOR	RC18.	ALL POSTS SHALL BE (VERTICALLY) BLOCKED THROUGH FLOC FOUNDATION WALL OR SUPPORTING BEAM.
NG CONCRETE MASONRY". COMPRESSIVE STRENG (fm), BUT NOT LESS THAN 2000 PSI FOR THE AVERAG GE NET AREA.	GE OF 3 UNITS OR 1700 PSI FO	OR SPECIFIED CONCRETE MASONRY R AN INDIVIDUAL UNIT, BASED ON	RC19.	PROVIDE MINIMUM HEADERS AS REQUIRED BY TABLES 2308.4 CODE, UNLESS OTHERWISE NOTED.
R REINFORCED CMU SHALL CONFORM TO ASTM C27 HE SPECIFIED CONCRETE MASONRY STRENGTH (fm LL CONFORM TO ASTM C476, FINE TYPE, AND HAVE J MASONRY STRENGTH (fm) BUT NOT LESS THAN 200	70, TYPE M OR S, AND HAVE A h), BUT NOT LESS THAN 1800 P A 28-DAY COMPRESSIVE STRE	28-DAY COMPRESSIVE STRENGTH SI. ENGTH EQUAL TO THE SPECIFIED	RC20.	PROVIDE MINIMUM BUILT-UP WALL STUDS AT JAMBS OF ALL W MORE STRINGENT REQUIREMENTS ARE NOTED ON THE DRAW OPENING SIZE JACK STUDS KING STUDS UP TO 4'-0" 1 1 4'-0" TO 6'-0" 1 2
SHALL BE LIMITED TO A MAXIMUM WALL HEIGHT OF S	5'-4" PER LIFT.			6'-0" TO 8'-0" 2 2 8'-0" TO 10'-0" 2 3
L JOINT REINFORCEMENT SHALL CONFORM TO ASTI ND TEES.	M A82, LADDER TYPE, #9 WIRE	. PROVIDE PREFABRICATED		ALL TRIMMERS AND JACK STUDS SHALL BE OF THE SA OTHERWISE NOTED ON THE DRAWINGS. ALL TRIMME TO THE CAP PLATE. JACK STUDS SHALL TERMINATE B TRIMMERS. ERAMING SHOWN IN THE TABLE ABOVE IS
IRIZONTAL JOINT REINFORCEMENT FOR WALLS AND IM, PROVIDE A BOND BEAM, WITH 2-#5 HORIZONTAL IG STEEL SHALL CONFORM TO ASTM A615, GRADE 6	0 PARTITIONS SHALL BE #9 WII AND CONTINUOUS BARS, AT E 0, DEFORMED BARS. LAP ALL	CONTINUOUS BARS A MINIMUM OF		AND TRIMMERS FOR MULTIPLE OPENINGS IMMEDIATED DISTANCE BETWEEN OPENINGS DOES NOT ACCOMMO ELIMINATE THE JACK STUD(S) AND CONNECT THE HEA HANGER (CONCEALED FLANGES).
RS. SHAFT WALLS SHALL BE FULLY GROUTED SOLID AND	N ADDITION TO HORIZONTAL		RC21. RC22	PROVIDE A MINIMUM OF 3 - 2X CORNER POSTS AT ALL CORNE
CMU WALLS AND PARTITIONS SHALL BE ANCHORED	AS SHOWN IN THE TYPICAL L	). DETAILS AND THE SECTIONS.	RC23.	PROVIDE METAL HURRICANE ANCHORS AT ALL ROOF RAFTER
RCHITECTURAL DRAWINGS AND SPECIFICATIONS FO	OR REQUIRED FIRE RATINGS.		RC24.	FRAME ALL OPENINGS IN FLOOR AND ROOF CONSTRUCTION N MATCH THE ADJACENT FRAMING) WITH METAL JOIST/BEAM HA
<b>L FRAMING (SECTION 051200)</b> IL STEEL WORK SHALL CONFORM TO THE LASTEST B	EDITIONS OF AISC 360 "SPECIF	FICATION FOR STRUCTURAL STEEL	RC25.	NOTCHING OF JOISTS, BEAMS, STUDS OR PLATES SHALL NOT
AND AISC "CODE OF STANDARD PRACTICE FOR STE HALL BE IN ACCORDANCE WITH THE LATEST AWS "D	EL BUILDINGS AND BRIDGES", 1.1-STRUCTURAL WELDING CO	AS MODIFIED BY THE SPECIFICATIONS.	<u>METAI</u> WT1.	L PLATE CONNECTED WOOD TRUSSES (SECTION 061753) ALL ROOF TRUSSES SHALL CONFORM TO THE TRUSS PLATE IN CONNECTED WOOD TRUSSES" LATEST EDITION AND SHALL D
L STEEL SHALL CONFORM TO THE FOLLOWING, UNL TES AND OTHER SHAPES (OTHER THAN W & WT)	ESS NOTED: ASTM A36 ASTM A500 GRADE C	Fy = 36KSI Fy = 50KSI (SQUARE & RECT)		TOTAL AND LIVE LOAD DEFLECTIONS SHALL BE LIMITED AS FC
JCTURAL PIPE WT SHAPES	ASTM A500 GRADE B ASTM A53 GRADE B ASTM A992 OR A572	Fy = 42KSI (ROUND) Fy = 35 KSI Fy = 50KSI	WT2.	TOTAL LOAD DEFLECTION:< L/360SUBMIT COMPLETE SHOP DRAWINGS (ERECTION PLANS, MEM
NS MAY BE BOLTED OR WELDED, UNLESS SPECIFIC, ED IN ACCORDANCE WITH AISC STANDARDS, USING	ALLY NOTED OTHERWISE. CO THE ASD METHOD.	NNECTIONS SHALL BE DESIGNED		MEMBERS AND SUPPORTS, BRACING, ETC.) AND CALCULATIO SHOP DRAWINGS SHALL BE PREPARED BY AND BEAR THE STA STATE OF MASSACHUSETTS. FINAL DESIGN OF TRUSSES SHA
NS SHALL BE WELDED TO CONFORM TO ASTM A233,	E70 SERIES, OR BOLTED TO (	CONFORM TO ASTM A325, TYPE N	WT3.	IN ADDITION TO THE SLOPING AND UNBALANCED SNOW LOAD ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING DI CHORD (BC):
" DIAMETER MINIMUM HEADED TYPE ANCHOR RODS	S AT COLUMNS AND POSTS, UI WITH ALL ANCHOR RODS, UN	NLESS NOTED OTHERWISE. LESS NOTED OTHERWISE.		LIVE LOAD = 46 PSF (TC), 20 PSF (BC) DEAD LOAD = 15 PSF (TC), 10 PSF (BC) INCLUDE DETAILED LOAD COMBINATIONS FOR ALL TRUSS CAL
PORTED BEAM-TO-BEAM CONNECTIONS SHALL BE D ONSTRUCTION", UNLESS SPECIFICALLY NOTED OTH	OOUBLE ANGLE TYPE IN CONF ERWISE ON THE STRUCTURAL	ORMANCE WITH THE AISC "MANUAL DRAWINGS.	WT4	TRUSS WEB CONFIGURATIONS SHOWN ON THE TRUSS ELEVA DETERMINED BY THE TRUSS DESIGNER / FABRICATOR. COOR
1/4" THICK LEVELING PLATE UNDER EACH COLUMN B /ELING PLATE SHALL BE SET AND GROUTED WITH AI	BASE PLATE FOR USE IN ALIGN N APPROVED NON-SHRINK, NO	IING ANCHOR RODS AND BASE DN-METALLIC GROUT. GROUT SHALL	WT5	UNITS OR PIPING IS NOT PERMITTED.
1/4" THICK MINIMUM CAP PLATE WELDED AT TOP OF TRUCTURAL MEMBERS WHERE NOT DETAILED ON DF	HSS COLUMNS, UNLESS NOTE RAWINGS IS PROHIBITED WITH	ED OTHERWISE. HOUT PRIOR APPROVAL OF	WT6.	BE 2x4. DESIGN ALL WEB MEMBERS HAVING A LENGTH LESS T WEB MEMBER DOES NOT REQUIRE LATERAL RESTRAINT OR B TRUSS CHORDS AND TRUSS WEB MEMBERS SHALL BE SOUTH
L STEEL EXPOSED TO THE WEATHER IN THE FINISH	ED PROJECT SHALL BE HOT D	IP GALVANIZED TO CONFORM TO	WT7.	ALL TRUSSES SHALL BE SECURELY BRACED BOTH DURING EF REQUIRED ON THE STRUCTURAL DRAWINGS THE SHOP DRAW
L STEEL EXPOSED TO VIEW IN THE COMPLETED PRO HE SPECIFICATIONS FOR SPECIFIC REQUIREMENTS	OJECT SHALL BE EXPOSED TO	) VIEW STRUCTURAL STEEL (E.V.S.S.).	WT8.	PUBLICATION BCSI 1-03, OR AS DIRECTED IN THE FIELD BY THE WEB TRUSS-TO-TRUSS CONTINUOUS LATERAL BRACING (CLB
HE SPECIFICATION FOR PAINTING AND SURFACE PR	EPARATION REQUIREMENTS.		WT9.	TO DETERMINE FINAL WEB LAYOUT (SEE NOTE #4) AND PROVI CONTINUOUS LATERAL BRACING LINES (CLB) SHALL BE RESTR
ACTOR SHALL PROVIDE ALL NECESSARY TEMPORAF TURE FOR WIND AND CONSTRUCTION LOADS. TEMP FOR STABILITY OF THE STEEL FRAME ARE COMPLET	₹Y GUYING AND BRACING REG PORARY SUPPORTS SHALL RE ED.	QUIRED TO ERECT AND HOLD THE MAIN IN PLACE UNTIL ALL ELEMENTS		CLB LINE AND AT 20'-0" O.C. MAX, PER BCSI 2013 AND THE TYP. THIS DRAWING. ALL WEB MEMBERS REQUIRING CLB AS SHOV BRACING.
RS (SECTION 051226)			WT10.	CONTINUOUS LATERAL BRACING REQUIREMENTS: a) TOP CHORD: SEE BOTH STRUCTURAL DRAWINGS AND SHO MORE STRINGENT REQUIREMENT.
NECTORS SHALL BE HEADED STUDS CONFORMING NECTORS SHALL BE 3/4" DIAMETER WITH A LENGTH	TO ASTM A108, GRADES 1010, EQUAL TO TOTAL SLAB THICK	1015, OR 1020. NESS MINUS 1 1/2 INCHES UNLESS		b) BOTTOM CHORD: PROVIDE CONTINUOUS LATERAL BRACIN 6'-0" o.c. MAX. SPACING. IF GYPSUM SHEATHING IS APPLIED DI LATERAL BRACING MAY BE OMITTED.
ERWISE THE NUMBER OF SHEAR CONNECTORS REG NNECTORS UNIFORMLY ALONG THE LENGTH OF THE AM LENGTH ON ALL BEAMS SUPPORTING CONCRET	QUIRED PER BEAM IS INDICATI E BEAM UNLESS NOTED. INSTA E SLABS OVER COMPOSITE D	ED THUS (X) ON THE DRAWINGS. ALL AT LEAST ONE CONNECTOR PER ECK.	WT11.	C) WEB MEMBERS: SEE THE SHOP DRAWINGS TO IDENTIFY W PROVIDE SIMPSON "H" SERIES HOLDDOWN CONNECTORS (OF BEARING POINTS). TRUSS DESIGNER SHALL SELECT AND SPE
				EXCEEDS THE LIPI IET FORCE PROVIDED IN THE TRUSS SHOP

ONFORM TO "SPECIFICATION	FOR DESIGN OF LIGHT GAUGE COLD-FORMED STEEL STRUCTURAL	F1.	FOUNDAT	
,K INSTITUTE DESIGN MANUA NG CODE - STEEL (AWS D1.1)"; ECK SHALL BE MADE FROM S DANCE WITH ASTM A653, COA	L FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS", AND "STRUCTURAL WELDING CODE - SHEET STEEL (AWS D1.3)". TEEL CONFORMING TO ASTM A653 - MINIMUM GRADE 40 (Fy = 40 KSI) TING CLASS G60. COMPOSITE STEEL FLOOR DECK TYPE, DEPTH, AND	F2.	THE OWN DESCRIB THE CON	IES, IN IER ANI ED ON TRACT
ON THE DRAWINGS. MADE FROM STEEL CONFOR 53, COATING CLASS G60. STE	MING TO ASTM A653 - GRADE 33 (Fy = 33 KSI) AND GALVANIZED IN EEL ROOF DECK TYPE, DEPTH, AND GAUGE SHALL BE AS NOTED ON THE	F3.	SPECIFIE THE CON INTERFEI ABANDOI	D LOCA TRACTO RE WITH
R STOP ANGLES WITH RETURI GE.	N LIP AT BUILDING PERIMETER AND AT ALL INTERIOR OPENINGS. REFER	F4.	THE BOT	TOM SU
ECK SHALL BE ATTACHED TO	THE SUPPORTING STRUCTURE WITH THE FOLLOWING MINIMUM		BEARING UNCOMP.	PRESS ACTED
D LAPS ORTS S AT SIDE SUPPORTS	5/8"Ø PUDDLE WELD AT 12" o.c. MAX. 5/8"Ø PUDDLE WELD AT 12" o.c. MAX. 5/8"Ø PUDDLE WELD AT 12" o.c. MAX.	F5.	THE ESTI EXTERIO	MATED R FOOT
ENT UNITS ROOF DECK SHALL BE ATTAC	BUTTON PUNCHED AT 24" o.c. MAX. HED TO THE SUPPORTING STRUCTURE WITH THE FOLLOWING MINIMUM	F6.	PROVIDE ON GRAD	A VAPO E.
D LAPS ORTS	5/8"Ø PUDDLE WELD AT EACH RIB 5/8"Ø PUDDLE WELD AT EACH RIB	F7.	BACKFILL PERCEN1 EARTHWO	. AGAIN <sup>-</sup> OF MA ORK SF
S AT SIDE SUPPORTS ENT UNITS	5/8"Ø PUDDLE WELD AT 12" o.c. MAX. 1-1/2" SEAM WELD AT 24" o.c. MAX.	F8.	VERIFY L RESPECT	OCATIO
ARCHITECTURAL COMPONE NT CONTRACTOR TO SPAN BE	NTS FROM METAL ROOF DECK. PROVIDE ENGINEERED SUPPLEMENTAL TWEEN STRUCTURAL BEAMS.	F9.	FOUNDAT	TIONS S
00)		F10.	REINFOR	
ES FOR WOOD CONSTRUCTION	ON" LATEST EDITION. MAXIMUM MOISTURE CONTENT SHALL BE	F11.	SLABS FF	RETE S ROM FR
RUCE-PINE-FIR NO. 2 OR BETT	ER, INCLUDING JOISTS, RAFTERS, BEAMS, STUDS, POSTS AND	112.	THE TOP	AND BO
E PRESERVATIVE PRESSURE REATED WOOD SHALL BE HO STEEL.	TREATED SOUTHERN PINE NO. 2 OR BETTER. ANCHOR BOLTS IN T DIP GALVANIZED. NAILS IN PRESERVATIVE PRESSURE TREATED	<u>A</u>	BREVIATIO	<u>NS</u>
ER SHALL BE PRESERVATIVE I REATED WOOD SHALL BE HO STEEL.	PRESSURE TREATED SOUTHERN PINE NO. 2 OR BETTER. BOLTS IN T DIP GALVANIZED. NAILS IN PRESERVATIVE PRESSURE TREATED	AD AE AF AL	DL SS F T	ADDI ARCH ABOV ALTE
? (LVL) SHALL BE MICRO-LAM, (PSL) SHALL BE PARALLAM, AS	AS MANUFACTURED BY WEYERHAEUSER, OR EQUAL.	AC AF AIS	I PA SC	AMEF AMEF AMEF
L BE TJI JOISTS, AS MANUFAC	CTURED BY WEYERHAEUSER, OR EQUAL. PROVIDE WEB	AIS AIT AS	SI TC TM	AMEF AMEF AMEF
NS SHALL HAVE METAL BEAM	OR JOIST HANGERS, MANUFACTURED BY SIMPSON STRONG-TIE	АЙ АМ & (	/S IC DR +	AMEF ANCH AND
L HAVE METAL CAPS AND BAS	ES, MANUFACTURED BY SIMPSON STRONG-TIE CO., INC., OR	AP AR @	PROX CH	APPR ARCH AT RA
A MINIMUM OF 5/8" NOMINAL ( EATHING WITH 10d NAILS 6" d	(19/32" ACTUAL) EXTERIOR GRADE (EXPOSURE 1) APA RATED b.c. AT EDGES AND 12" o.c. AT INTERMEDIATE SUPPORTS.	BA BL	L K	BALA BLOC
PANEL EDGES. A MINIMUM OF 1/2" NOMINAL ( IEATHING WITH 8d NAILS 6" o.	15/32" ACTUAL) EXTERIOR GRADE (EXPOSURE 1) APA RATED c. AT EDGES AND 12" o.c. AT INTERMEDIATE SUPPORTS.	BN BR B/S B (	I G S DR BOT	BEAM BEAR BOTH BOTT
OOD WALL SHEATHING. E A MINIMUM OF 3/4" NOMINAL ONGUE AND GROOVE, GLUED	(23/32" ACTUAL) EXTERIOR GRADE (EXPOSURE 1) APA RATED AND NAILED WITH 10d NAILS AT 6" o.c. AT ENDS	BE BE BL BC	W TWN W DCA	BOTT BETW BOTT BUILT
E SUFFORTS. GERED JOINTS AND NAILS SH RAIN PERPENDICULAR TO TH	HALL BE THREADED. ALL PLYWOOD SHEATHING SHALL BE IE SUPPORTS.	BP BR BS	KT MT	BASE BRAC BASE
DANCE WITH THE 2015 INTERI	NATIONAL BUILDING CODE, TABLE 2304.10.1, UNLESS OTHERWISE	BS CG CH	W ; 5	BOTT CENT CAST
DGED AT 8'-0" o.c. MAX.		CJ CT CI	J OR€	CONS CONT CENT
LOCKED AT 4'-0" o.c. MAX. ANI ETWEEN JOISTS AND RAFTER	D AT ALL PLYWOOD EDGES. S OVER ALL STUD BEARING WALLS OR SUPPORTING BEAMS.	CL CL CN	G R NU	CEILI
CALLY) BLOCKED THROUGH F ORTING BEAM.	LOOR CONSTRUCTION AT ALL LEVELS, TO THE TOP OF		) )L )MP	CLEA COLU COM
AS REQUIRED BY TABLES 23 NOTED. WALL STUDS AT JAMBS OF AL	08.4.1.1(1) AND 2308.4.1.1(2) OF THE MASSACHUSETTS STATE BUILDING		NNC NNN NNSTR NT	CON CON CONS CON
IENTS ARE NOTED ON THE DI STUDS KING STUDS 1 1	RAWINGS. TOTAL STUDS	CR DL	ISI ISI	CONT
1 2 2 2 2 3	- 3 4 5	DE DE DE	MO PR T	DEMO DEPR DETA
ACK STUDS SHALL BE OF THE ON THE DRAWINGS. ALL TRIM IACK STUDS SHALL TERMINAT S SHOWN IN THE TABLE ABOV MULTIPLE OPENINGS IMMEDIA OPENINGS DOES NOT ACCOM STUD(S) AND CONNECT THE I D FLANGES).	E SAME MATERIAL AS THE TYPICAL WALL STUDS, UNLESS MERS SHALL BE FULL HEIGHT, EXTENDING FROM THE SOLE PLATE TE BELOW THE HEADER AND BE THOROUGHLY SPIKED TO THE E IS FOR ONE OPENING - PROVIDE 2X THE NUMBER OF JACK STUDS ATELY ADJACENT TO EACH OTHER. IN CASES WHERE THE IMODATE THE TOTAL NUMBER OF JACK STUDS AND TRIMMERS, HEADER TO THE TRIMMERS WITH A STANDARD METAL JOIST	DE DIA D DI DII DII DN DV DV	IV LNGTH AG A OR Ø M R R VL VG	DEVE DIAG DIAM DIAM DIME DIRE DOW DOW DOW
X CORNER POSTS AT ALL COP	RNERS AND WALL INTERSECTIONS.	EA EE EF		EACH EACH EACH
ANCHORS AT ALL ROOF RAF	TERS AND ROOF TRUSSES TO PLATE CONNECTIONS.	ES EN EL	/	EACH EACH ELEV
OOR AND ROOF CONSTRUCTIO IING) WITH METAL JOIST/BEAN	DN WITH MINIMUM OF 2 - 2X HEADERS AND TRIMMERS (DEPTH TO A HANGERS, UNLESS OTHERWISE NOTED.	EL EN EG	EV IB	ELEV. EMBE EQUA
S, STUDS OR PLATES SHALL N	IOT BE PERMITTED.	EQ EQ ET	UIP UIV C	EQUII EQUI ETCE
RUSSES (SECTION 061753)	TE INSTITUTE "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE	EX EX EX	IST OR (E) P BLT P JT E	EXIST EXPA XPAN
ST LATEST EDITION AND SHAL CTIONS SHALL BE LIMITED AS < L/480	L BE DESIGNED FOR THE LOADINGS INDICATED ON THE DRAWINGS. S FOLLOWS (UNLESS OTHERWISE INDICATED ON THE DRAWINGS):	EX FD FF FP FS	Τ	EXTE FLOO FAR F FULL FAR S
AWINGS (ERECTION PLANS, N RACING, ETC.) AND CALCULA PREPARED BY AND BEAR THE	MEMBER DETAILS, MEMBER CONNECTIONS, CONNECTIONS BETWEEN ATIONS FOR ALL TRUSSES AND CONNECTIONS. ALL CALCULATIONS AND STAMP OF A PROFESSIONAL ENGINEER, REGISTERED IN THE	FT FV FIN FIN FP	l I FLR RF	FOOT FIELD FINIS
AND UNBALANCED SNOW LO	DADS AND WIND LOADS LISTED UNDER STRUCTURAL LOADS (THIS SHEET), G DEAD AND LIVE LOADS APPLIED TO THE TOP CHORD (TC) AND BOTTOM	FLI FN FT	R D G	FLOO FOUN FOOT
TC), 20 PSF (BC) (TC). 10 PSF (BC)		GA GE	3	GAGE GRAD
MBÍNATIONS FÓR ALL TRUSS IS SHOWN ON THE TRUSS EL	CALCULATIONS. EVATIONS ARE SCHEMATIC. FINAL WEB CONFIGURATIONS SHALL BE	GC GC GE	EN S	GENE
DESIGNER / FABRICATOR. CC THE DRAWINGS. FIELD MODIF MITTED.	ORDINATE WEB MEMBER LOCATIONS WITH M.E.P. AND ACCESS ICATION OF TRUSSES TO FACILITATE THE INSTALLATION OF MECHANICAL	Gr H(	DR HOR	HORI
L BE AS SHOWN ON THE TRU IBERS HAVING A LENGTH LES QUIRE LATERAL RESTRAINT C	SS ELEVATIONS ON DRAWING S304. MINIMUM WEB MEMBER SIZES SHALL SS THAN 8'-0" (FROM WORKING POINT TO WORKING POINT) SO THAT THE DR BRACING.	HIF HC HF HS	- )F :::::::::::::::::::::::::::::::::::	HORIZ HORIZ HIGH HIGH
WEB MEMBERS SHALL BE SO	UTHERN PINE NO. 2 OR BETTER. ALL LUMBER SHALL BE 19% MAXIMUM	HT HV	AC	HEIGI HEAT
URELY BRACED BOTH DURING RAL DRAWINGS, THE SHOP D IS DIRECTED IN THE FIELD BY	GERECTION AND AS REQUIRED AFTER PERMANENT INSTALLATION AS RAWINGS, BY THE TRUSS PLATE INSTITUTE SPECIFICATIONS , TPL/WTCA THE ARCHITECT/STRUCTURAL ENGINEER.	IN INC INF S ID	OR " CL =0	INCH INCLU INFOI INSID
AYOUT (SEE NOTE #4) AND PR	OVIDE CLB AS REQUIRED AND SHOWN ON THE TRUSS SHOPS	IF INS F IN	SUL T	INSID INSUI INTEF
MAX, PER BCSI 2013 AND THE MAX, PER BCSI 2013 AND THE MBERS REQUIRING CLB AS SI	TYPICAL DETAILS ON THE CONTRACT DRAWINGS. SEE TYPICAL DETAIL ON TYPICAL DETAILS ON THE CONTRACT DRAWINGS. SEE TYPICAL DETAIL ON HOWN ON THE TRUSS SHOPS SHALL RECIEVE ARROWHEAD OR 'X'	IN JS JT	/ T	INVEI JOIST JOINT
ING REQUIREMENTS: TRUCTURAL DRAWINGS AND MENT. E CONTINUOUS LATERAL BRA PSUM SHEATHING IS APPI IFI	SHOP DRAWINGS FOR UNSHEATHED PORTIONS OF TOP CHORD. USE THE ACING OF THE BOTTOM CHORDS OF ALL TRUSSES WITH 2x4 (MIN.) AT D DIRECTLY TO THE BOTTOM CHORD. BOTTOM CHORD CONTINUOUS	K KS KC	1	KIP (1 KIPS   KNOC
MITTED. SHOP DRAWINGS TO IDENTIF	Y WEB MEMBERS THAT REQUIRE BRACING.			

S HOLDDOWN CONNECTORS (OR EQUAL) AT ALL ROOF TRUSS BEARINGS (INTERIOR AND EXTERIOR ESIGNER SHALL SELECT AND SPECIFY ON THE TRUSS ERECTION DRAWINGS A HOLDDOWN THAT PROVIDED IN THE TRUSS SHOP DRAWINGS.

## FOUNDATION (SECTION 310000)

- ORK SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING REPORT BY MCARDLE GANNON NC., FILE # W0890, DATED DECEMBER 28, 2021. ND ARCHITECT ASSUME NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS
- N THE DRAWINGS, SPECIFICATIONS, BORING LOGS, OR TEST PITS. THIS DATA IS INCLUDED ONLY TO ASSIST TOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND REPRESENT CONDITIONS ONLY OF THOSE CATIONS AT THE PARTICULAR TIME THEY WERE MADE.
- TOR SHALL INFORM THE ARCHITECT AND RELOCATE, AS REQUIRED, ANY EXISTING UTILITY LINES THAT MAY TH NEW FOUNDATIONS. THE CONTRACTOR SHALL REMOVE ANY EXISTING UTILITY LINES THAT ARE BEING I THE VICINITY OF THE NEW FOUNDATION AND BACKFILL THE AREA WITH COMPACTED STRUCTURAL FILL. SURFACE OF ALL SPREAD FOOTINGS SHALL REST ON A 3" THICK LAYER OF COMPACTED 3/4" CRUSHED
- INDISTURBED GRANULAR SOIL DEPOSITS OR COMPACTED STRUCTURAL FILL, WITH A MINIMUM ALLOWABLE SSURE OF 2.5 TONS PER SQUARE FOOT. REMOVE ALL ORGANICS, CLAYS, SILTS, OR UNSUITABLE OR D FILL MATERIALS FROM BENEATH NEW FOOTINGS AND REPLACE WITH COMPACTED STRUCTURAL FILL.
- D BOTTOM ELEVATION OF EACH FOOTING IS INDICATED THUS [X'-X"] ON PLAN. THE BOTTOM OF EACH DTING SHALL BE A MINIMUM OF 4'-0" BELOW ADJACENT EXTERIOR FINISH GRADE.
- POR BARRIER AND AN 8" MINIMUM THICKNESS OF 3/4" CRUSHED STONE UNDER INTERIOR CONCRETE SLABS INST WALLS, MATS, AND FOOTINGS SHALL BE ENGINEERED BACKFILL COMPACTED IN SPECIFIED LIFTS TO 95
- AXIMUM DENSITY, UNLESS OTHERWISE INDICATED OR SPECIFIED. REFER TO GEOTECHNICAL REPORT AND PECIFICATION FOR ADDITIONAL REQUIREMENTS. IONS AND REQUIREMENTS FOR INSERTS, SLEEVES, CONDUITS, EMBEDMENTS, AND PENETRATIONS WITH
- RADES BEFORE PLACING CONCRETE.
- SHALL BE CENTERED UNDER SUPPORTED MEMBERS, UNLESS NOTED OTHERWISE. I FOUNDATIONS INTO PIERS, COLUMNS, BUTTRESSES, OR WALLS SHALL BE THE SAME SIZE AND NUMBER AS
- NT IN PIERS, COLUMNS, AND BUTTRESSES, OR WALLS ABOVE, UNLESS NOTED OTHERWISE.
- SHALL BE PLACED UNDER WATER OR ON FROZEN SUBGRADE. PROTECT IN-PLACE FOUNDATIONS AND ROST PENETRATION UNTIL PROJECT IS COMPLETED.
- FILL FOUNDATION WALLS UNTIL WALLS HAVE REACHED THEIR 28 DAY STRENGTHS AND FLOOR SLABS AT BOTTOM OF WALLS ARE IN PLACE AND CURED.

ADDL AESS	ADDITIONAL ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	L LBS	ANGLE POUNDS
AFF AI T	ABOVE FINISHED FLOOR AI TERNATE	LE	LEFT END LONG
ACI	AMERICAN CONCRETE INSTITUTE	LL	LIVE LOAD
AFPA	AMERICAN FOREST & PAPER ASSOCIATION	LLH	LONG LEG HORIZONTAL
AISI	AMERICAN IRON AND STEEL INSTITUTE	LOC	LOCATION
AITC ASTM	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION	LONG I P	LONGITUDINAL LOW POINT
AWS	AMERICAN WELDING SOCIETY	LRFD	LOAD AND RESISTANCE FAC
ANC & OR +	ANCHOR	LT GA I W	LIGHT GAGE, GAUGE
APPROX	APPROXIMATELY	LWT	LIGHTWEIGHT
ARCH @	ARCHITECT OR ARCHITECTURAL AT RATE OF	LWC	
BAL	BALANCE	MAS MANUF	MASONRY MANUFACTURER
BLK BM	BLOCK BEAM	MATL MAX	MATERIAL MAXIMUM
BRG	BEARING	MECH	MECHANICAL, ELECTRICAL, F
B/S B OR BOT	BOTH SIDES BOTTOM	MID	MIDDLE
BO	BOTTOM OF	MID-PT	MID-POINT
BEW BETWN	BOTTOM EACH WAY BETWEEN	MISC	MISCELLANEOUS
BLW	BOTTOM LONG WAY	MO MTI	MASONRY OPENING
BOCA BP	BUILDING OFFICIALS AND CODE ADMINISTRATORS BASE PLATE	MWR	MINIMUM WALL REINFORCIN
BRKT	BRACKET	(N)	NEW
BSW	BOTTOM SHORT WAY	NA NCMA	NOT APPLICABLE
CG	CENTER OF GRAVITY		ASSOCIATION
CIP C.I	CAST-IN-PLACE	NF NIC	NEAR FACE NOT IN CONTRACT
CTJ	CONTROL JOINT	NO OR #	NUMBER
CL OR ≌ CLG	CENTER LINE CEILING	NTS NWT	NOT TO SCALE NORMAL WEIGHT
	CLEAR, CLEARANCE	00	
СМU CO	CONCRETE MASONRY UNIT CLEANOUT	OD	OUTSIDE DIAMETER
COL	COLUMN	OF OH	OUTSIDE FACE
COMP CONC	COMPRESSION CONCRETE	OPNG	OPENING
CONN	CONNECTION	OPP OSHA	OPPOSITE OCCUPATIONAL SAFETY &
CONSTR CONT	CONSTRUCTION CONTINUE, CONTINUOUS	0.011/1	HEALTH ADMINISTRATION
CONTR		Pc	PIECE
JKSI	CONCRETE REINFORGING INSTITUTE	PC PCI	PILE CAP
JL DEMO	DEAD LOAD DEMOLISH. DEMOLITION	PEN	PENETRATION
DEPR	DEPRESS, DEPRESSION	PERIM ₽	PERIMETER PLATE
JET DEV LNGTH	DETAIL DEVELOPMENT LENGTH	P/T	POST TENSIONED
DIAG	DIAGONAL	PSI PSF	POUNDS PER SQUARE INCH POUNDS PER SQUARE FOOT
DA UK Ø D	DIAMETERS	PT	PRESSURE TREATED
DIM	DIMENSION	PVC	POLYVINYL CHLORIDE
DN	DOWN	R RD	RADIUS ROOF DRAIN
DWL	DOWEL	RE	RIGHT END
		REF REINF	REFERENCE REINFORCING
=A EE	EACH EACH END	REM	REMAINDER
EF	EACH FACE	RET REQD	RE I URN REQUIRED
=3 EW	EACH WAY	RETG	RETAINING
EL	ELEVATION ELEVATOR	REV	REVISION
EMB	EMBED, EMBEDMENT	S SC	SECTION MODULUS
EQ EQUUP	EQUAL	SDI	STEEL DECK INSTITUTE
EQUIV	EQUIVALENT	SECT SF	SECTION SQUARF FOOT
ETC	ETCETERA	SIM	SIMILAR
EXP BLT	EXPANSION BOLT	SJI SL	STEEL JOIST INSTITUTE SPLICE LENGTH
EXP JT E EXT	XPANSION JOINT EXTERIOR	SOG	SLAB ON GRADE
=D	FLOOR DRAIN	SP SPECS	SPACES SPECIFICATIONS
=F		SQ	SQUARE
- <i>P</i> =S	FULL PENETRATION WELD FAR SIDE	SS STIFF	STAINLESS STEEL STIFFENER
=T	FOOT OR FEET	STL	STEEL
-v =IN	FILLD VERIET FINISH	SYMM	SYMMETRICAL
IN FLR	FINISHED FLOOR	Т	ТОР
-LR	FLOOR	TB T I	TIE BEAM
=ND =TG	FOUNDATION	T&B	TOP AND BOTTOM
7.0 7.4	GAGE OR GAUGE	T&G TEMP	TONGUE AND GROOVE TEMPER∆TURE
3B	GRADE BEAM	TEN	TENSION
GALV GC	HOT-DIPPED GALVANIZED GENERAL CONTRACTOR	TEW TEF	ΤΟΡ ΕΑCΗ WAY ΤΕΤΡΔΕΙ ΠΩΡΕΤΗΜΙ ΕΝΕ
GEN	GENERAL	THK	THICK, THICKNESS
GR	GRADE	THRD THRU	THREADED THROUGH
OR HOR	HORIZONTAL	TIM	TIMBER
HEF HIF	HORIZONTAL EACH FACE HORIZONTAL INSIDE FACE	TLW TOC	TOP LONG WAY TOP OF CONCRETE
HOF	HORIZONTAL OUTSIDE FACE	TOS	TOP OF STEEL
19 HS	HIGH POINT HIGH STRENGTH	TOW TRANS	TOP OF WALL TRANSVERSE
HSS	HOLLOW STRUCTURAL SECTION	TSW	TOP SHORT WAY
יי אר אר	HEIGH I HEATING, VENTILATION AND AIR CONDITIONING		IYPICAL
		UBC UJ	UNIFORM BUILDING CODE
N OR " NCL	INCH INCLUSIVE. INCLUDING	UNO	UNLESS NOTED OTHERWISE
NFO		V OR VERT	VERTICAL
D F	INSIDE DIAMETER INSIDE FACE	VEF	VERTICAL EACH FACE
NSUL	INSULATION	VIF VOF	VERTICAL INSIDE FACE
NT NV	INTERIOR INVERT	Λ//	
IST		W/O	WITHOUT
1.31	JOINT	WD WP	WOOD WORKING POINT
IT		**/	
IT (	KIP (1000 POUNDS)	WPG	WATERPROOFING
( ( (SI	KIP (1000 POUNDS) KIPS PER SQUARE INCH	WPG WS WT	WATERPROOFING WATER STOP WEIGHT
IT K KSI KO	KIP (1000 POUNDS) KIPS PER SQUARE INCH KNOCKOUT	WPG WS WT WWF	WATERPROOFING WATER STOP WEIGHT WELDED WIRE FABRIC







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REVISIONS
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