



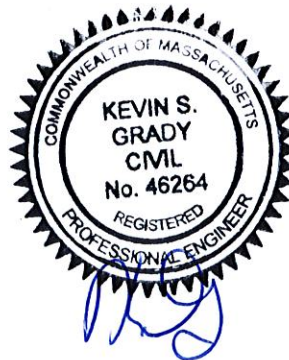
GRADY CONSULTING, L.L.C.

Registered Professional Civil Engineers & Land Surveyors

STORMWATER MANAGEMENT DESIGN CALCULATIONS

715 Washington Street
Assessors Map F9 Lots 24 & 40

Pembroke, Massachusetts



Prepared for

George Thibeault
599 Summer Street
Marshfield MA 02050

October 27, 2021
December 16, 2021

Table of Contents

Table of Contents		2
Summary		3
Peak Flow Summary		4
Overall Site Analysis	Section I	5
HydroCAD Report		6-97
Stormwater Compliance	Section II	98
Stormwater Checklist		99-106
Documenting Compliance		107-117
Recharge & Drawdown	107	
Mounding	108, 116, 117	
WQV	109	
Forebay sizing	109	
TSS Worksheets	101-104	
Standard 7-10	110	
Illicit Discharge Statement	110-111	
Operation and Maintenance Plan	Section III	118
During Construction		119-121
Post Construction		122-126
BMP Deep Sump Catch Basin		127-130
BMP Infiltration Basin		131-138
BMP Sediment Forebay		139-142
BMP Subsurface Structures		143-146
NRCS Soils Report		147-168
Soil Evaluation Forms		169-179
Testholes 2021-04-21		180-181
Stage Storage Basins and Forebays		172-175
Tributary Area Plans		Attached

Summary

This analysis was prepared to demonstrate Compliance with the Massachusetts Stormwater Management Regulations and the Town of Pembroke Planning Board Rules and Regulations for Stormwater Management. The proposed project is for the construction of one 10,000 square foot structure for office use.

The existing site is a vacant parcel located within the Town of Pembroke Residential-Commercial Zoning District. The proposed stormwater system consists of catch basins, sediment forebays, two infiltration basins and one subsurface infiltration system for the roof proposed building.

Pre-Development Stormwater flows are separated into catchments areas and routed as follows:

Pre 1 – Existing woodland that flows to an existing bordering vegetated wetland on the north western portion of the property.

Post-Development Stormwater flows are separated into 3 catchment areas. A proposed stormwater system consisting of one sediment forebay and an infiltration basin has been designed at an infiltration rate of 2.41 in/hr.

Post 4 – Pavement areas routed through catch basins, to infiltration basin #1.

Post 5 – Roof drains flow to the subsurface infiltration system#1.

Post 6 – Overland flow to infiltration basin #1.

Post 7 – Flows to Sediment forebays to infiltration basin#2

Post 8 – undisturbed woodland flows to an existing bordering vegetated wetland on the northern portion of the property

The design, as proposed, reduces peak runoff rates, improves and promotes infiltration, and improves stormwater quality and treatment prior to discharge.

This analysis is divided into the following sections:

Section I	Overall Site Analysis
Section II	Compliance with Massachusetts Storm water Management Regulations
Section III	Operation And Maintenance Plan

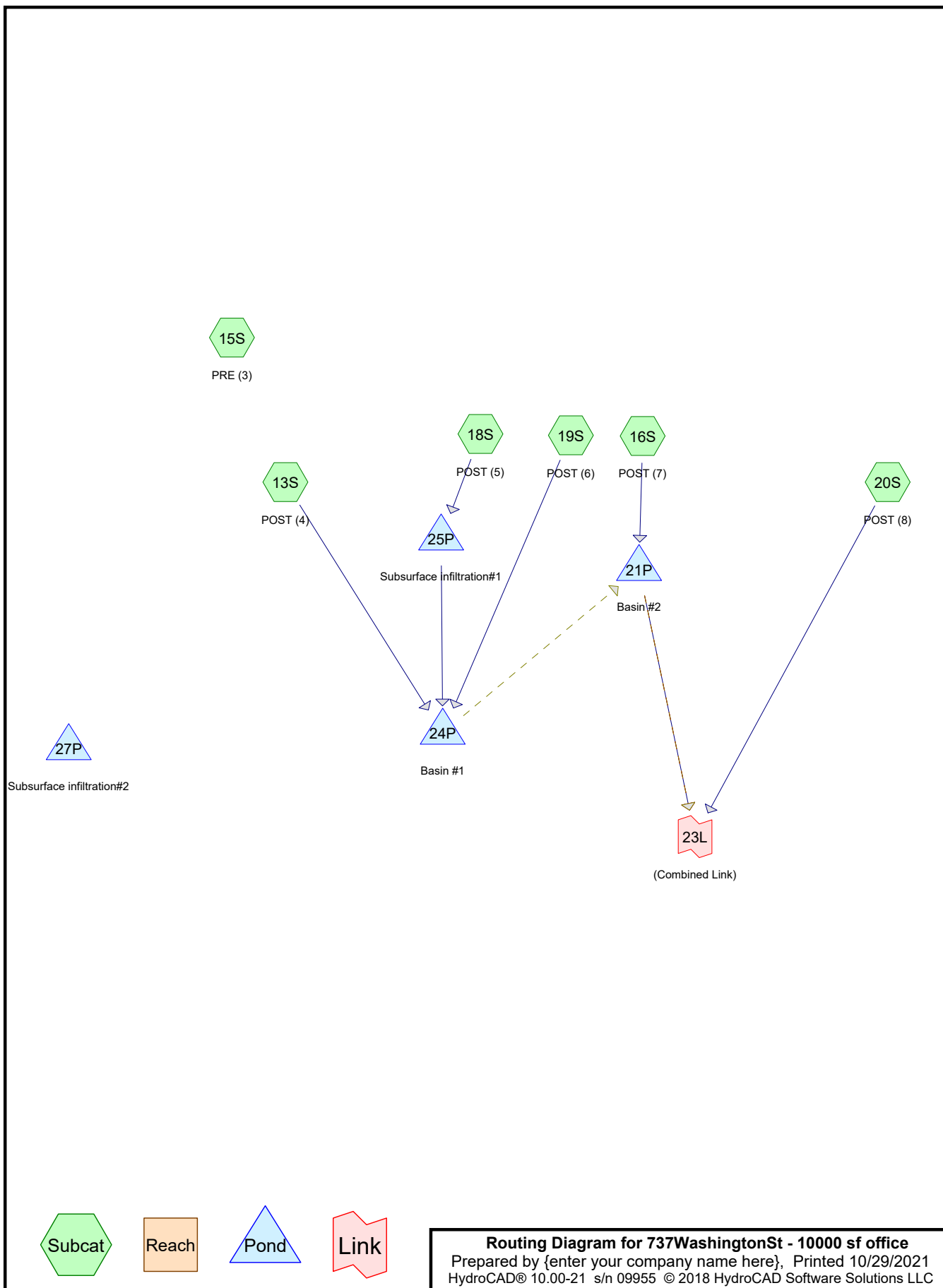
The calculations have been performed for the 2, 10, 25, and 100-year 24 hour storm event, using the HydroCAD 10.00 computer program. This computer program is based upon the Soils Conservation Service (SCS) TR-20 and TR-55 computer models and uses the SCS Curvilinear Unit rainfall distribution.

Peak Flow Summary

	2Year	10 Year	25 Year	100 Year		
Pre 1	1.98	4.05	5.64	8.26		
Total	1.98	4.05	5.64	8.26		
Pre 1 vs	1.98	4.05	5.64	8.26		
Post Combined Link	0.70	1.48	2.46	5.39		
Elevations	2Year	10 Year	25 Year	100 Year	Top	bottom
Basin #1	78.11	78.44	78.63	78.92	80.00	76.00
Basin#2	76.08	76.38	76.58	76.80	78.00	75.50
Subsurface system #1	83.17	83.48	83.73	84.05	84.5	79.00

Section I

Overall Site Analysis



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Page 2

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
41,072	74	>75% Grass cover, Good, HSG C (16S, 19S, 20S)
21,859	98	Paved parking, HSG C (13S, 16S)
10,000	98	Roofs, HSG C (18S)
129,601	70	Woods, Good, HSG C (15S, 20S)
202,532	75	TOTAL AREA

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Page 3

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
202,532	HSG C	13S, 15S, 16S, 18S, 19S, 20S
0	HSG D	
0	Other	
202,532		TOTAL AREA

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Page 4

Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Sub Num
0	0	41,072	0	0	41,072	>75% Grass cover, Good	
0	0	21,859	0	0	21,859	Paved parking	
0	0	10,000	0	0	10,000	Roofs	
0	0	129,601	0	0	129,601	Woods, Good	
0	0	202,532	0	0	202,532	TOTAL AREA	

737WashingtonSt - 10000 sf office*Type III 24-hr 2-Year Rainfall=3.40"*

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Page 5

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment13S: POST (4) Runoff Area=17,504 sf 100.00% Impervious Runoff Depth=3.17"
 Tc=6.0 min CN=98 Runoff=1.30 cfs 4,619 cf

Subcatchment15S: PRE (3) Runoff Area=97,629 sf 0.00% Impervious Runoff Depth=0.95"
 Flow Length=382' Tc=9.7 min CN=70 Runoff=1.98 cfs 7,704 cf

Subcatchment16S: POST (7) Runoff Area=13,121 sf 33.19% Impervious Runoff Depth=1.70"
 Tc=6.0 min CN=82 Runoff=0.59 cfs 1,859 cf

Subcatchment18S: POST (5) Runoff Area=10,000 sf 100.00% Impervious Runoff Depth=3.17"
 Tc=6.0 min CN=98 Runoff=0.74 cfs 2,639 cf

Subcatchment19S: POST (6) Runoff Area=30,906 sf 0.00% Impervious Runoff Depth=1.17"
 Flow Length=180' Tc=9.2 min CN=74 Runoff=0.82 cfs 3,017 cf

Subcatchment20S: POST (8) Runoff Area=33,372 sf 0.00% Impervious Runoff Depth=0.95"
 Flow Length=134' Tc=8.0 min CN=70 Runoff=0.70 cfs 2,633 cf

Pond 21P: Basin #2 Peak Elev=76.08' Storage=783 cf Inflow=0.59 cfs 2,497 cf
 Discarded=0.08 cfs 2,048 cf Primary=0.03 cfs 75 cf Tertiary=0.12 cfs 374 cf Outflow=0.23 cfs 2,497 cf

Pond 24P: Basin #1 Peak Elev=78.11' Storage=2,839 cf Inflow=2.05 cfs 7,794 cf
 Discarded=0.17 cfs 7,157 cf Tertiary=0.19 cfs 638 cf Outflow=0.36 cfs 7,794 cf

Pond 25P: Subsurface infiltration#1 Peak Elev=83.17' Storage=1,099 cf Inflow=0.74 cfs 2,639 cf
 Discarded=0.05 cfs 2,481 cf Primary=0.10 cfs 158 cf Outflow=0.15 cfs 2,639 cf

Pond 27P: Subsurface infiltration#2 Peak Elev=0.00' Storage=0 cf
 Discarded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Link 23L: (Combined Link) Inflow=0.70 cfs 3,082 cf
 Primary=0.70 cfs 3,082 cf

Total Runoff Area = 202,532 sf Runoff Volume = 22,472 cf Average Runoff Depth = 1.33"
84.27% Pervious = 170,673 sf 15.73% Impervious = 31,859 sf

Summary for Subcatchment 13S: POST (4)

Runoff = 1.30 cfs @ 12.09 hrs, Volume= 4,619 cf, Depth= 3.17"

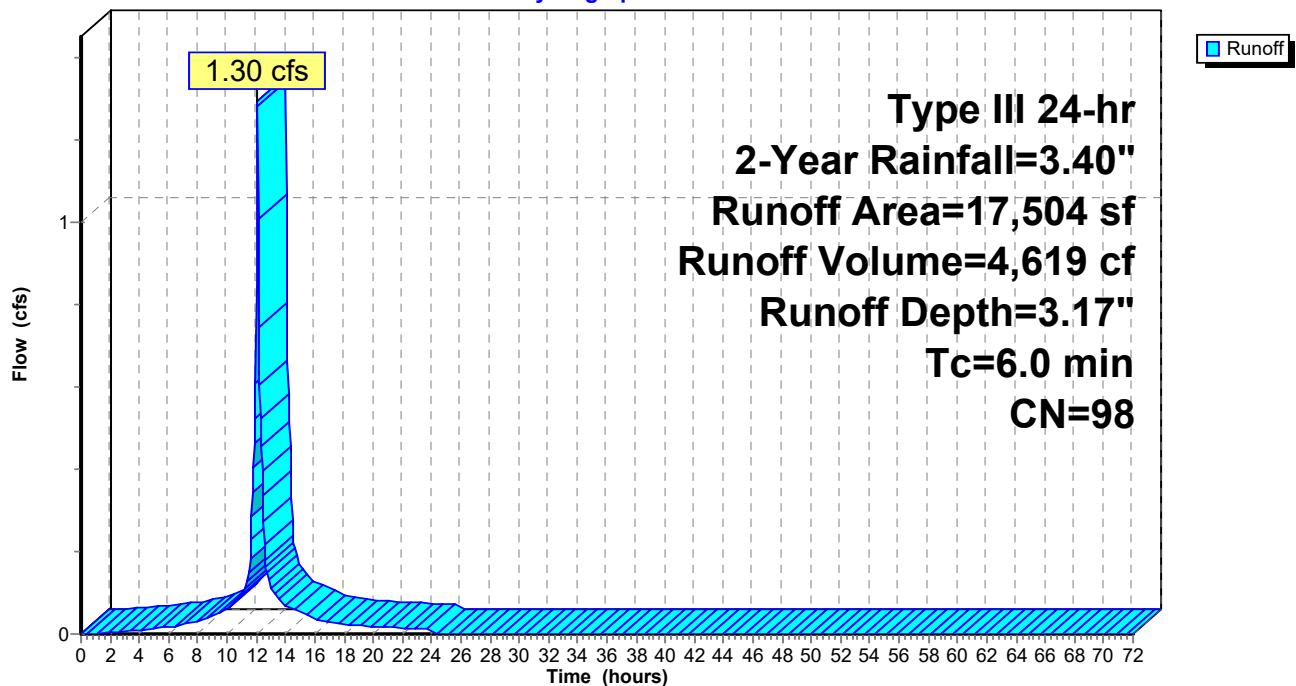
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.40"

Area (sf)	CN	Description
17,504	98	Paved parking, HSG C
17,504	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, runoff from pavement

Subcatchment 13S: POST (4)

Hydrograph



Summary for Subcatchment 15S: PRE (3)

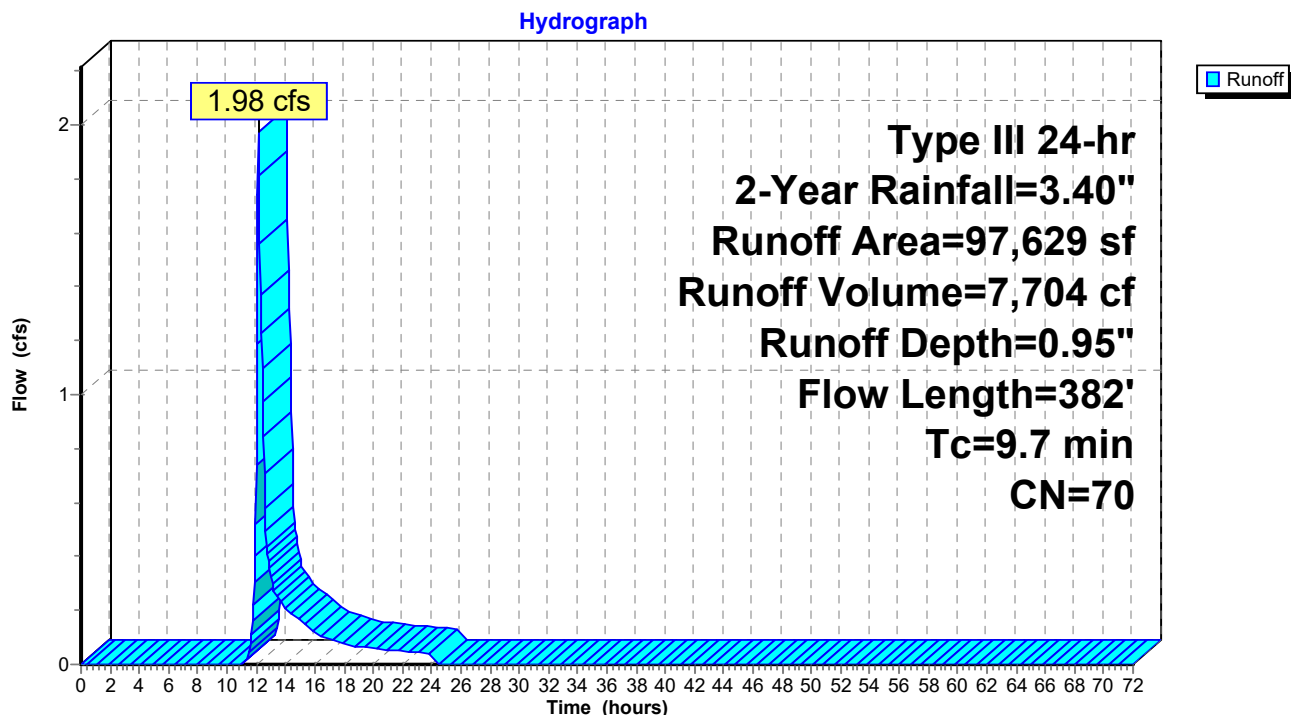
Runoff = 1.98 cfs @ 12.15 hrs, Volume= 7,704 cf, Depth= 0.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.40"

Area (sf)	CN	Description
97,629	70	Woods, Good, HSG C
97,629	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.2000	0.17		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
2.6	138	0.0320	0.89		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.8	146	0.0710	1.33		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.5	48	0.0850	1.46		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
9.7	382	Total			

Subcatchment 15S: PRE (3)



Summary for Subcatchment 16S: POST (7)

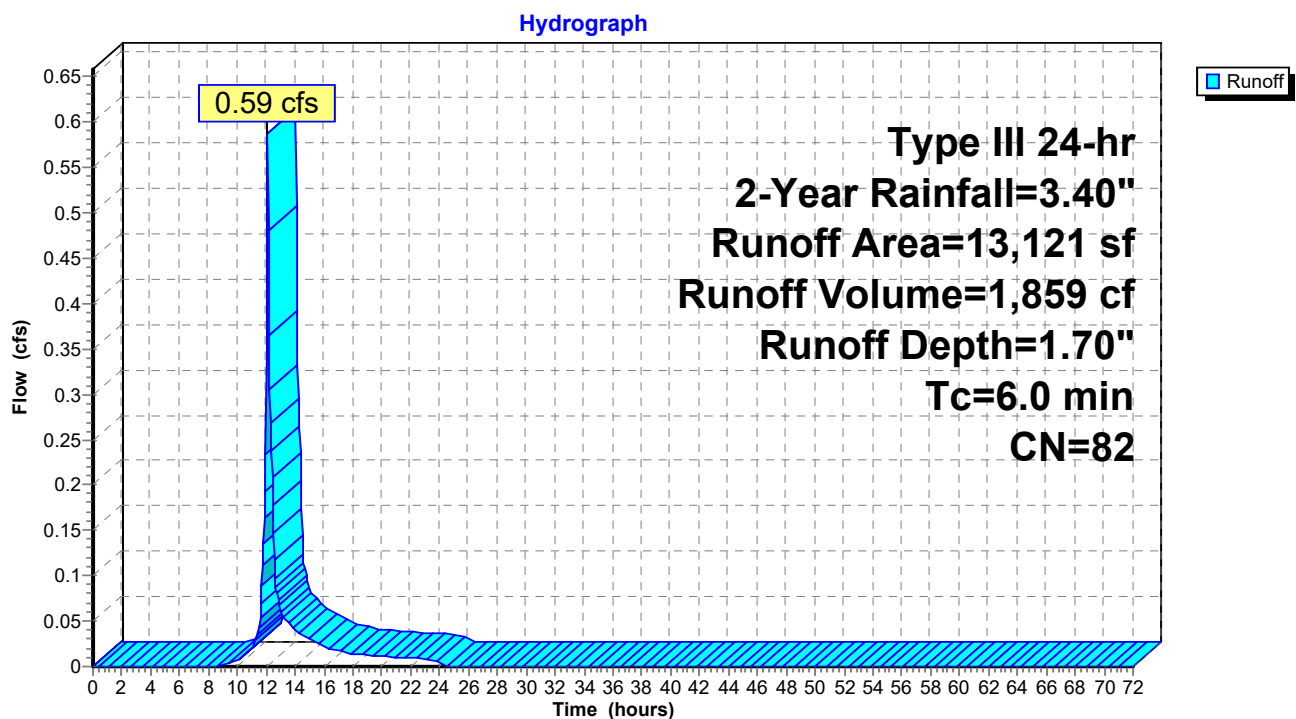
Runoff = 0.59 cfs @ 12.09 hrs, Volume= 1,859 cf, Depth= 1.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.40"

Area (sf)	CN	Description
8,766	74	>75% Grass cover, Good, HSG C
4,355	98	Paved parking, HSG C
13,121	82	Weighted Average
8,766	74	66.81% Pervious Area
4,355	98	33.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 16S: POST (7)



Summary for Subcatchment 18S: POST (5)

Runoff = 0.74 cfs @ 12.09 hrs, Volume= 2,639 cf, Depth= 3.17"

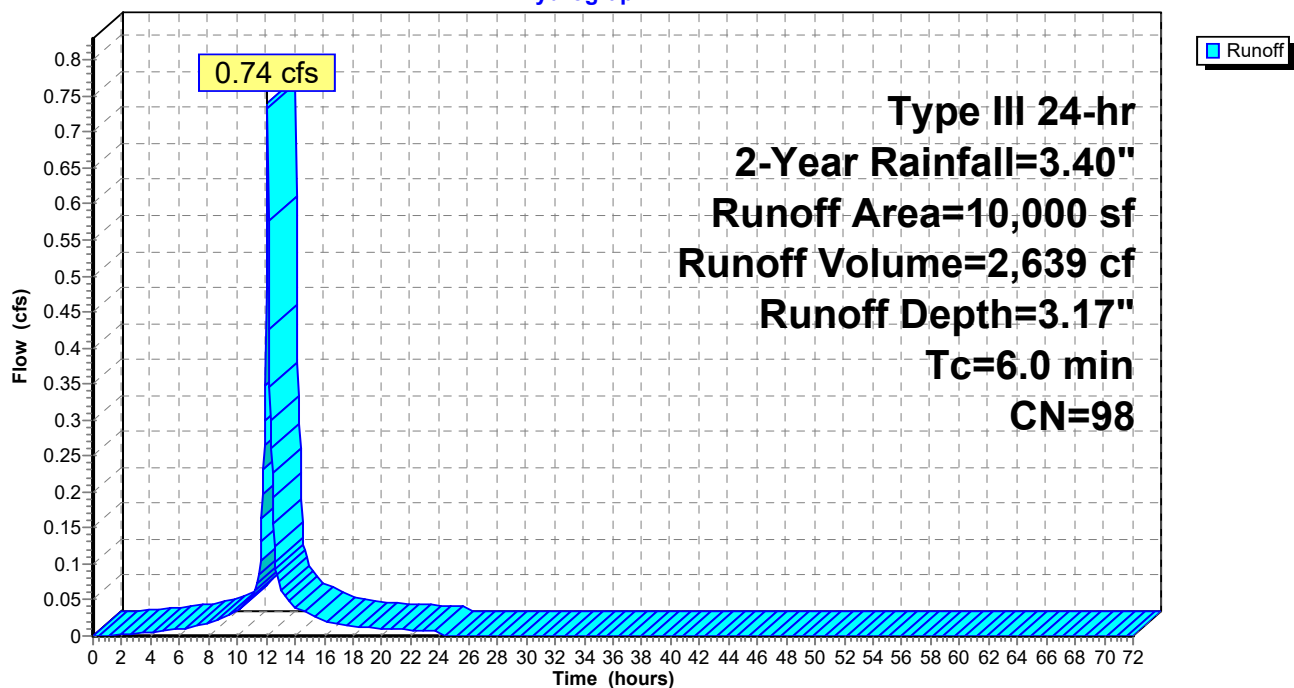
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.40"

Area (sf)	CN	Description
10,000	98	Roofs, HSG C
10,000	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 18S: POST (5)

Hydrograph



Summary for Subcatchment 19S: POST (6)

Runoff = 0.82 cfs @ 12.14 hrs, Volume= 3,017 cf, Depth= 1.17"

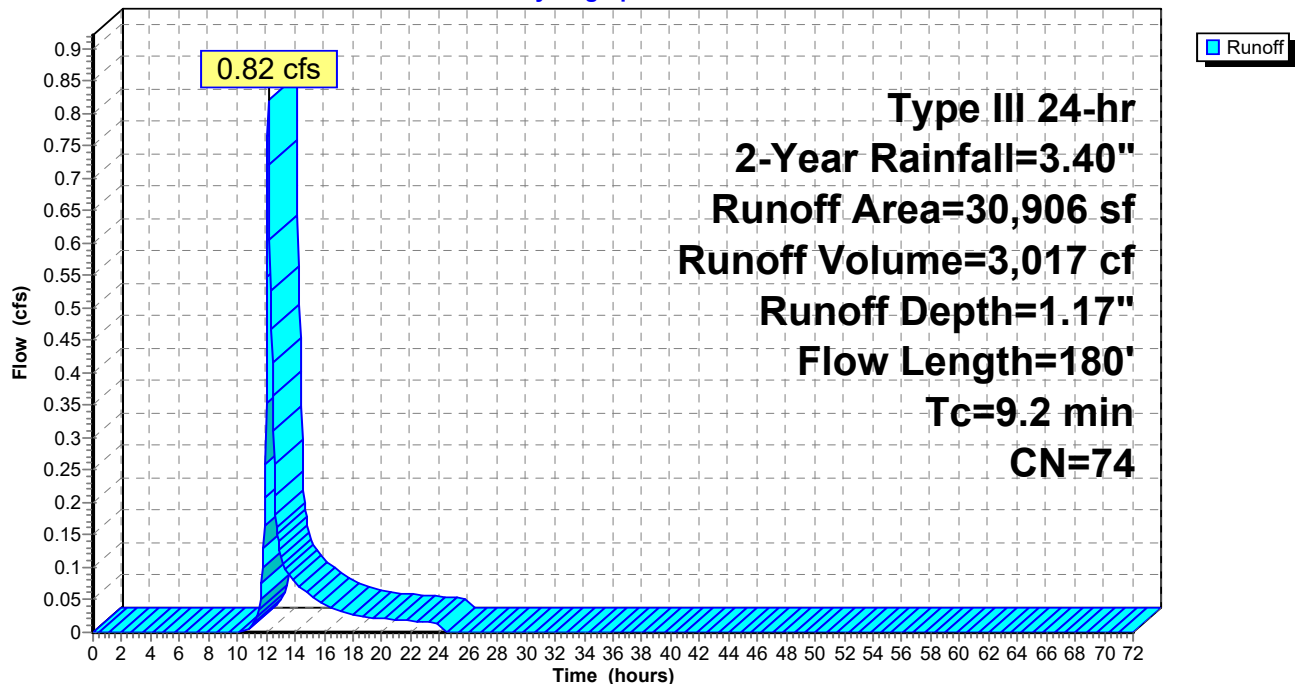
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.40"

Area (sf)	CN	Description
30,906	74	>75% Grass cover, Good, HSG C
30,906	74	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	50	0.0200	0.10		Sheet Flow, Grass: Dense n= 0.240 P2= 3.40"
0.9	93	0.0600	1.71		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.3	37	0.1200	2.42		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
9.2	180	Total			

Subcatchment 19S: POST (6)

Hydrograph



Summary for Subcatchment 20S: POST (8)

Runoff = 0.70 cfs @ 12.13 hrs, Volume= 2,633 cf, Depth= 0.95"

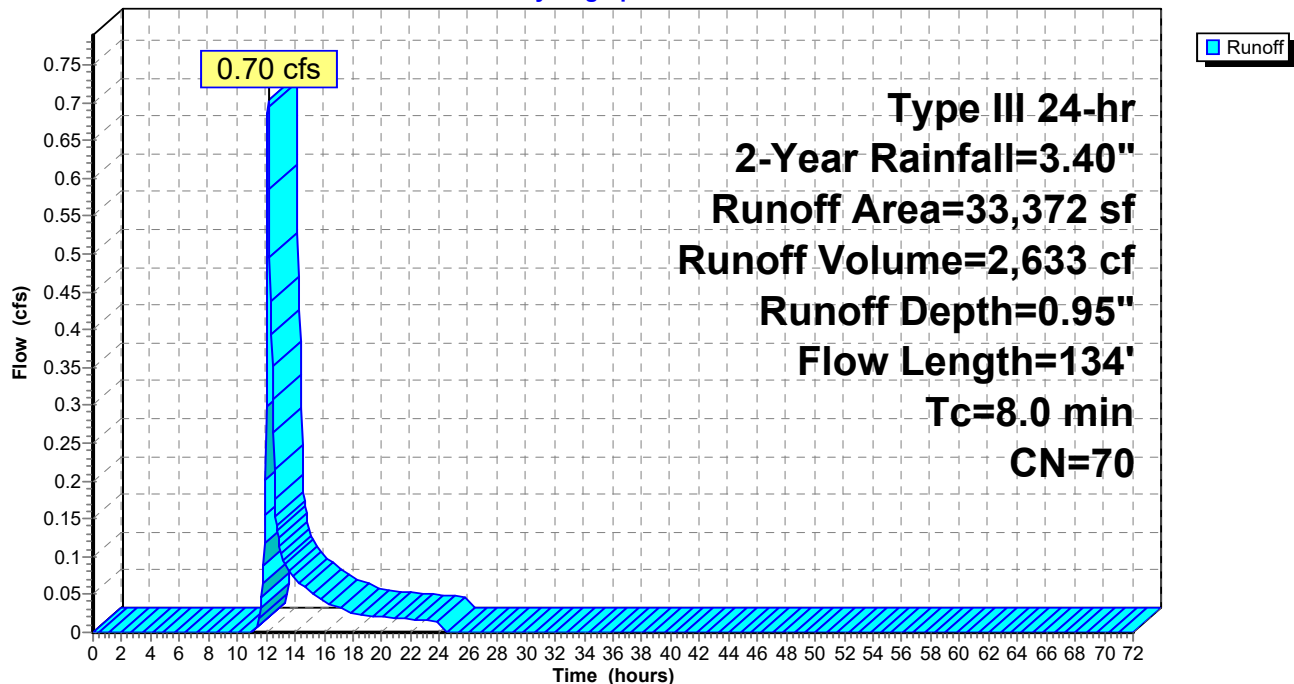
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-Year Rainfall=3.40"

Area (sf)	CN	Description
1,400	74	>75% Grass cover, Good, HSG C
31,972	70	Woods, Good, HSG C
33,372	70	Weighted Average
33,372	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0800	0.12		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
1.1	84	0.0700	1.32		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
8.0	134	Total			

Subcatchment 20S: POST (8)

Hydrograph



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Type III 24-hr 2-Year Rainfall=3.40"

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Page 12

Summary for Pond 21P: Basin #2

Inflow Area = 13,121 sf, 33.19% Impervious, Inflow Depth = 2.28" for 2-Year event
 Inflow = 0.59 cfs @ 12.09 hrs, Volume= 2,497 cf
 Outflow = 0.23 cfs @ 12.94 hrs, Volume= 2,497 cf, Atten= 61%, Lag= 50.5 min
 Discarded = 0.08 cfs @ 12.94 hrs, Volume= 2,048 cf
 Primary = 0.03 cfs @ 12.94 hrs, Volume= 75 cf
 Tertiary = 0.12 cfs @ 12.94 hrs, Volume= 374 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 76.08' @ 12.94 hrs Surf.Area= 1,475 sf Storage= 783 cf

Plug-Flow detention time= 67.8 min calculated for 2,495 cf (100% of inflow)
 Center-of-Mass det. time= 67.7 min (888.5 - 820.8)

Volume	Invert	Avail.Storage	Storage Description	
#1	75.50'	4,644 cf	stage (Conic) Listed below	
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
75.50	1,179	0	0	1,179
76.00	1,429	651	651	1,437
76.50	1,702	782	1,433	1,719
77.00	1,988	922	2,354	2,015
77.50	2,288	1,068	3,422	2,326
78.00	2,603	1,222	4,644	2,653

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	76.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height
#3	Primary	76.00'	9.0" Vert. Orifice/Grate C= 0.600

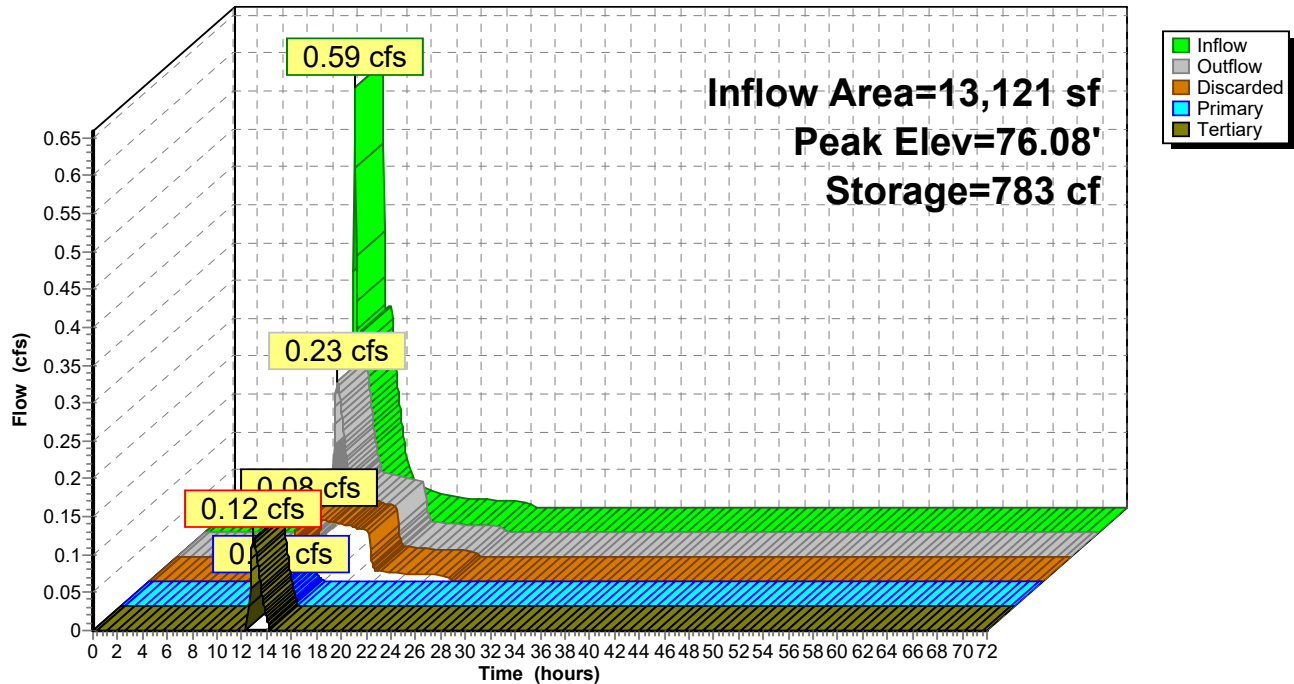
Discarded OutFlow Max=0.08 cfs @ 12.94 hrs HW=76.08' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.03 cfs @ 12.94 hrs HW=76.08' (Free Discharge)
 ↑ **3=Orifice/Grate** (Orifice Controls 0.03 cfs @ 0.99 fps)

Tertiary OutFlow Max=0.12 cfs @ 12.94 hrs HW=76.08' (Free Discharge)
 ↑ **2=Sharp-Crested Rectangular Weir** (Weir Controls 0.12 cfs @ 0.96 fps)

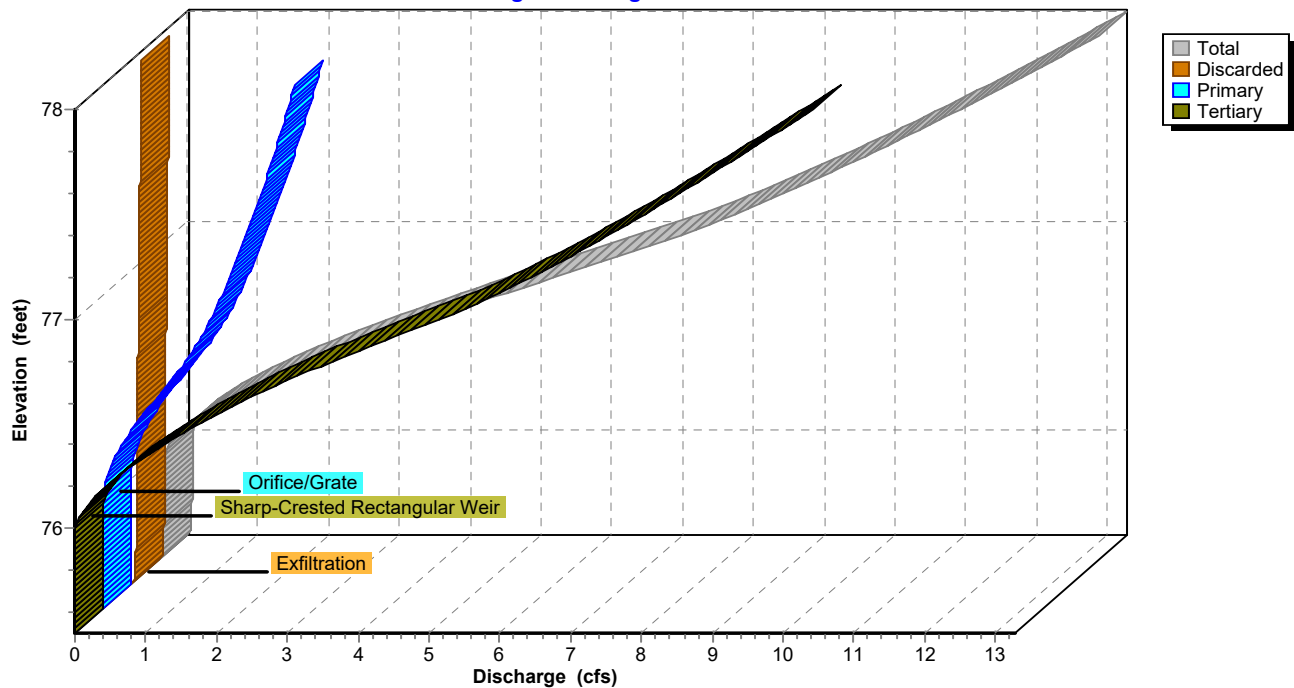
Pond 21P: Basin #2

Hydrograph

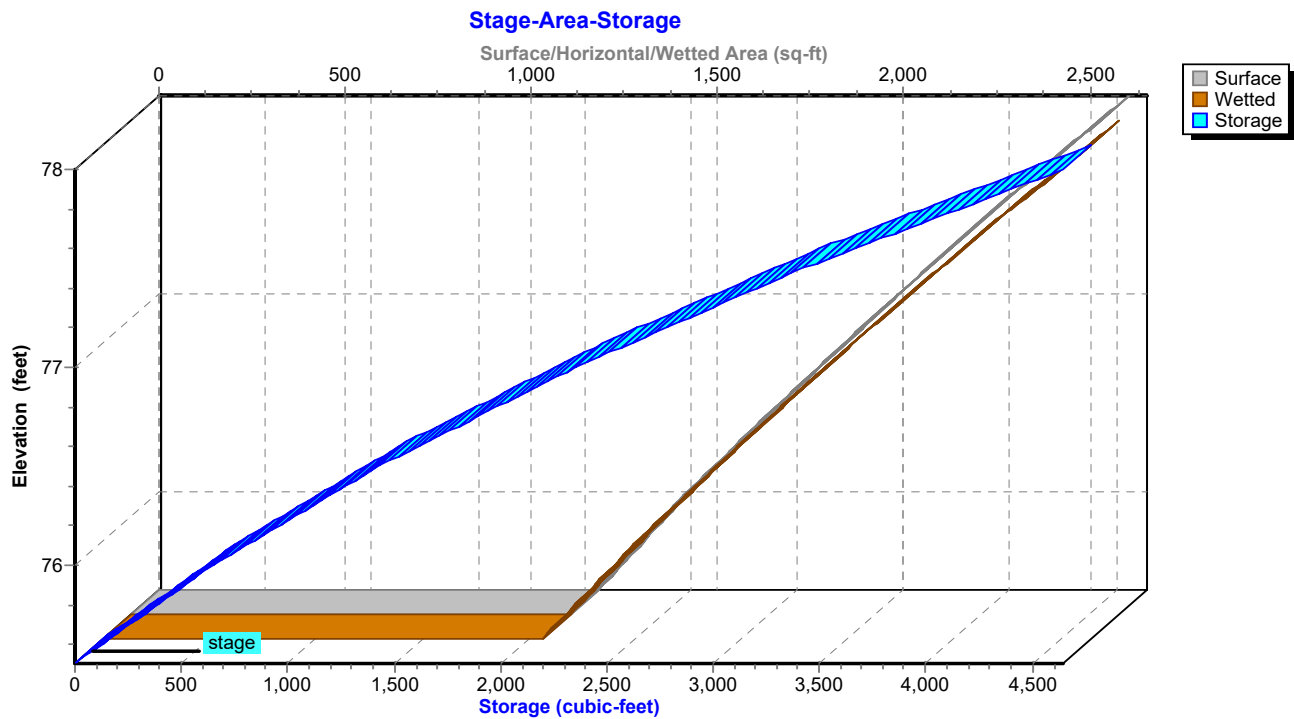


Pond 21P: Basin #2

Stage-Discharge



Pond 21P: Basin #2



Summary for Pond 24P: Basin #1

Inflow Area = 58,410 sf, 47.09% Impervious, Inflow Depth = 1.60" for 2-Year event
 Inflow = 2.05 cfs @ 12.10 hrs, Volume= 7,794 cf
 Outflow = 0.36 cfs @ 12.71 hrs, Volume= 7,794 cf, Atten= 82%, Lag= 36.6 min
 Discarded = 0.17 cfs @ 12.71 hrs, Volume= 7,157 cf
 Tertiary = 0.19 cfs @ 12.71 hrs, Volume= 638 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 78.11' @ 12.71 hrs Surf.Area= 3,109 sf Storage= 2,839 cf

Plug-Flow detention time= 125.8 min calculated for 7,794 cf (100% of inflow)
 Center-of-Mass det. time= 125.7 min (922.2 - 796.4)

Volume	Invert	Avail.Storage	Storage Description
#1	77.10'	9,844 cf	stage (Conic) Listed below

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
77.10	2,446	0	0	2,446
78.00	3,036	2,462	2,462	3,059
79.00	3,682	3,354	5,816	3,737
80.00	4,385	4,028	9,844	4,475

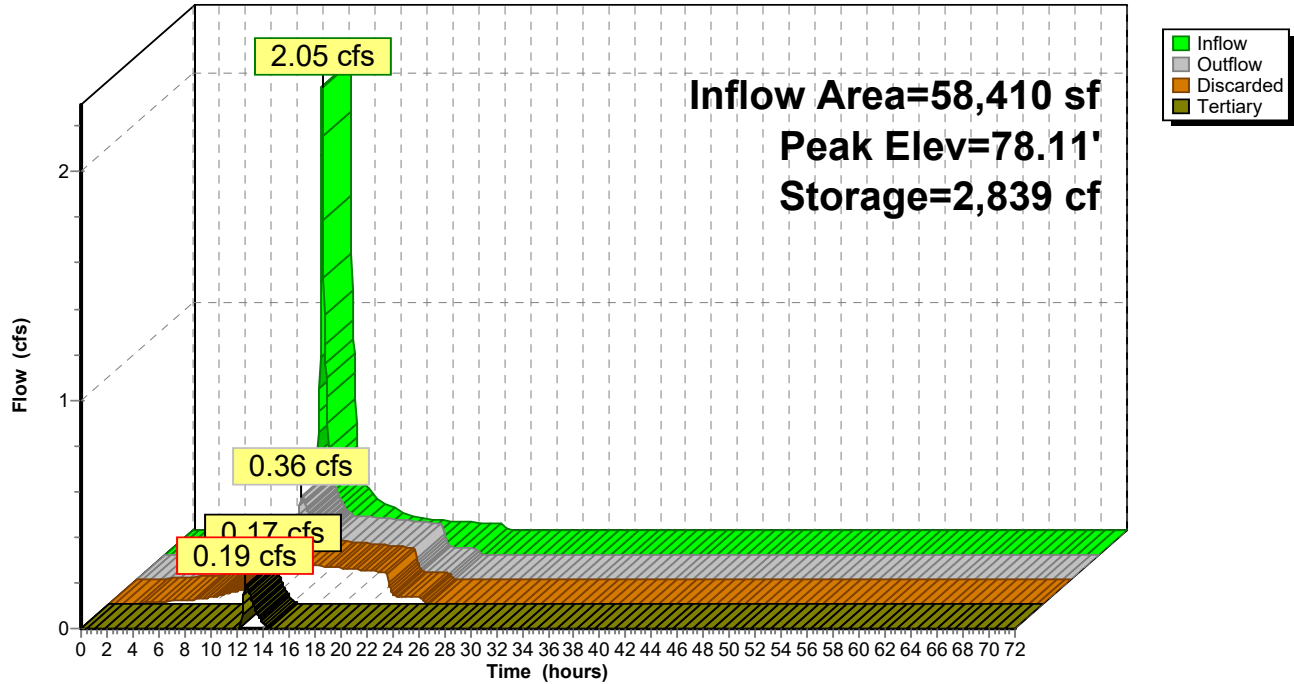
Device	Routing	Invert	Outlet Devices
#1	Discarded	77.10'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	78.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height

Discarded OutFlow Max=0.17 cfs @ 12.71 hrs HW=78.11' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.17 cfs)

Tertiary OutFlow Max=0.19 cfs @ 12.71 hrs HW=78.11' (Free Discharge)
 ↑ **2=Sharp-Crested Rectangular Weir** (Weir Controls 0.19 cfs @ 1.11 fps)

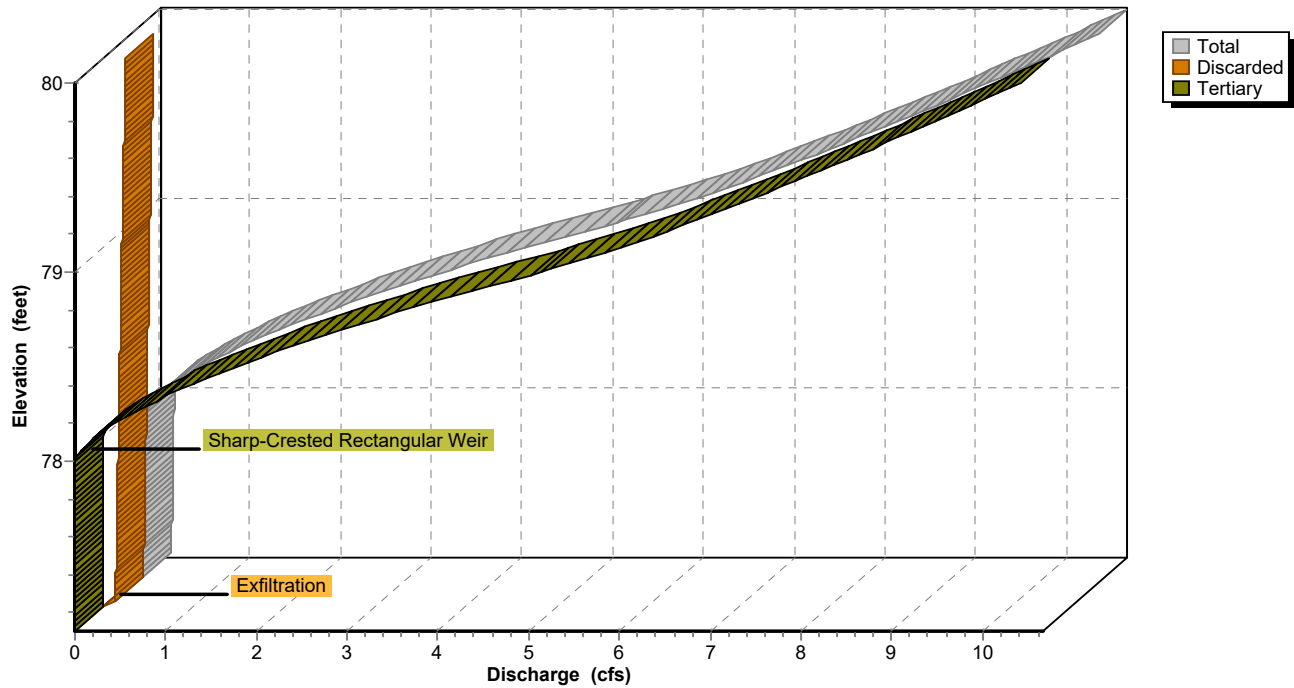
Pond 24P: Basin #1

Hydrograph



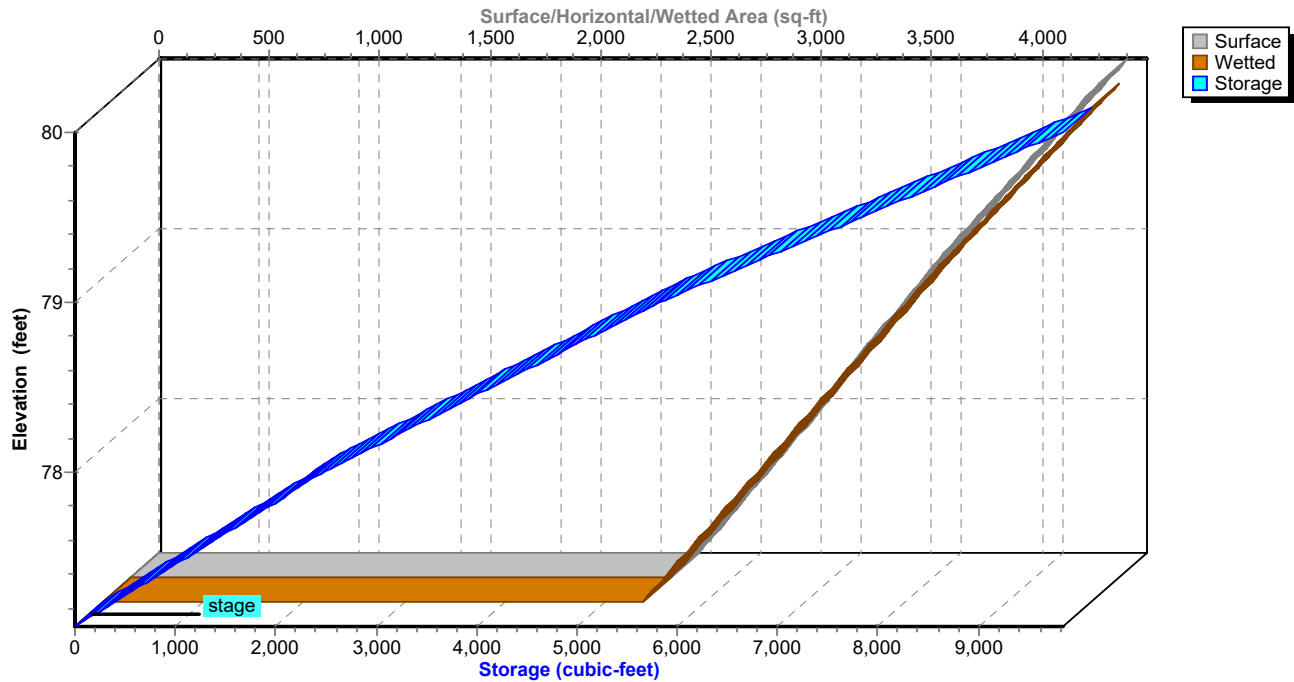
Pond 24P: Basin #1

Stage-Discharge



Pond 24P: Basin #1

Stage-Area-Storage



Summary for Pond 25P: Subsurface infiltration#1

Inflow Area = 10,000 sf, 100.00% Impervious, Inflow Depth = 3.17" for 2-Year event
 Inflow = 0.74 cfs @ 12.09 hrs, Volume= 2,639 cf
 Outflow = 0.15 cfs @ 12.52 hrs, Volume= 2,639 cf, Atten= 79%, Lag= 25.7 min
 Discarded = 0.05 cfs @ 12.52 hrs, Volume= 2,481 cf
 Primary = 0.10 cfs @ 12.52 hrs, Volume= 158 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 83.17' @ 12.52 hrs Surf.Area= 461 sf Storage= 1,099 cf

Plug-Flow detention time= 200.4 min calculated for 2,637 cf (100% of inflow)
 Center-of-Mass det. time= 200.4 min (955.5 - 755.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	79.00'	637 cf	12.80'W x 36.00'L x 5.50'H Field A 2,534 cf Overall - 942 cf Embedded = 1,592 cf x 40.0% Voids
#2A	80.00'	710 cf	Concrete Galley 4x4x4 x 16 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 2 Rows of 8 Chambers
		1,346 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	79.00'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	83.00'	8.0" Vert. Orifice/Grate C= 0.600

Discarded OutFlow Max=0.05 cfs @ 12.52 hrs HW=83.17' (Free Discharge)
 ↑**1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.10 cfs @ 12.52 hrs HW=83.17' (Free Discharge)
 ↑**2=Orifice/Grate** (Orifice Controls 0.10 cfs @ 1.41 fps)

Pond 25P: Subsurface infiltration#1 - Chamber Wizard Field A

Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

8 Chambers/Row x 4.00' Long = 32.00' Row Length +24.0" End Stone x 2 = 36.00' Base Length

2 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 12.80' Base Width

12.0" Base + 48.0" Chamber Height + 6.0" Cover = 5.50' Field Height

16 Chambers x 44.3 cf = 709.5 cf Chamber Storage

16 Chambers x 58.9 cf = 942.0 cf Displacement

2,534.4 cf Field - 942.0 cf Chambers = 1,592.4 cf Stone x 40.0% Voids = 637.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,346.5 cf = 0.031 af

Overall Storage Efficiency = 53.1%

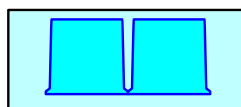
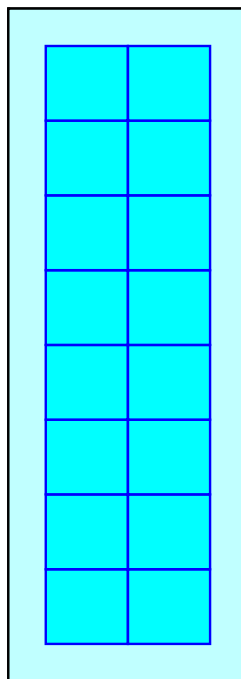
Overall System Size = 36.00' x 12.80' x 5.50'

16 Chambers @ \$ 300.00 /ea = \$ 4,800.00

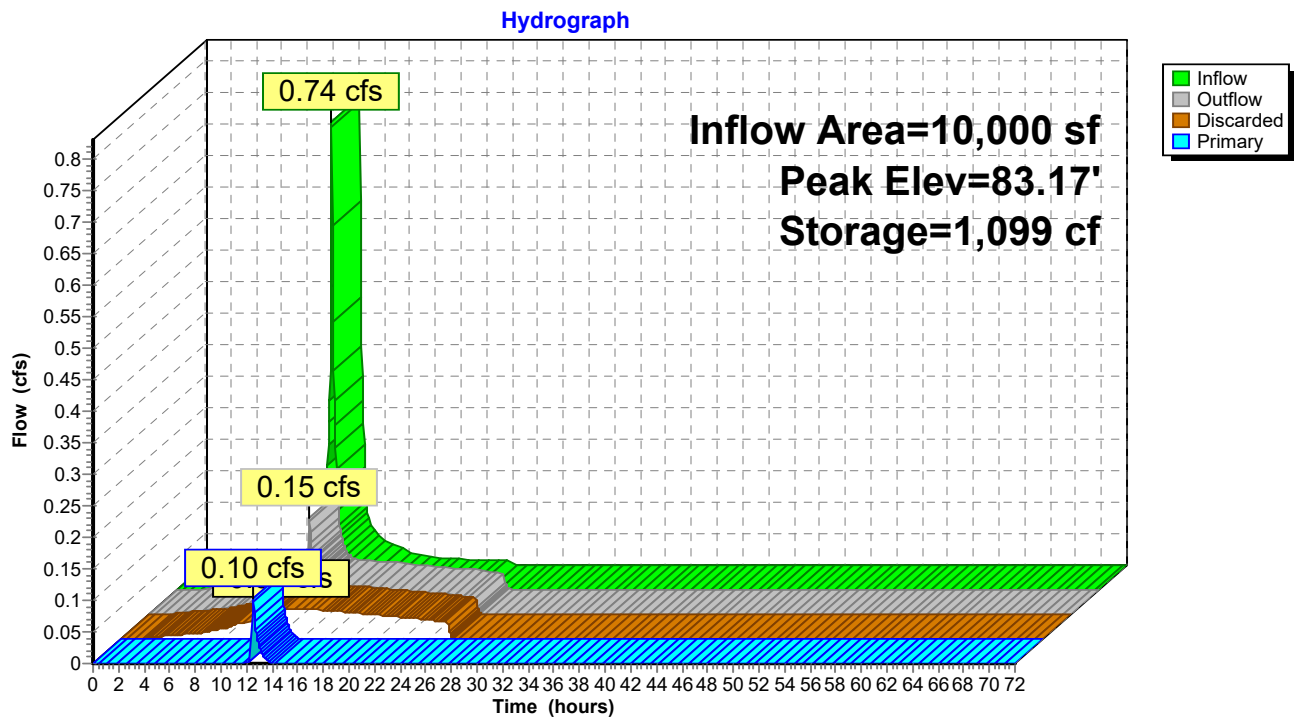
93.9 cy Field Excavation @ \$ 10.00 /cy = \$ 938.67

59.0 cy Stone @ \$ 30.00 /cy = \$ 1,769.31

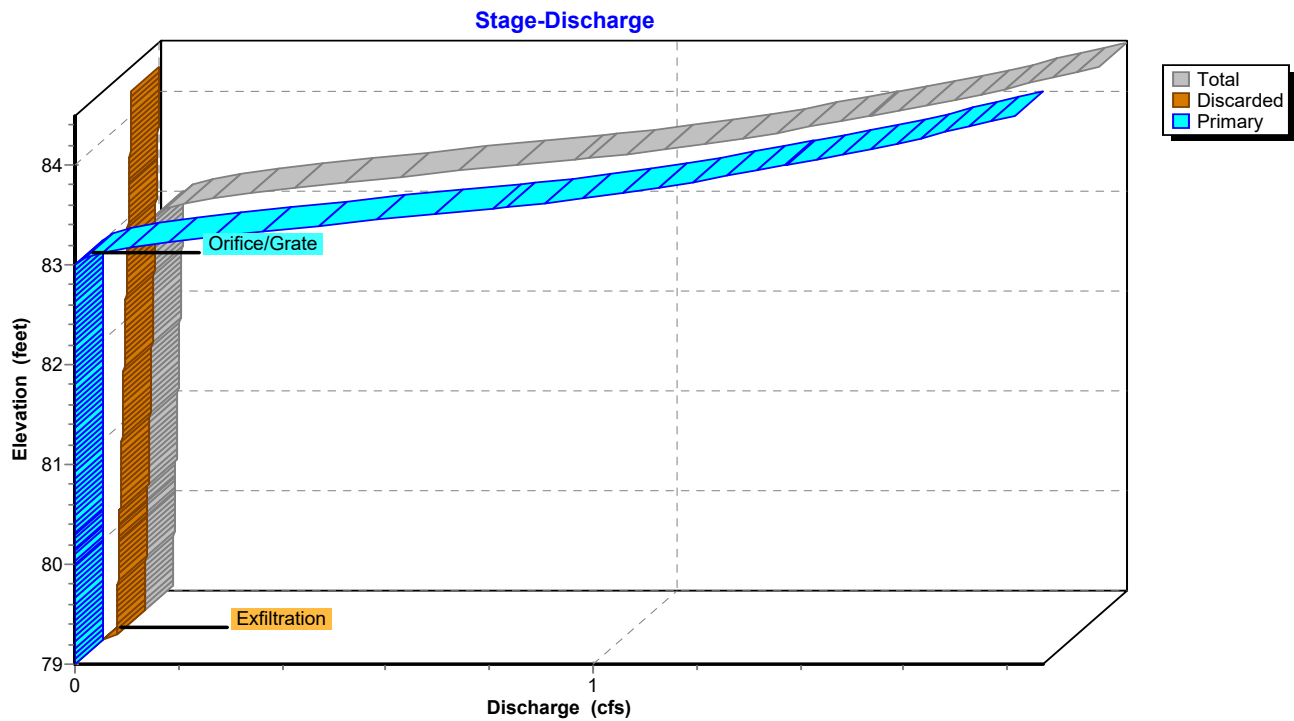
Total Cost = \$ 7,507.98



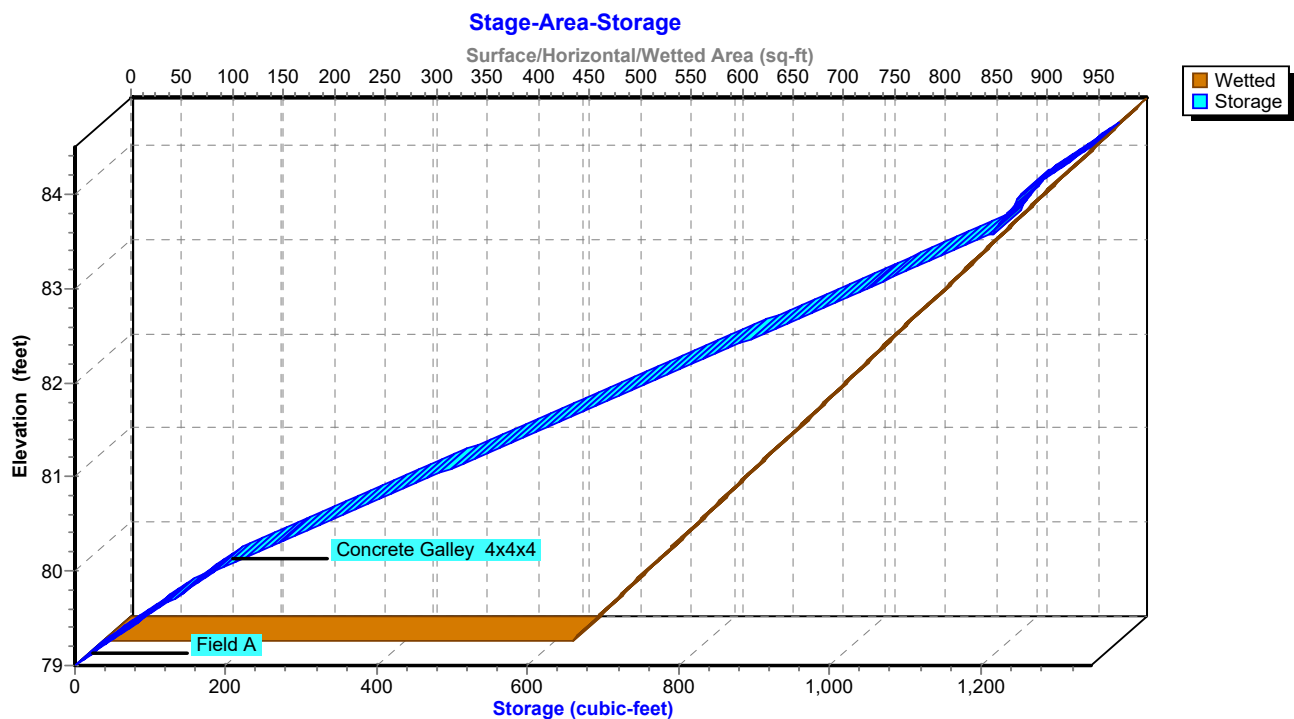
Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Summary for Pond 27P: Subsurface infiltration#2

[43] Hint: Has no inflow (Outflow=Zero)

Volume	Invert	Avail.Storage	Storage Description
#1A	75.50'	2,584 cf	21.60'W x 88.00'L x 6.00'H Field A 11,405 cf Overall - 4,946 cf Embedded = 6,459 cf x 40.0% Voids
#2A	76.50'	3,725 cf	Concrete Galley 4x4x4 x 84 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 4 Rows of 21 Chambers
#3	81.50'	15 cf	Custom Stage Data (Prismatic) Listed below (Recalc) -Impervious
		6,324 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.50	6	0	0
84.00	6	15	15

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	80.00'	6.0" Vert. Orifice/Grate C= 0.600
#3	Primary	78.75'	6.0" Vert. Orifice/Grate C= 0.600
#4	Primary	78.10'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)↑**1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)↑**2=Orifice/Grate** (Controls 0.00 cfs)↑**3=Orifice/Grate** (Controls 0.00 cfs)↑**4=Orifice/Grate** (Controls 0.00 cfs)

Pond 27P: Subsurface infiltration#2 - Chamber Wizard Field A

Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

21 Chambers/Row x 4.00' Long = 84.00' Row Length +24.0" End Stone x 2 = 88.00' Base Length

4 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 21.60' Base Width

12.0" Base + 48.0" Chamber Height + 12.0" Cover = 6.00' Field Height

84 Chambers x 44.3 cf = 3,725.0 cf Chamber Storage

84 Chambers x 58.9 cf = 4,945.6 cf Displacement

11,404.8 cf Field - 4,945.6 cf Chambers = 6,459.2 cf Stone x 40.0% Voids = 2,583.7 cf Stone Storage

Chamber Storage + Stone Storage = 6,308.6 cf = 0.145 af

Overall Storage Efficiency = 55.3%

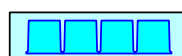
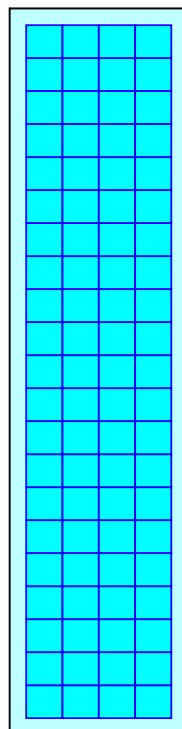
Overall System Size = 88.00' x 21.60' x 6.00'

84 Chambers @ \$ 300.00 /ea = \$ 25,200.00

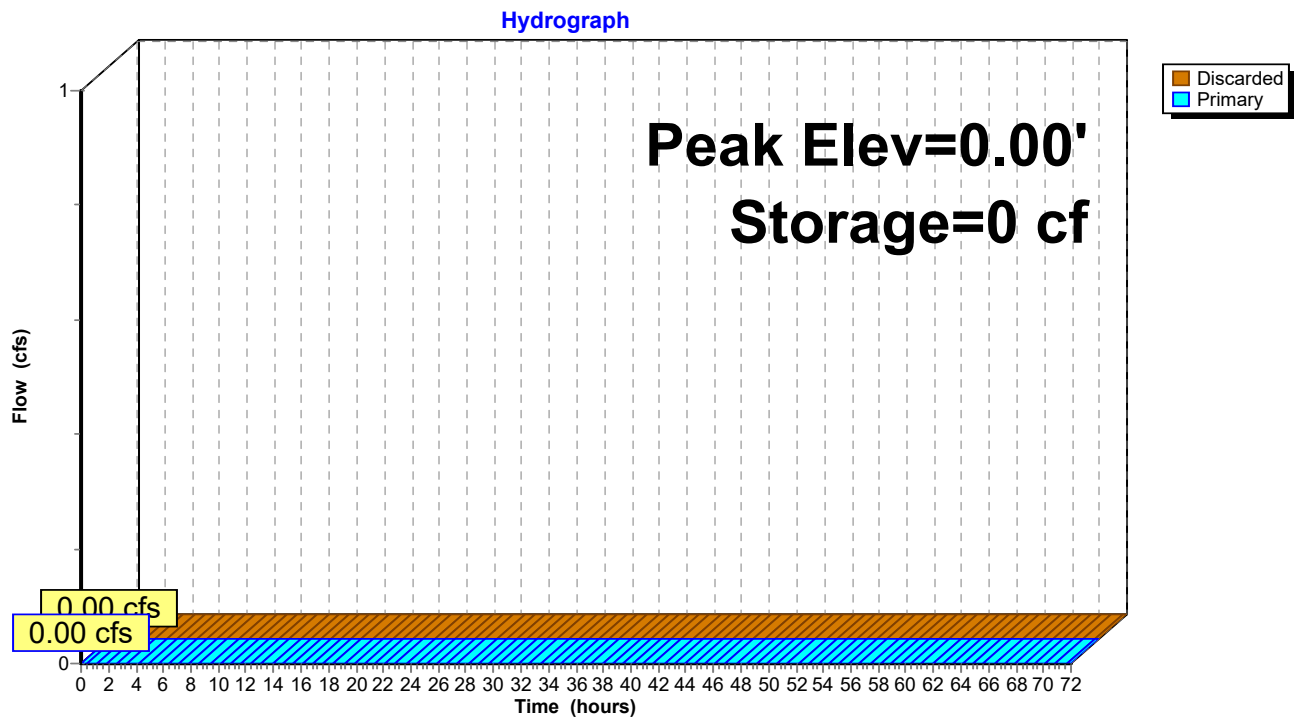
422.4 cy Field Excavation @ \$ 10.00 /cy = \$ 4,224.00

239.2 cy Stone @ \$ 30.00 /cy = \$ 7,176.88

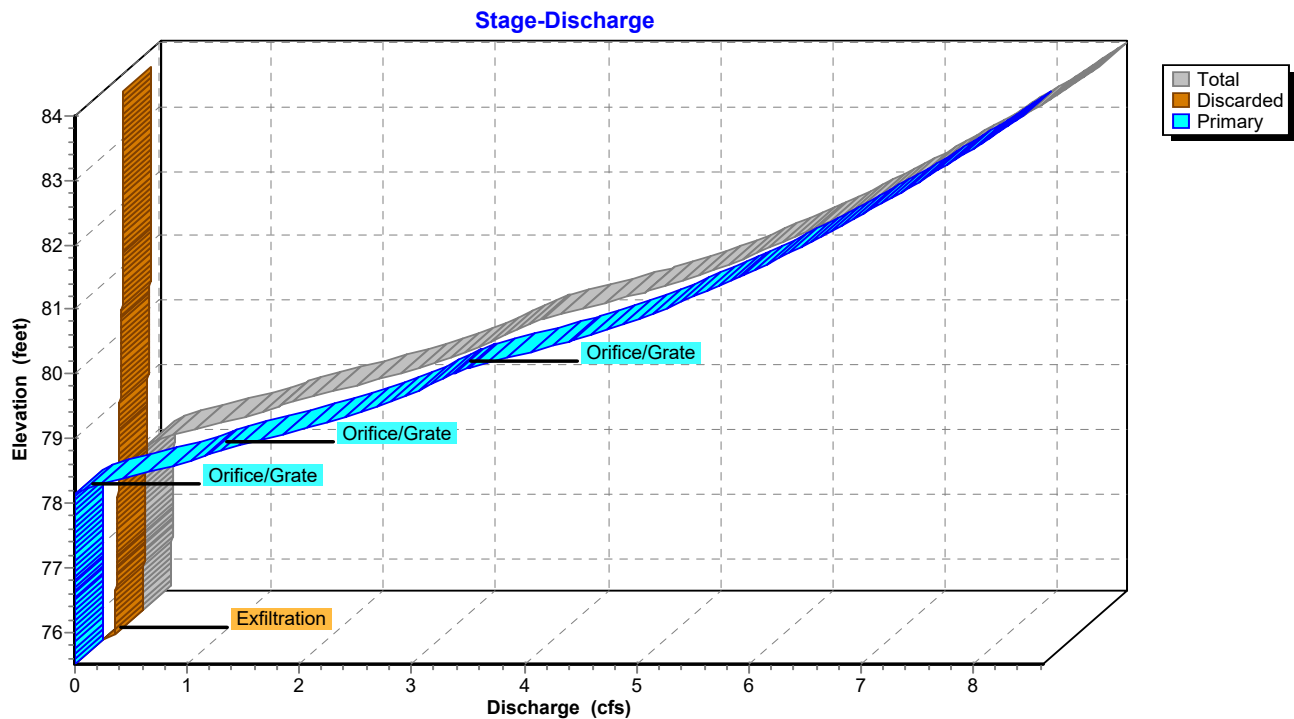
Total Cost = \$ 36,600.88



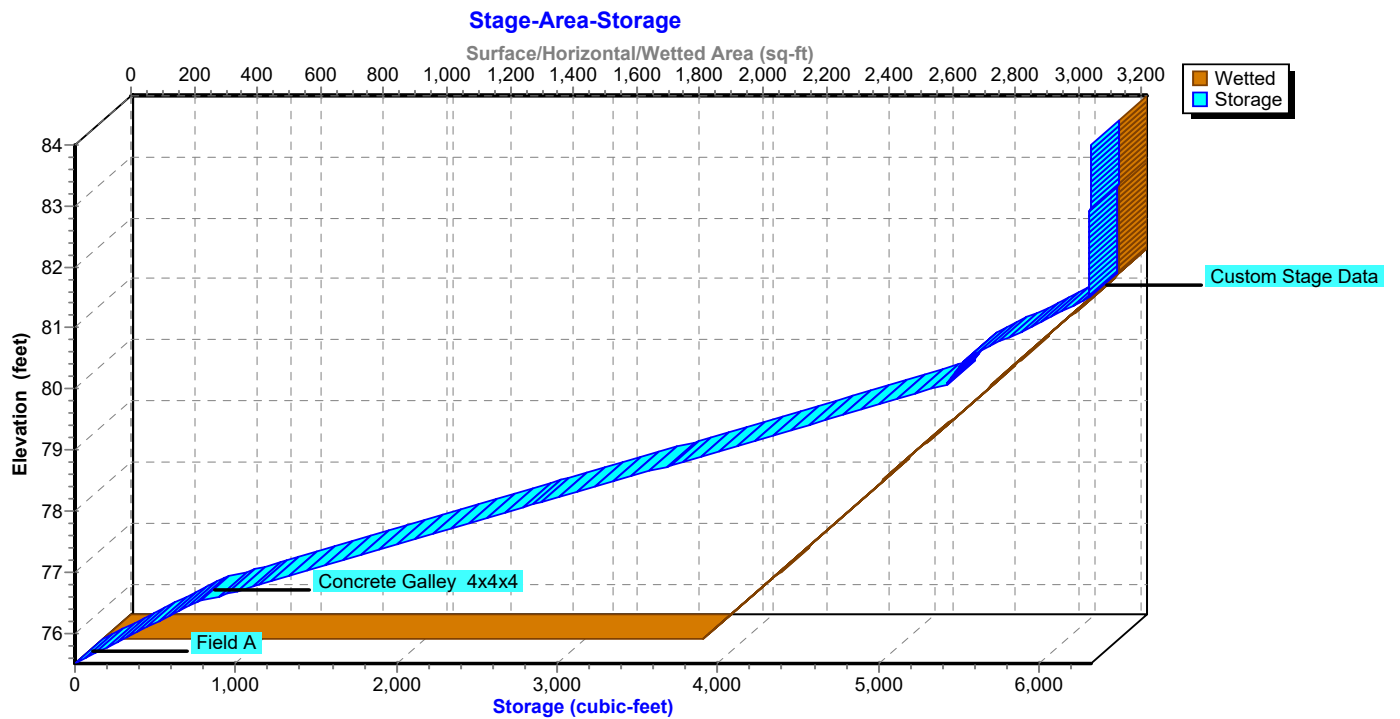
Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2

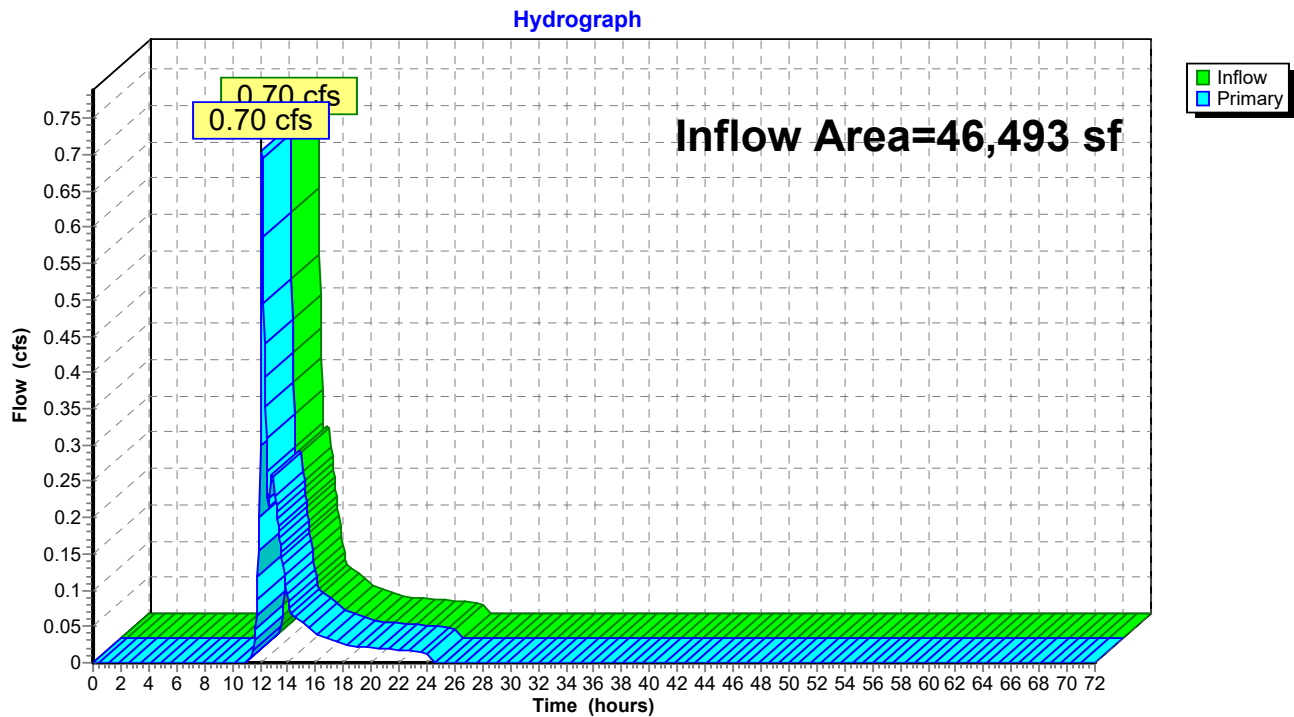


Summary for Link 23L: (Combined Link)

Inflow Area = 46,493 sf, 9.37% Impervious, Inflow Depth = 0.80" for 2-Year event
 Inflow = 0.70 cfs @ 12.13 hrs, Volume= 3,082 cf
 Primary = 0.70 cfs @ 12.13 hrs, Volume= 3,082 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Link 23L: (Combined Link)



737WashingtonSt - 10000 sf office

Type III 24-hr 10-Year Rainfall=4.70"

Prepared by {enter your company name here}

Printed 10/29/2021

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Page 27

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment13S: POST (4) Runoff Area=17,504 sf 100.00% Impervious Runoff Depth=4.46"
 Tc=6.0 min CN=98 Runoff=1.80 cfs 6,511 cf

Subcatchment15S: PRE (3) Runoff Area=97,629 sf 0.00% Impervious Runoff Depth=1.82"
 Flow Length=382' Tc=9.7 min CN=70 Runoff=4.05 cfs 14,781 cf

Subcatchment16S: POST (7) Runoff Area=13,121 sf 33.19% Impervious Runoff Depth=2.81"
 Tc=6.0 min CN=82 Runoff=0.97 cfs 3,075 cf

Subcatchment18S: POST (5) Runoff Area=10,000 sf 100.00% Impervious Runoff Depth=4.46"
 Tc=6.0 min CN=98 Runoff=1.03 cfs 3,720 cf

Subcatchment19S: POST (6) Runoff Area=30,906 sf 0.00% Impervious Runoff Depth=2.13"
 Flow Length=180' Tc=9.2 min CN=74 Runoff=1.55 cfs 5,479 cf

Subcatchment20S: POST (8) Runoff Area=33,372 sf 0.00% Impervious Runoff Depth=1.82"
 Flow Length=134' Tc=8.0 min CN=70 Runoff=1.46 cfs 5,052 cf

Pond 21P: Basin #2 Peak Elev=76.38' Storage=1,242 cf Inflow=1.81 cfs 7,211 cf
 Discarded=0.09 cfs 2,682 cf Primary=0.47 cfs 1,149 cf Tertiary=1.16 cfs 3,380 cf Outflow=1.72 cfs 7,211 cf

Pond 24P: Basin #1 Peak Elev=78.44' Storage=3,932 cf Inflow=3.55 cfs 12,861 cf
 Discarded=0.19 cfs 8,724 cf Tertiary=1.46 cfs 4,136 cf Outflow=1.64 cfs 12,861 cf

Pond 25P: Subsurface infiltration#1 Peak Elev=83.48' Storage=1,187 cf Inflow=1.03 cfs 3,720 cf
 Discarded=0.05 cfs 2,849 cf Primary=0.64 cfs 871 cf Outflow=0.69 cfs 3,720 cf

Pond 27P: Subsurface infiltration#2 Peak Elev=0.00' Storage=0 cf
 Discarded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Link 23L: (Combined Link) Inflow=2.26 cfs 9,581 cf
 Primary=2.26 cfs 9,581 cf

Total Runoff Area = 202,532 sf Runoff Volume = 38,618 cf Average Runoff Depth = 2.29"
84.27% Pervious = 170,673 sf 15.73% Impervious = 31,859 sf

Summary for Subcatchment 13S: POST (4)

Runoff = 1.80 cfs @ 12.09 hrs, Volume= 6,511 cf, Depth= 4.46"

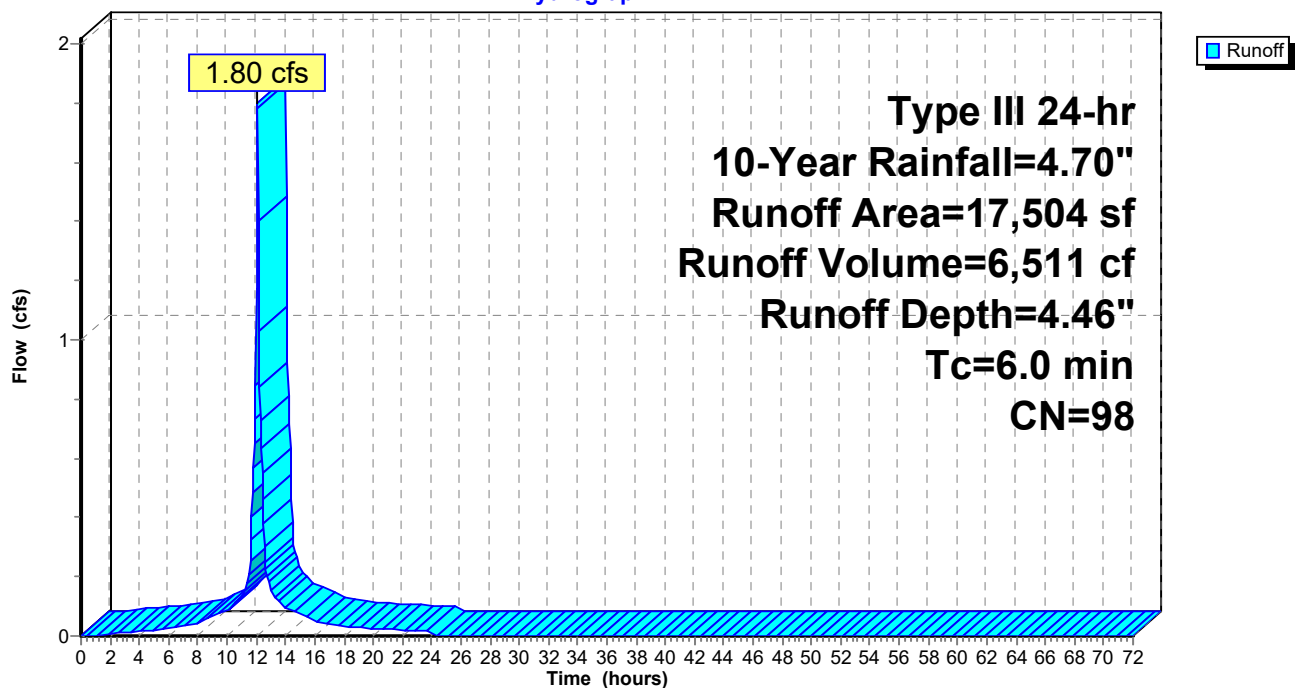
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.70"

Area (sf)	CN	Description
17,504	98	Paved parking, HSG C
17,504	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, runoff from pavement

Subcatchment 13S: POST (4)

Hydrograph



Summary for Subcatchment 15S: PRE (3)

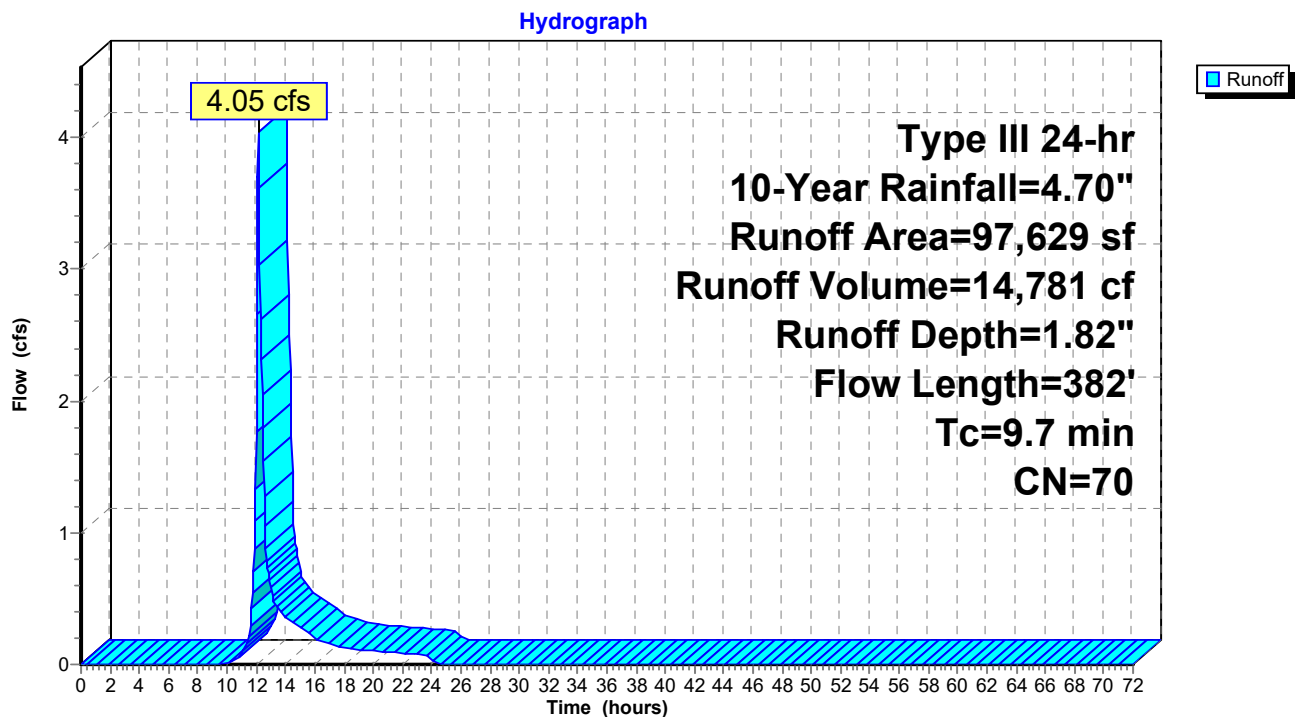
Runoff = 4.05 cfs @ 12.15 hrs, Volume= 14,781 cf, Depth= 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.70"

Area (sf)	CN	Description
97,629	70	Woods, Good, HSG C
97,629	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.2000	0.17		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
2.6	138	0.0320	0.89		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.8	146	0.0710	1.33		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.5	48	0.0850	1.46		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
9.7	382	Total			

Subcatchment 15S: PRE (3)



Summary for Subcatchment 16S: POST (7)

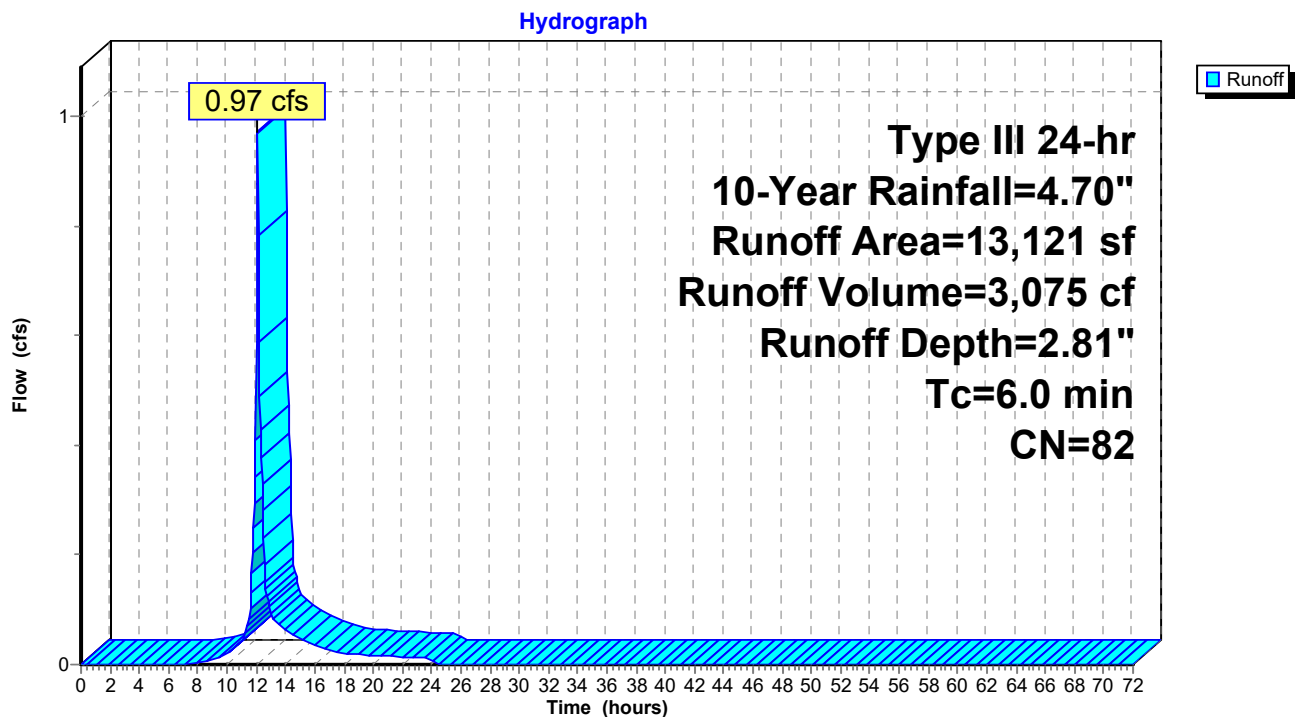
Runoff = 0.97 cfs @ 12.09 hrs, Volume= 3,075 cf, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.70"

Area (sf)	CN	Description
8,766	74	>75% Grass cover, Good, HSG C
4,355	98	Paved parking, HSG C
13,121	82	Weighted Average
8,766	74	66.81% Pervious Area
4,355	98	33.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 16S: POST (7)



Summary for Subcatchment 18S: POST (5)

Runoff = 1.03 cfs @ 12.09 hrs, Volume= 3,720 cf, Depth= 4.46"

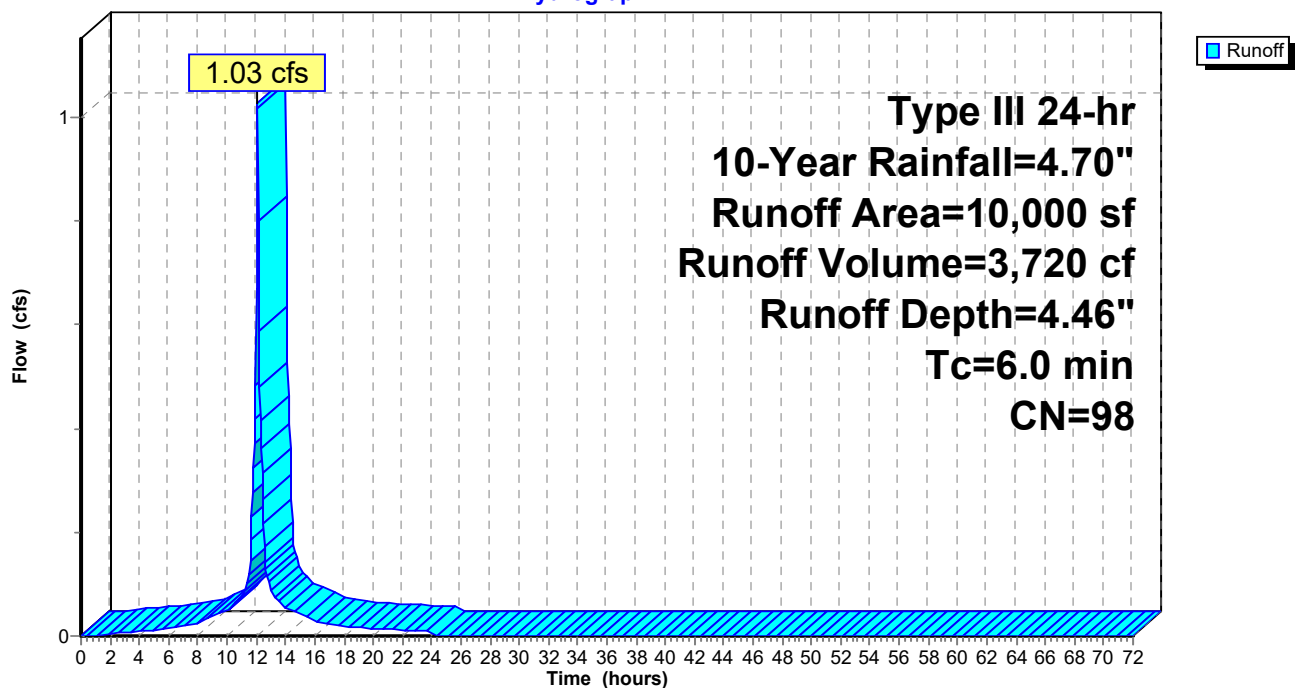
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.70"

Area (sf)	CN	Description
10,000	98	Roofs, HSG C
10,000	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 18S: POST (5)

Hydrograph



Summary for Subcatchment 19S: POST (6)

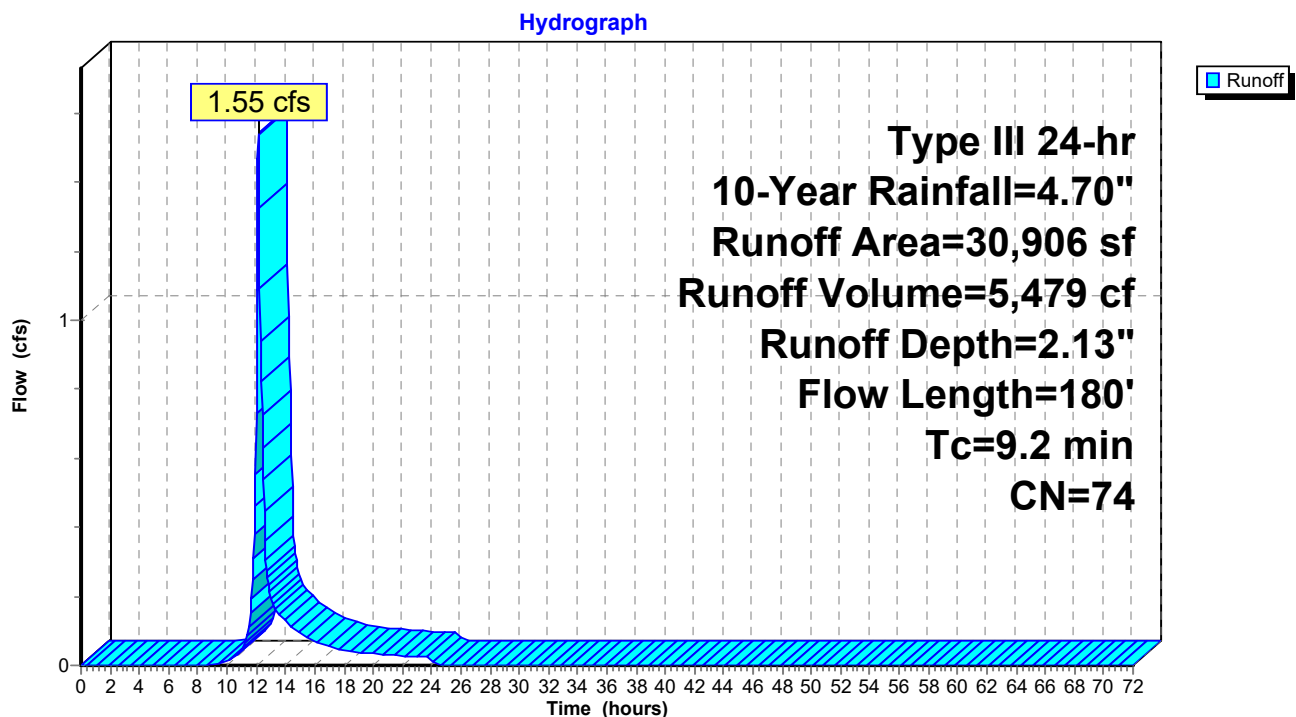
Runoff = 1.55 cfs @ 12.14 hrs, Volume= 5,479 cf, Depth= 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.70"

Area (sf)	CN	Description
30,906	74	>75% Grass cover, Good, HSG C
30,906	74	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	50	0.0200	0.10		Sheet Flow, Grass: Dense n= 0.240 P2= 3.40"
0.9	93	0.0600	1.71		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.3	37	0.1200	2.42		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
9.2	180	Total			

Subcatchment 19S: POST (6)



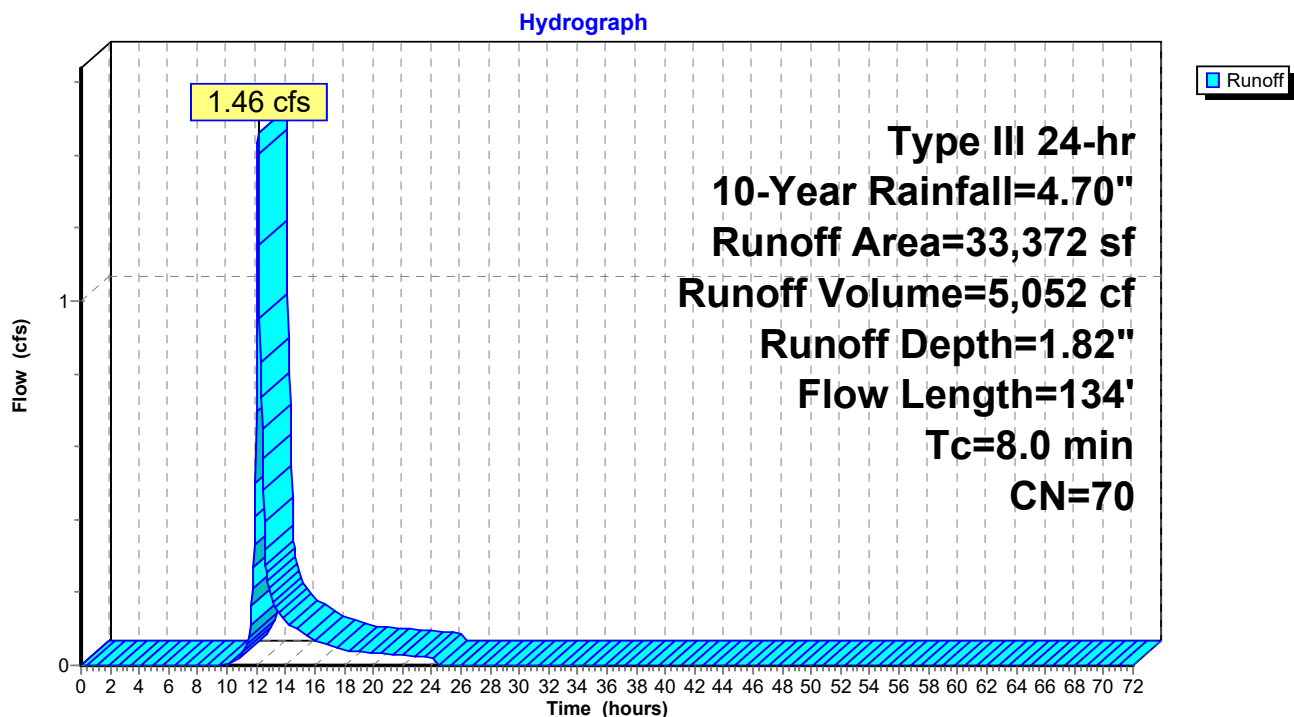
Summary for Subcatchment 20S: POST (8)

Runoff = 1.46 cfs @ 12.12 hrs, Volume= 5,052 cf, Depth= 1.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-Year Rainfall=4.70"

Area (sf)	CN	Description
1,400	74	>75% Grass cover, Good, HSG C
31,972	70	Woods, Good, HSG C
33,372	70	Weighted Average
33,372	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0800	0.12		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
1.1	84	0.0700	1.32		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
8.0	134	Total			

Subcatchment 20S: POST (8)

Summary for Pond 21P: Basin #2

Inflow Area = 13,121 sf, 33.19% Impervious, Inflow Depth = 6.60" for 10-Year event
 Inflow = 1.81 cfs @ 12.36 hrs, Volume= 7,211 cf
 Outflow = 1.72 cfs @ 12.44 hrs, Volume= 7,211 cf, Atten= 5%, Lag= 5.1 min
 Discarded = 0.09 cfs @ 12.44 hrs, Volume= 2,682 cf
 Primary = 0.47 cfs @ 12.44 hrs, Volume= 1,149 cf
 Tertiary = 1.16 cfs @ 12.44 hrs, Volume= 3,380 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 76.38' @ 12.44 hrs Surf.Area= 1,635 sf Storage= 1,242 cf

Plug-Flow detention time= 35.9 min calculated for 7,206 cf (100% of inflow)
 Center-of-Mass det. time= 35.9 min (827.3 - 791.4)

Volume	Invert	Avail.Storage	Storage Description
#1	75.50'	4,644 cf	stage (Conic) Listed below

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
75.50	1,179	0	0	1,179
76.00	1,429	651	651	1,437
76.50	1,702	782	1,433	1,719
77.00	1,988	922	2,354	2,015
77.50	2,288	1,068	3,422	2,326
78.00	2,603	1,222	4,644	2,653

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	76.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height
#3	Primary	76.00'	9.0" Vert. Orifice/Grate C= 0.600

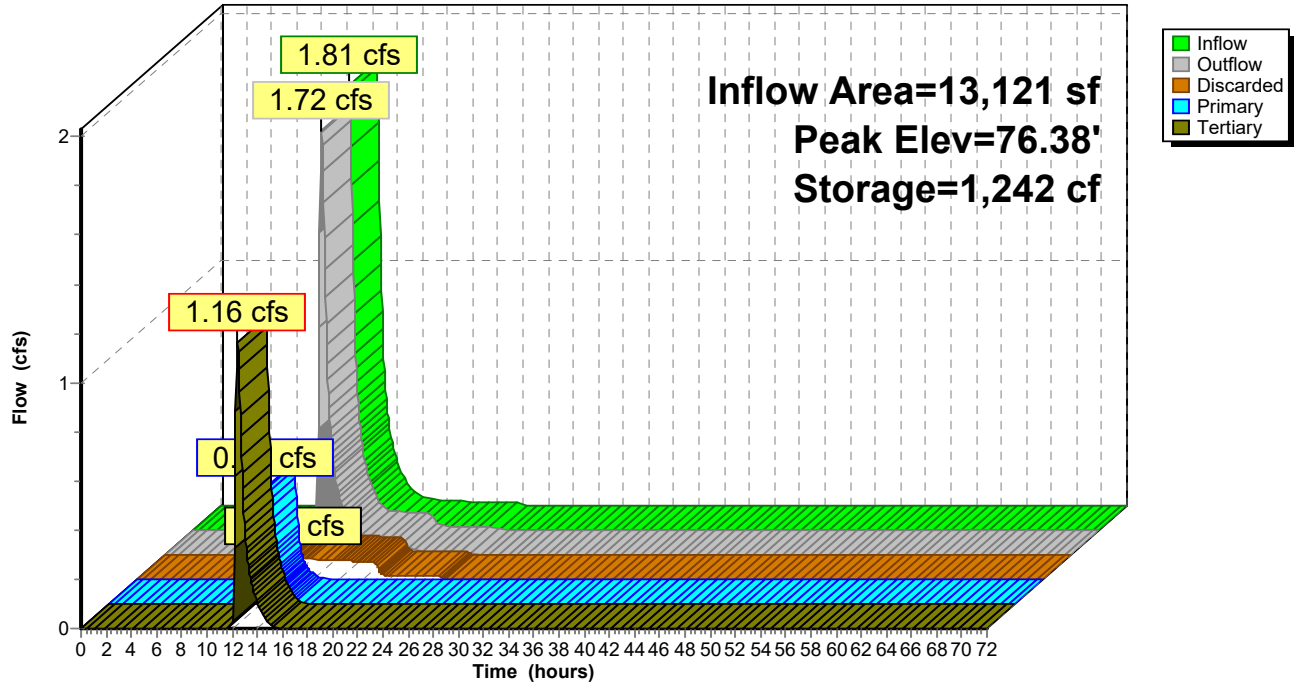
Discarded OutFlow Max=0.09 cfs @ 12.44 hrs HW=76.38' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.47 cfs @ 12.44 hrs HW=76.38' (Free Discharge)
 ↑ **3=Orifice/Grate** (Orifice Controls 0.47 cfs @ 2.09 fps)

Tertiary OutFlow Max=1.16 cfs @ 12.44 hrs HW=76.38' (Free Discharge)
 ↑ **2=Sharp-Crested Rectangular Weir** (Weir Controls 1.16 cfs @ 2.10 fps)

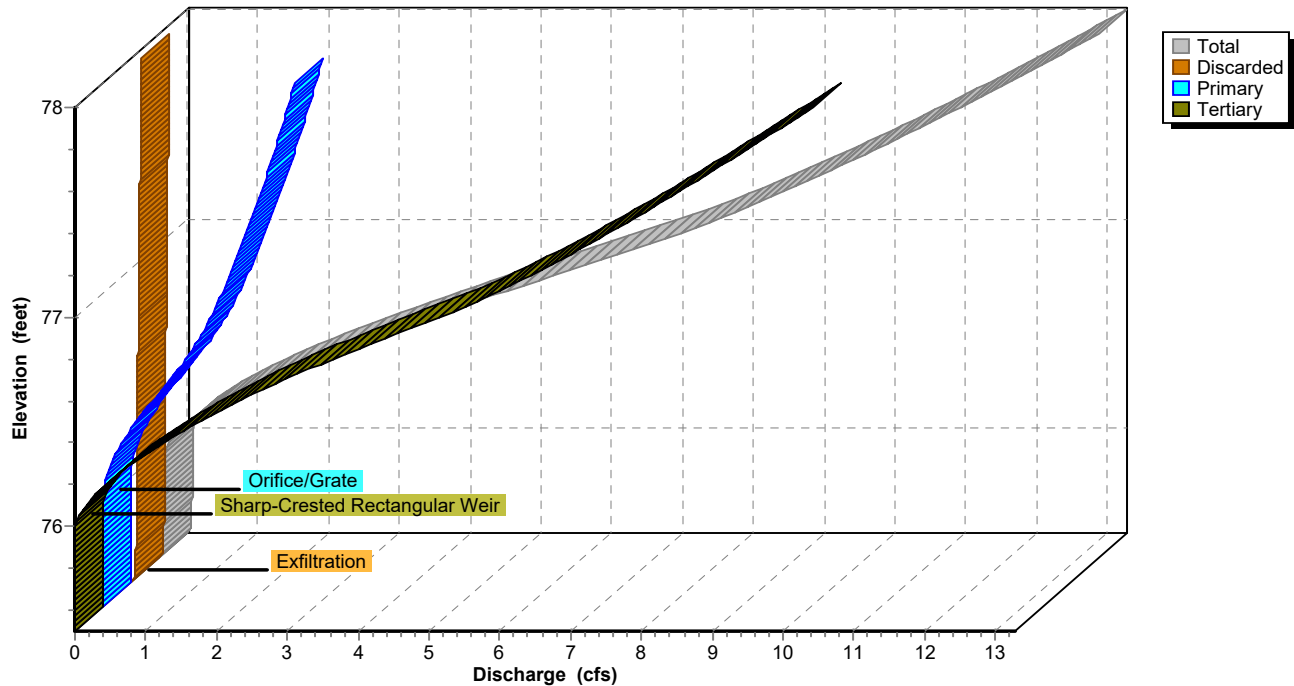
Pond 21P: Basin #2

Hydrograph



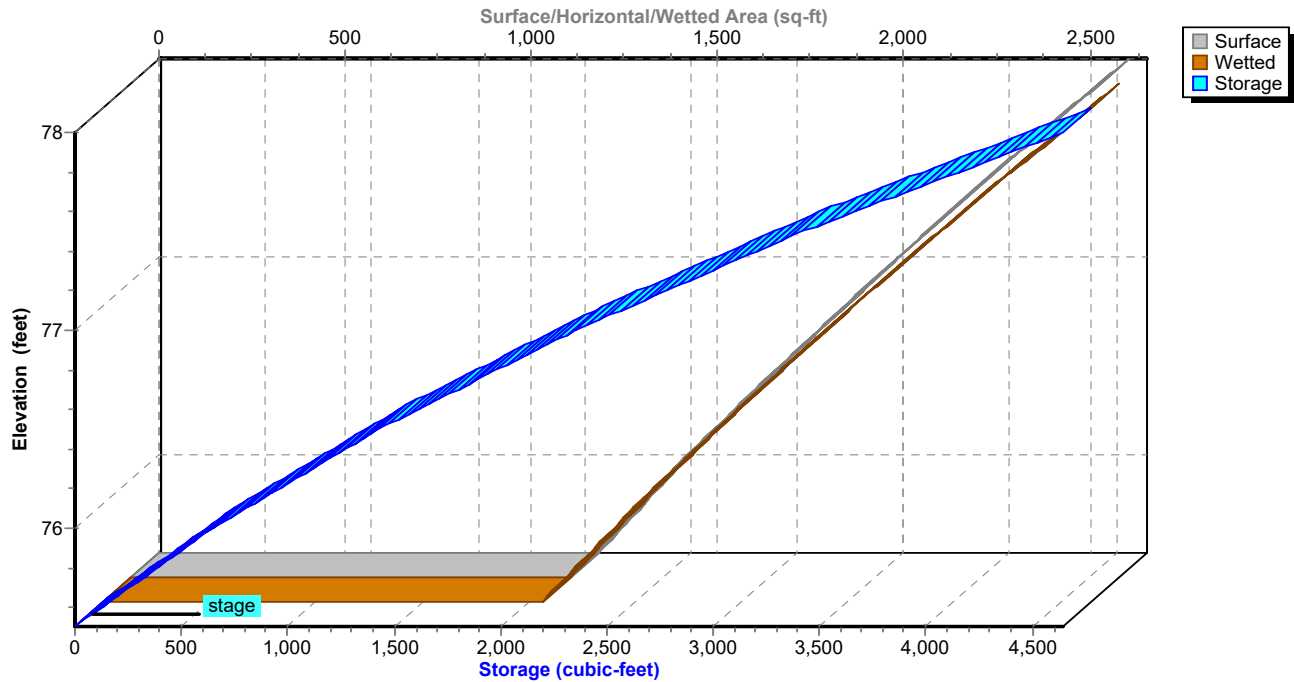
Pond 21P: Basin #2

Stage-Discharge



Pond 21P: Basin #2

Stage-Area-Storage



Summary for Pond 24P: Basin #1

Inflow Area = 58,410 sf, 47.09% Impervious, Inflow Depth = 2.64" for 10-Year event
 Inflow = 3.55 cfs @ 12.14 hrs, Volume= 12,861 cf
 Outflow = 1.64 cfs @ 12.39 hrs, Volume= 12,861 cf, Atten= 54%, Lag= 15.0 min
 Discarded = 0.19 cfs @ 12.39 hrs, Volume= 8,724 cf
 Tertiary = 1.46 cfs @ 12.39 hrs, Volume= 4,136 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 78.44' @ 12.39 hrs Surf.Area= 3,319 sf Storage= 3,932 cf

Plug-Flow detention time= 102.7 min calculated for 12,852 cf (100% of inflow)
 Center-of-Mass det. time= 102.6 min (892.0 - 789.4)

Volume	Invert	Avail.Storage	Storage Description
#1	77.10'	9,844 cf	stage (Conic) Listed below

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
77.10	2,446	0	0	2,446
78.00	3,036	2,462	2,462	3,059
79.00	3,682	3,354	5,816	3,737
80.00	4,385	4,028	9,844	4,475

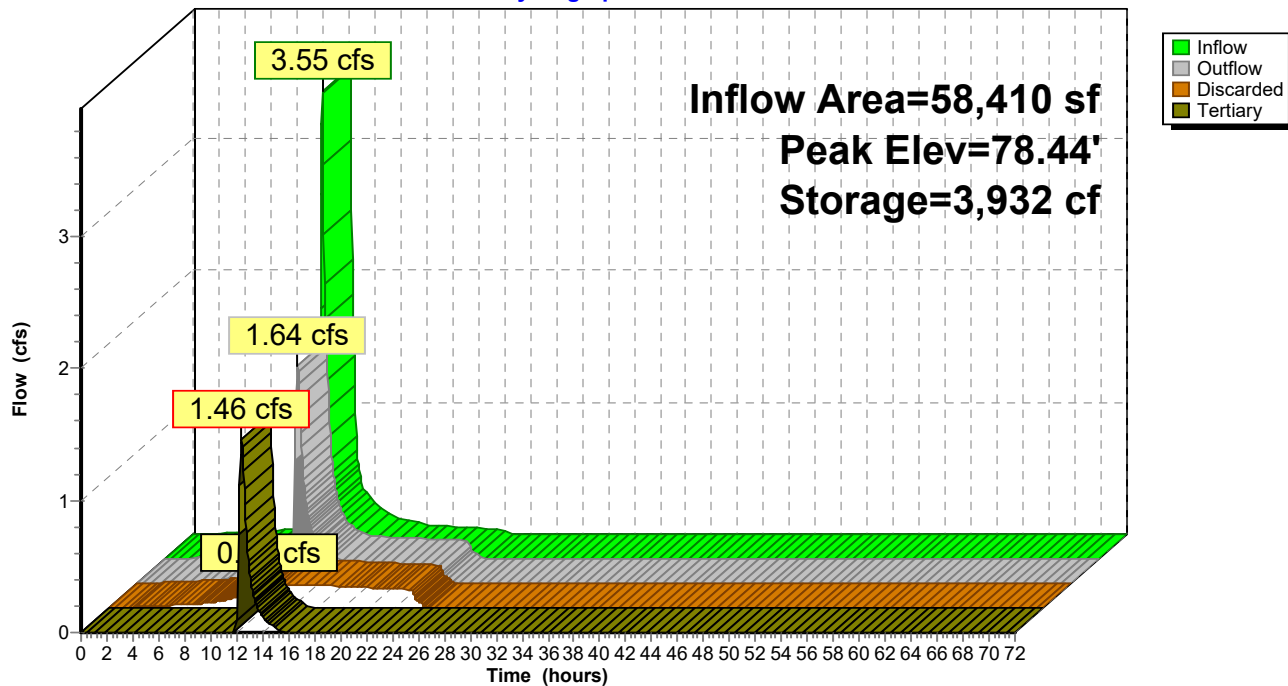
Device	Routing	Invert	Outlet Devices
#1	Discarded	77.10'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	78.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height

Discarded OutFlow Max=0.19 cfs @ 12.39 hrs HW=78.44' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Tertiary OutFlow Max=1.45 cfs @ 12.39 hrs HW=78.44' (Free Discharge)
 ↑ **2=Sharp-Crested Rectangular Weir** (Weir Controls 1.45 cfs @ 2.28 fps)

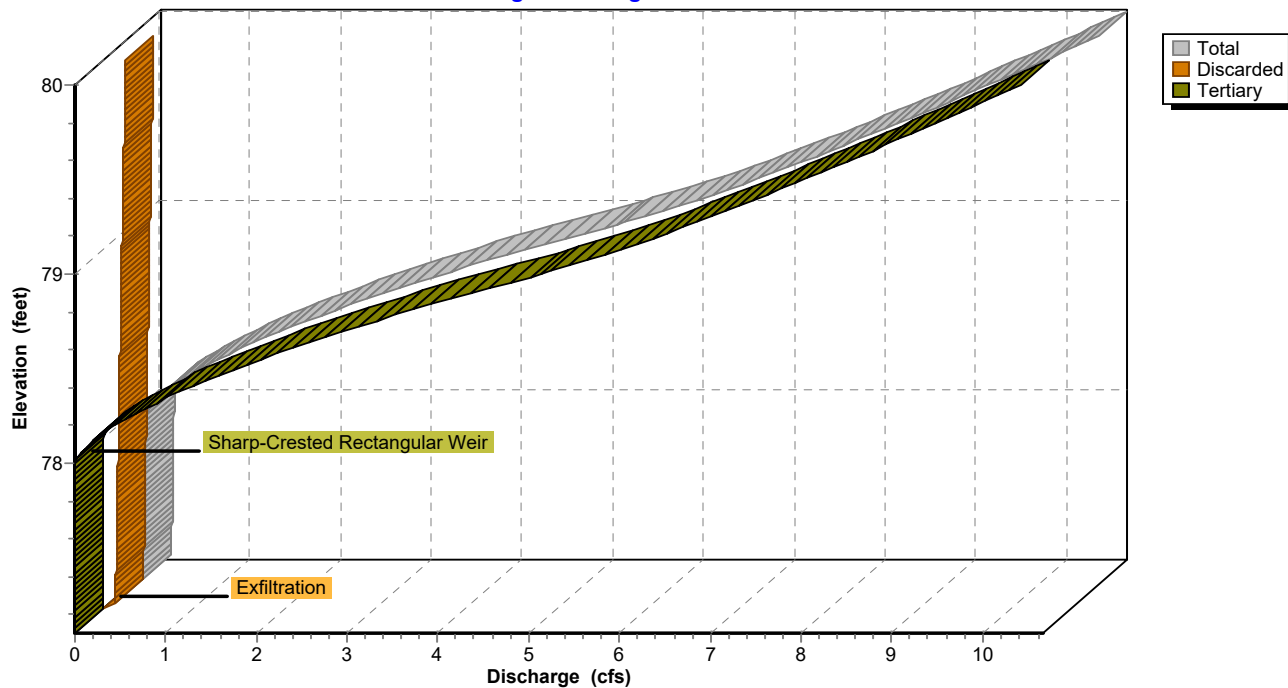
Pond 24P: Basin #1

Hydrograph



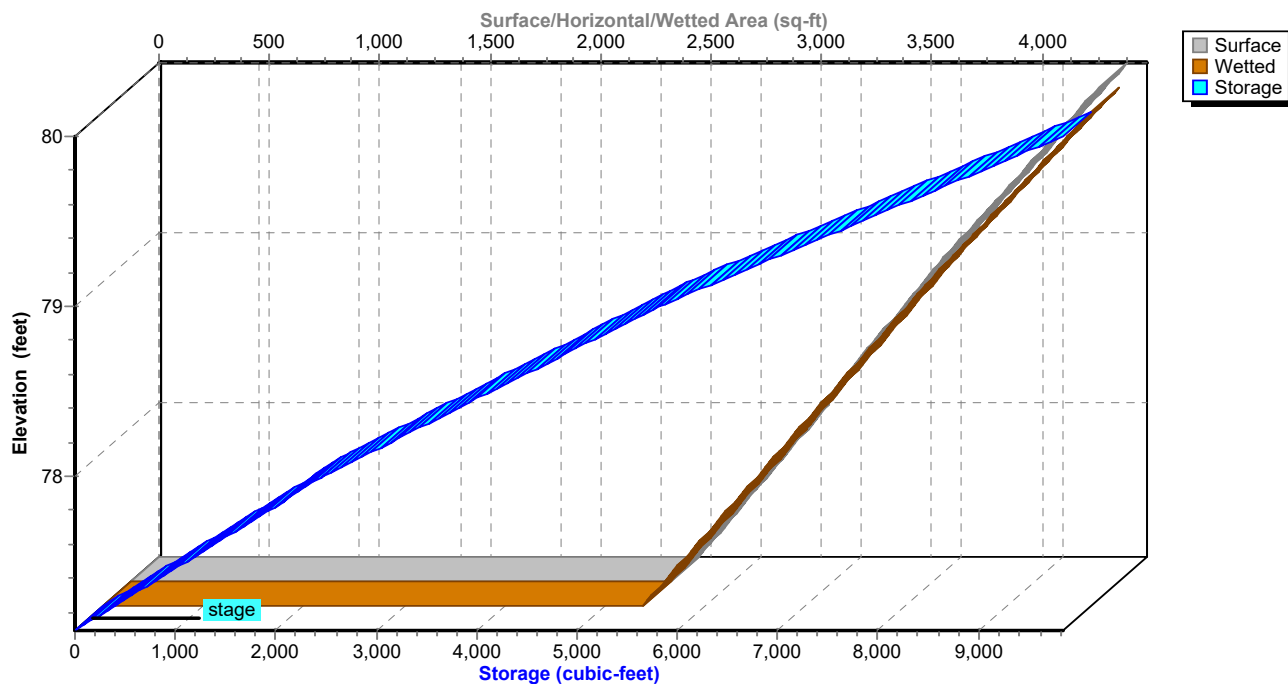
Pond 24P: Basin #1

Stage-Discharge



Pond 24P: Basin #1

Stage-Area-Storage



Summary for Pond 25P: Subsurface infiltration#1

Inflow Area = 10,000 sf, 100.00% Impervious, Inflow Depth = 4.46" for 10-Year event
 Inflow = 1.03 cfs @ 12.09 hrs, Volume= 3,720 cf
 Outflow = 0.69 cfs @ 12.19 hrs, Volume= 3,720 cf, Atten= 33%, Lag= 6.2 min
 Discarded = 0.05 cfs @ 12.19 hrs, Volume= 2,849 cf
 Primary = 0.64 cfs @ 12.19 hrs, Volume= 871 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 83.48' @ 12.19 hrs Surf.Area= 461 sf Storage= 1,187 cf

Plug-Flow detention time= 170.7 min calculated for 3,717 cf (100% of inflow)
 Center-of-Mass det. time= 170.7 min (919.8 - 749.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	79.00'	637 cf	12.80'W x 36.00'L x 5.50'H Field A 2,534 cf Overall - 942 cf Embedded = 1,592 cf x 40.0% Voids
#2A	80.00'	710 cf	Concrete Galley 4x4x4 x 16 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 2 Rows of 8 Chambers
		1,346 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	79.00'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	83.00'	8.0" Vert. Orifice/Grate C= 0.600

Discarded OutFlow Max=0.05 cfs @ 12.19 hrs HW=83.47' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.62 cfs @ 12.19 hrs HW=83.47' (Free Discharge)
 ↑ **2=Orifice/Grate** (Orifice Controls 0.62 cfs @ 2.35 fps)

Pond 25P: Subsurface infiltration#1 - Chamber Wizard Field A

Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

8 Chambers/Row x 4.00' Long = 32.00' Row Length +24.0" End Stone x 2 = 36.00' Base Length

2 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 12.80' Base Width

12.0" Base + 48.0" Chamber Height + 6.0" Cover = 5.50' Field Height

16 Chambers x 44.3 cf = 709.5 cf Chamber Storage

16 Chambers x 58.9 cf = 942.0 cf Displacement

2,534.4 cf Field - 942.0 cf Chambers = 1,592.4 cf Stone x 40.0% Voids = 637.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,346.5 cf = 0.031 af

Overall Storage Efficiency = 53.1%

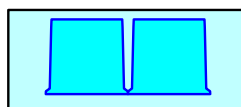
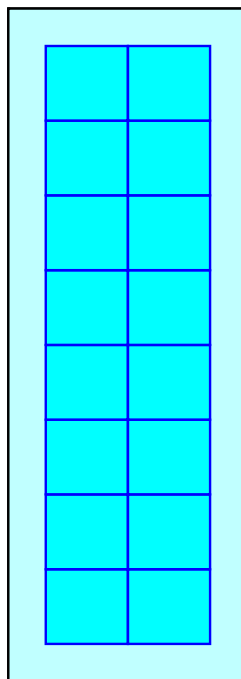
Overall System Size = 36.00' x 12.80' x 5.50'

16 Chambers @ \$ 300.00 /ea = \$ 4,800.00

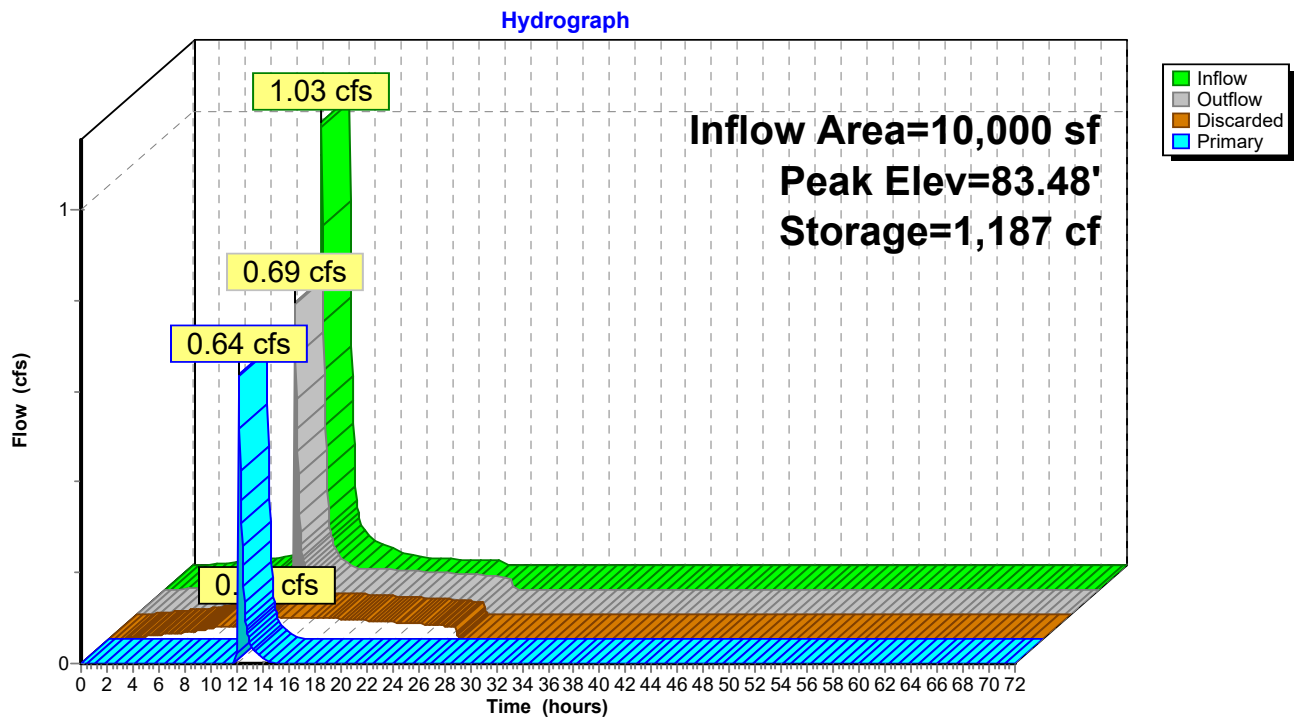
93.9 cy Field Excavation @ \$ 10.00 /cy = \$ 938.67

59.0 cy Stone @ \$ 30.00 /cy = \$ 1,769.31

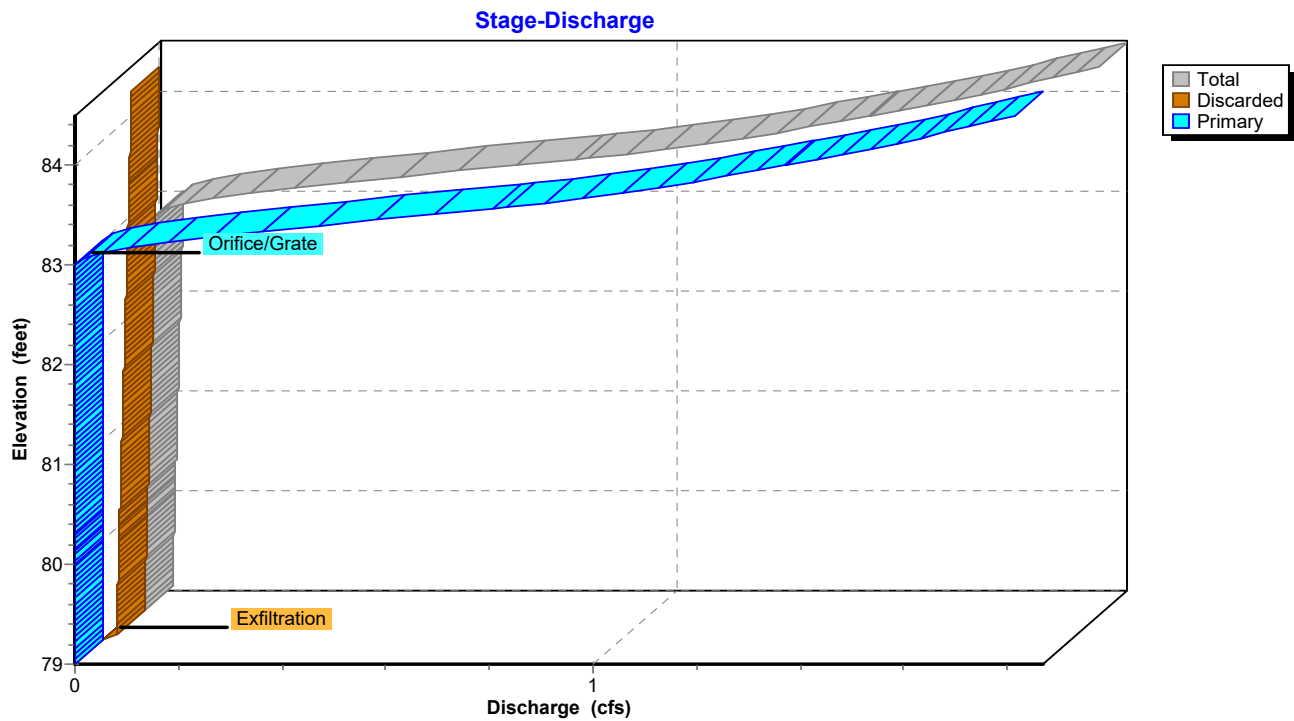
Total Cost = \$ 7,507.98



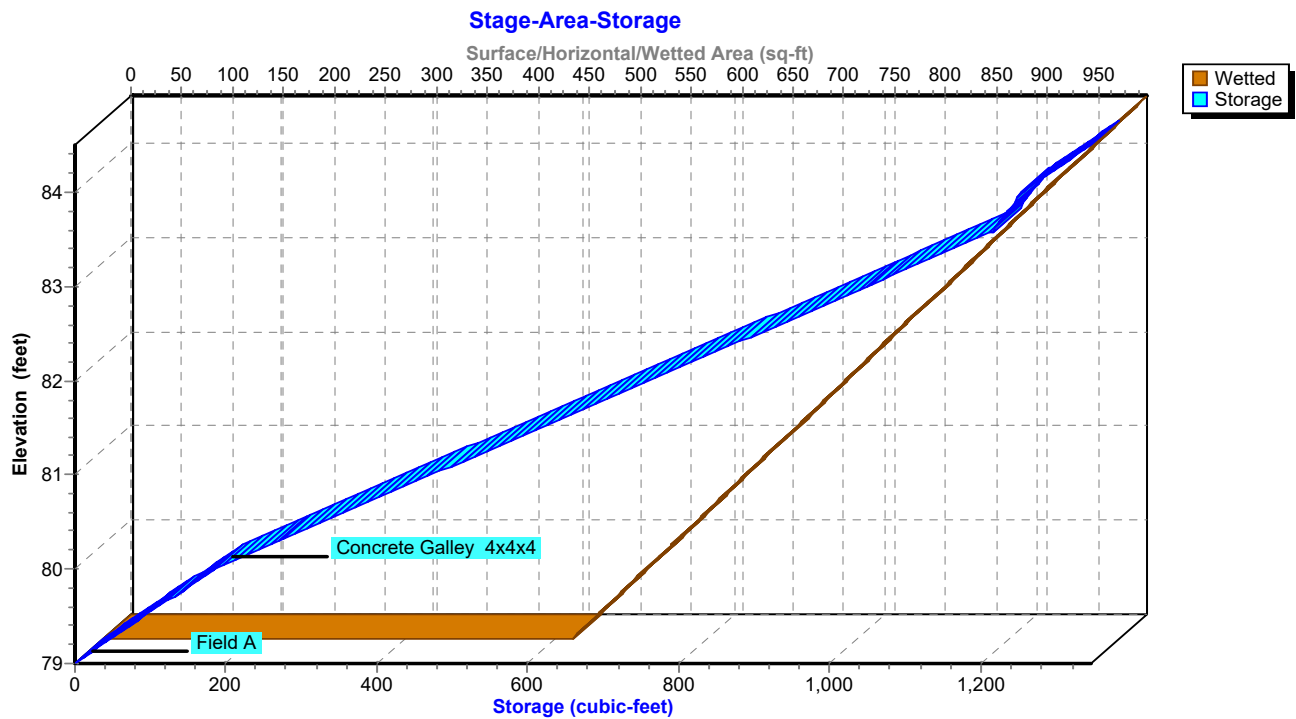
Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Summary for Pond 27P: Subsurface infiltration#2

[43] Hint: Has no inflow (Outflow=Zero)

Volume	Invert	Avail.Storage	Storage Description
#1A	75.50'	2,584 cf	21.60'W x 88.00'L x 6.00'H Field A 11,405 cf Overall - 4,946 cf Embedded = 6,459 cf x 40.0% Voids
#2A	76.50'	3,725 cf	Concrete Galley 4x4x4 x 84 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 4 Rows of 21 Chambers
#3	81.50'	15 cf	Custom Stage Data (Prismatic) Listed below (Recalc) -Impervious
		6,324 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.50	6	0	0
84.00	6	15	15

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	80.00'	6.0" Vert. Orifice/Grate C= 0.600
#3	Primary	78.75'	6.0" Vert. Orifice/Grate C= 0.600
#4	Primary	78.10'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**1=Exfiltration** (Controls 0.00 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**2=Orifice/Grate** (Controls 0.00 cfs)

↑**3=Orifice/Grate** (Controls 0.00 cfs)

↑**4=Orifice/Grate** (Controls 0.00 cfs)

Pond 27P: Subsurface infiltration#2 - Chamber Wizard Field A

Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

21 Chambers/Row x 4.00' Long = 84.00' Row Length +24.0" End Stone x 2 = 88.00' Base Length

4 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 21.60' Base Width

12.0" Base + 48.0" Chamber Height + 12.0" Cover = 6.00' Field Height

84 Chambers x 44.3 cf = 3,725.0 cf Chamber Storage

84 Chambers x 58.9 cf = 4,945.6 cf Displacement

11,404.8 cf Field - 4,945.6 cf Chambers = 6,459.2 cf Stone x 40.0% Voids = 2,583.7 cf Stone Storage

Chamber Storage + Stone Storage = 6,308.6 cf = 0.145 af

Overall Storage Efficiency = 55.3%

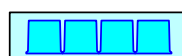
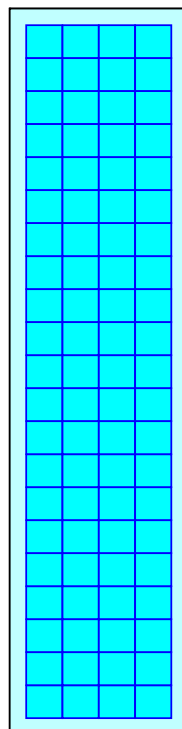
Overall System Size = 88.00' x 21.60' x 6.00'

84 Chambers @ \$ 300.00 /ea = \$ 25,200.00

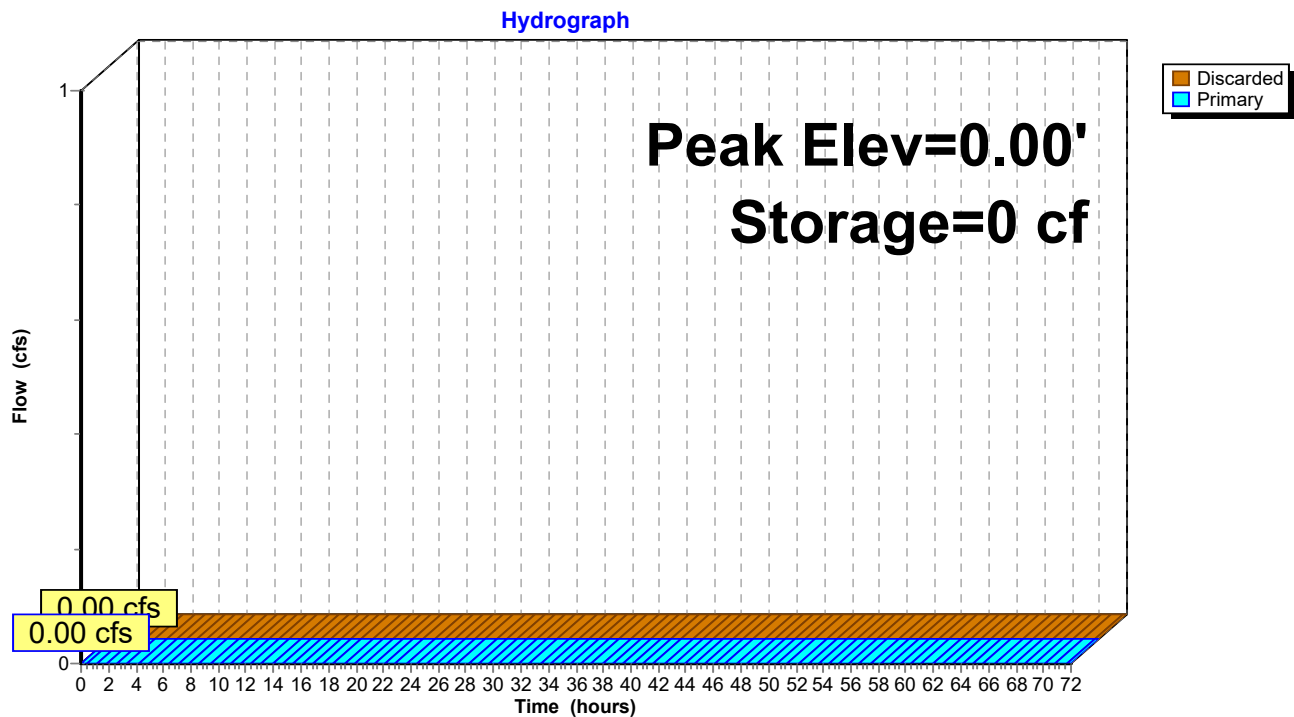
422.4 cy Field Excavation @ \$ 10.00 /cy = \$ 4,224.00

239.2 cy Stone @ \$ 30.00 /cy = \$ 7,176.88

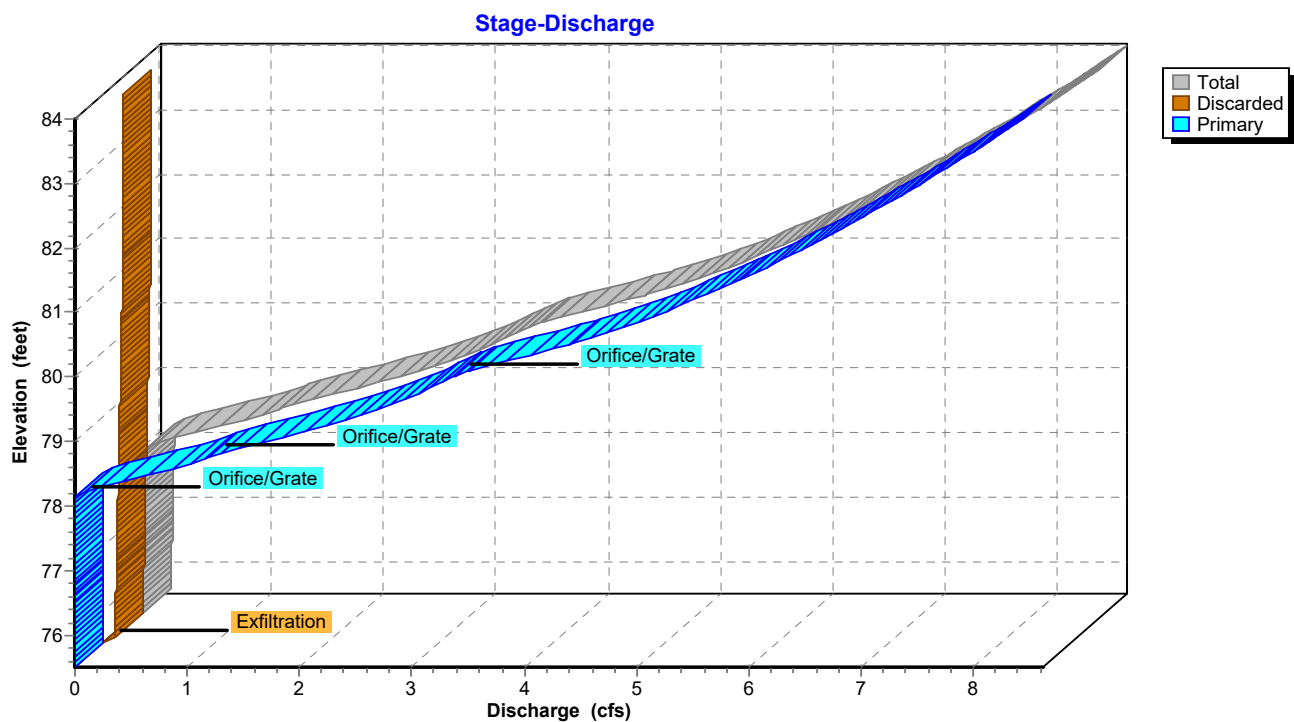
Total Cost = \$ 36,600.88



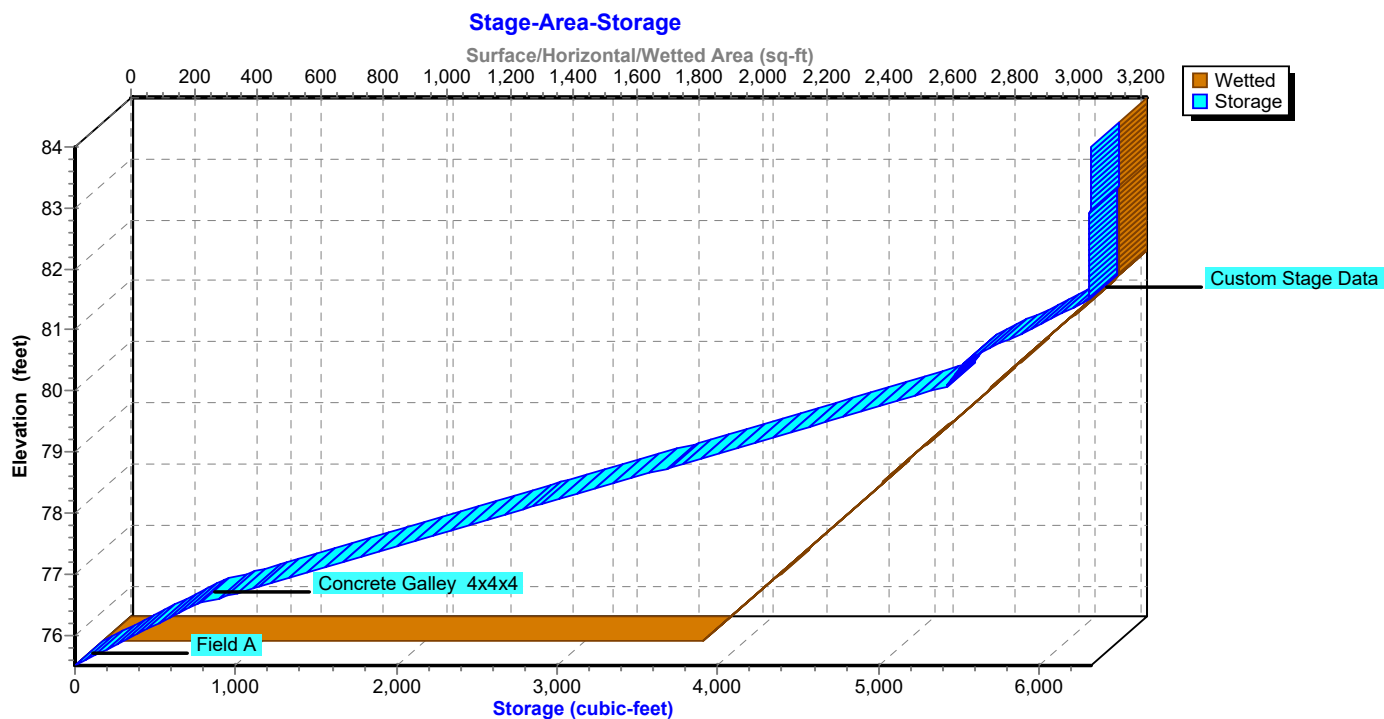
Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2

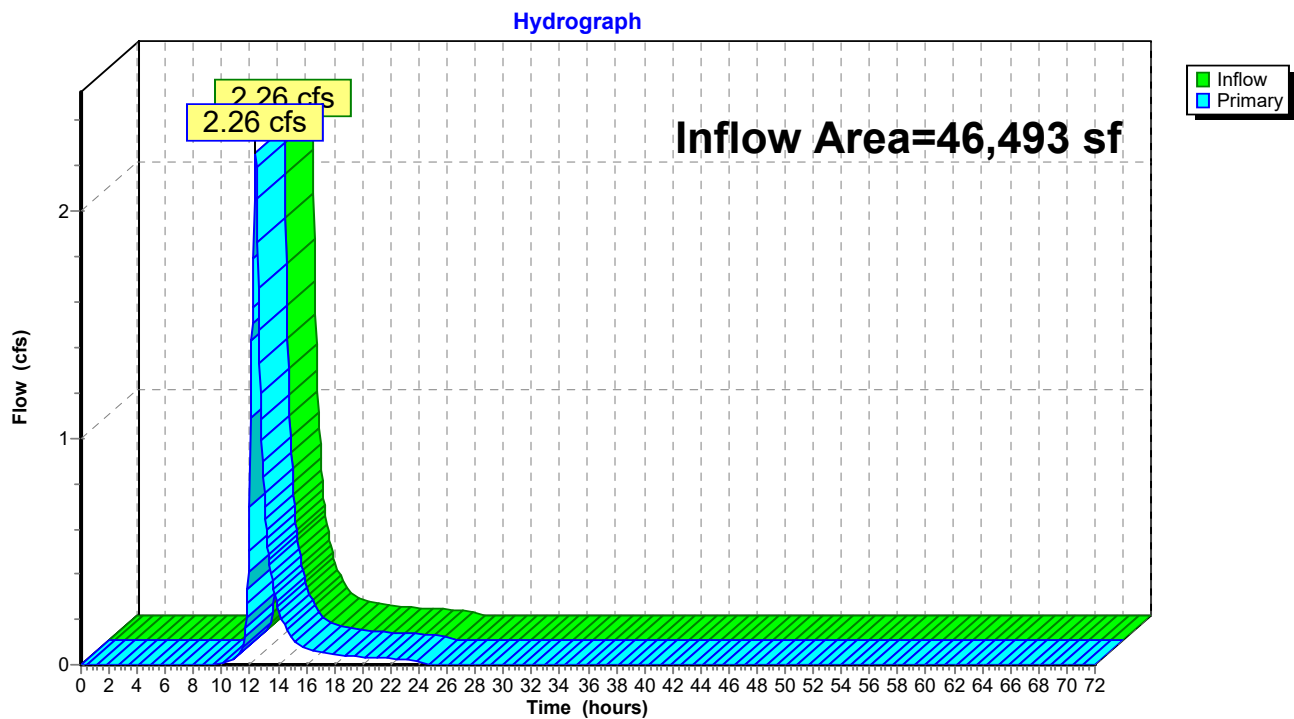


Summary for Link 23L: (Combined Link)

Inflow Area = 46,493 sf, 9.37% Impervious, Inflow Depth = 2.47" for 10-Year event
 Inflow = 2.26 cfs @ 12.40 hrs, Volume= 9,581 cf
 Primary = 2.26 cfs @ 12.40 hrs, Volume= 9,581 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Link 23L: (Combined Link)



737WashingtonSt - 10000 sf office

Type III 24-hr 25-Year Rainfall=5.60"

Prepared by {enter your company name here}

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Page 49

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment13S: POST (4) Runoff Area=17,504 sf 100.00% Impervious Runoff Depth=5.36"
 Tc=6.0 min CN=98 Runoff=2.15 cfs 7,822 cf

Subcatchment15S: PRE (3) Runoff Area=97,629 sf 0.00% Impervious Runoff Depth=2.49"
 Flow Length=382' Tc=9.7 min CN=70 Runoff=5.64 cfs 20,270 cf

Subcatchment16S: POST (7) Runoff Area=13,121 sf 33.19% Impervious Runoff Depth=3.62"
 Tc=6.0 min CN=82 Runoff=1.24 cfs 3,959 cf

Subcatchment18S: POST (5) Runoff Area=10,000 sf 100.00% Impervious Runoff Depth=5.36"
 Tc=6.0 min CN=98 Runoff=1.23 cfs 4,469 cf

Subcatchment19S: POST (6) Runoff Area=30,906 sf 0.00% Impervious Runoff Depth=2.85"
 Flow Length=180' Tc=9.2 min CN=74 Runoff=2.09 cfs 7,344 cf

Subcatchment20S: POST (8) Runoff Area=33,372 sf 0.00% Impervious Runoff Depth=2.49"
 Flow Length=134' Tc=8.0 min CN=70 Runoff=2.04 cfs 6,929 cf

Pond 21P: Basin #2 Peak Elev=76.54' Storage=1,513 cf Inflow=3.14 cfs 10,913 cf
 Discarded=0.10 cfs 3,058 cf Primary=0.86 cfs 2,117 cf Tertiary=2.02 cfs 5,738 cf Outflow=2.98 cfs 10,913 cf

Pond 24P: Basin #1 Peak Elev=78.63' Storage=4,588 cf Inflow=5.19 cfs 16,573 cf
 Discarded=0.19 cfs 9,619 cf Tertiary=2.56 cfs 6,954 cf Outflow=2.75 cfs 16,573 cf

Pond 25P: Subsurface infiltration#1 Peak Elev=83.73' Storage=1,229 cf Inflow=1.23 cfs 4,469 cf
 Discarded=0.05 cfs 3,062 cf Primary=1.06 cfs 1,407 cf Outflow=1.11 cfs 4,469 cf

Pond 27P: Subsurface infiltration#2 Peak Elev=0.00' Storage=0 cf
 Discarded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Link 23L: (Combined Link) Inflow=3.95 cfs 14,784 cf
 Primary=3.95 cfs 14,784 cf

Total Runoff Area = 202,532 sf Runoff Volume = 50,793 cf Average Runoff Depth = 3.01"
84.27% Pervious = 170,673 sf 15.73% Impervious = 31,859 sf

Summary for Subcatchment 13S: POST (4)

Runoff = 2.15 cfs @ 12.09 hrs, Volume= 7,822 cf, Depth= 5.36"

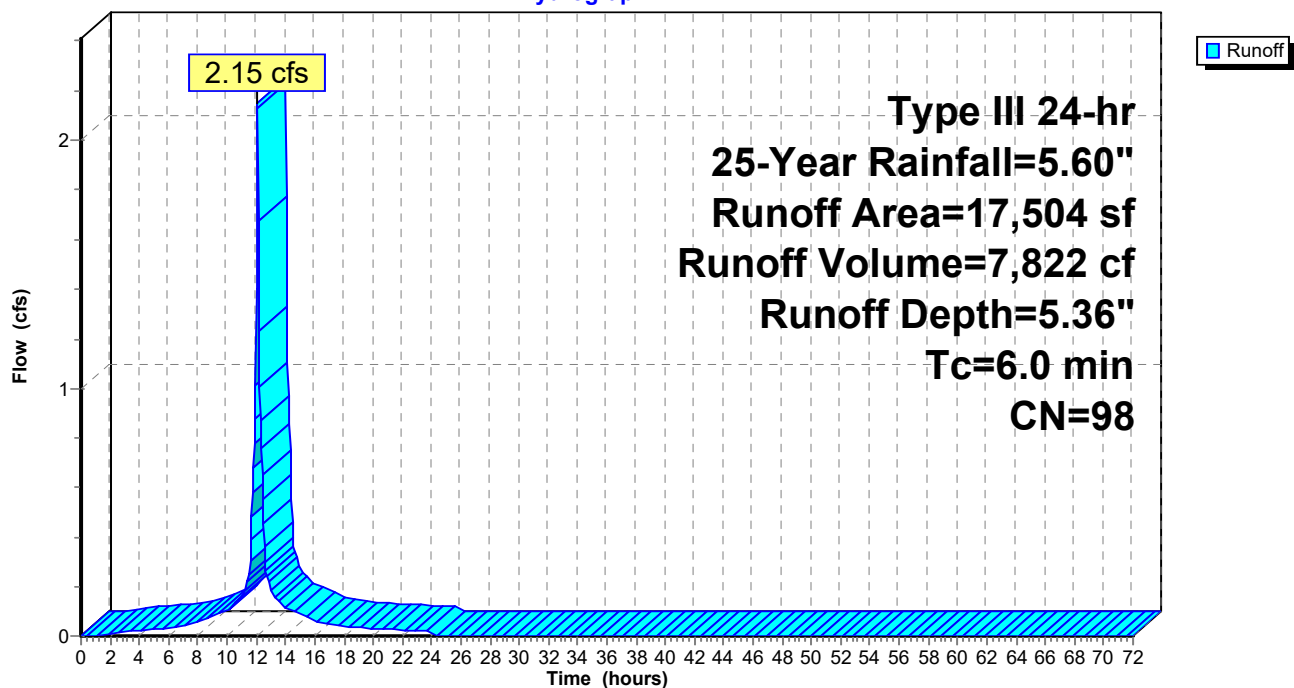
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.60"

Area (sf)	CN	Description
17,504	98	Paved parking, HSG C
17,504	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, runoff from pavement

Subcatchment 13S: POST (4)

Hydrograph



Summary for Subcatchment 15S: PRE (3)

Runoff = 5.64 cfs @ 12.14 hrs, Volume= 20,270 cf, Depth= 2.49"

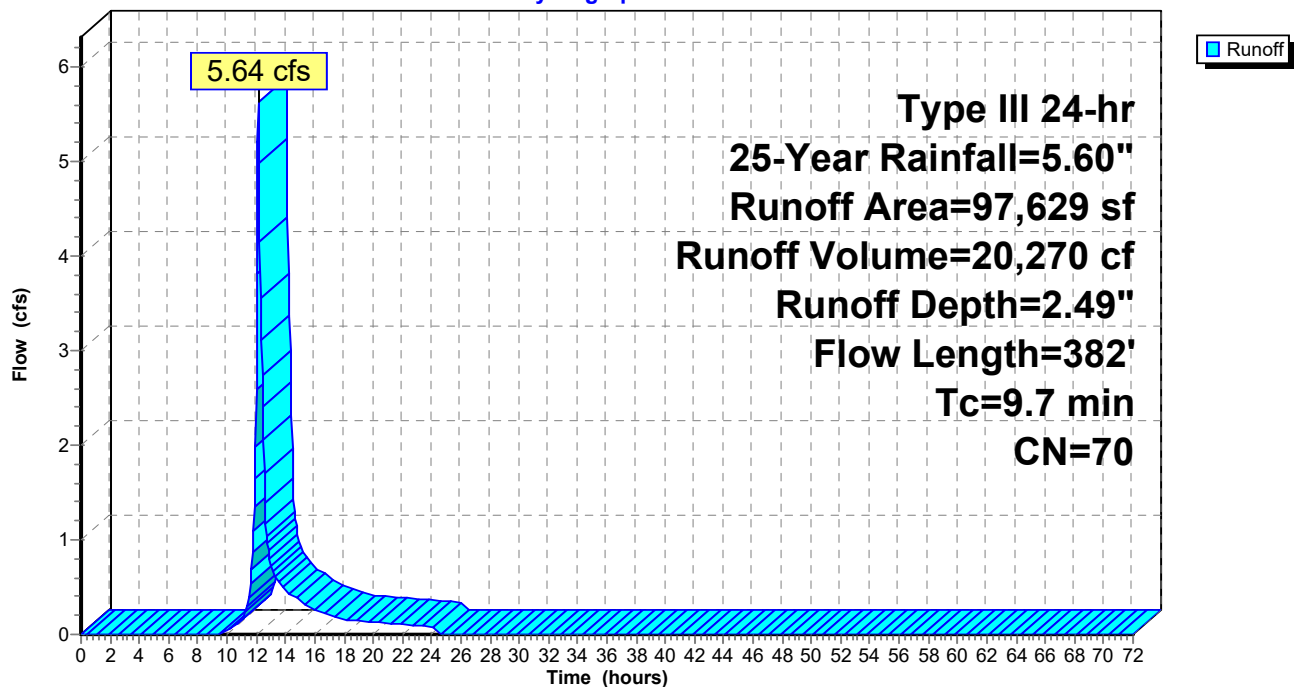
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.60"

Area (sf)	CN	Description
97,629	70	Woods, Good, HSG C
97,629	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.2000	0.17		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
2.6	138	0.0320	0.89		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.8	146	0.0710	1.33		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.5	48	0.0850	1.46		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
9.7	382	Total			

Subcatchment 15S: PRE (3)

Hydrograph



Summary for Subcatchment 16S: POST (7)

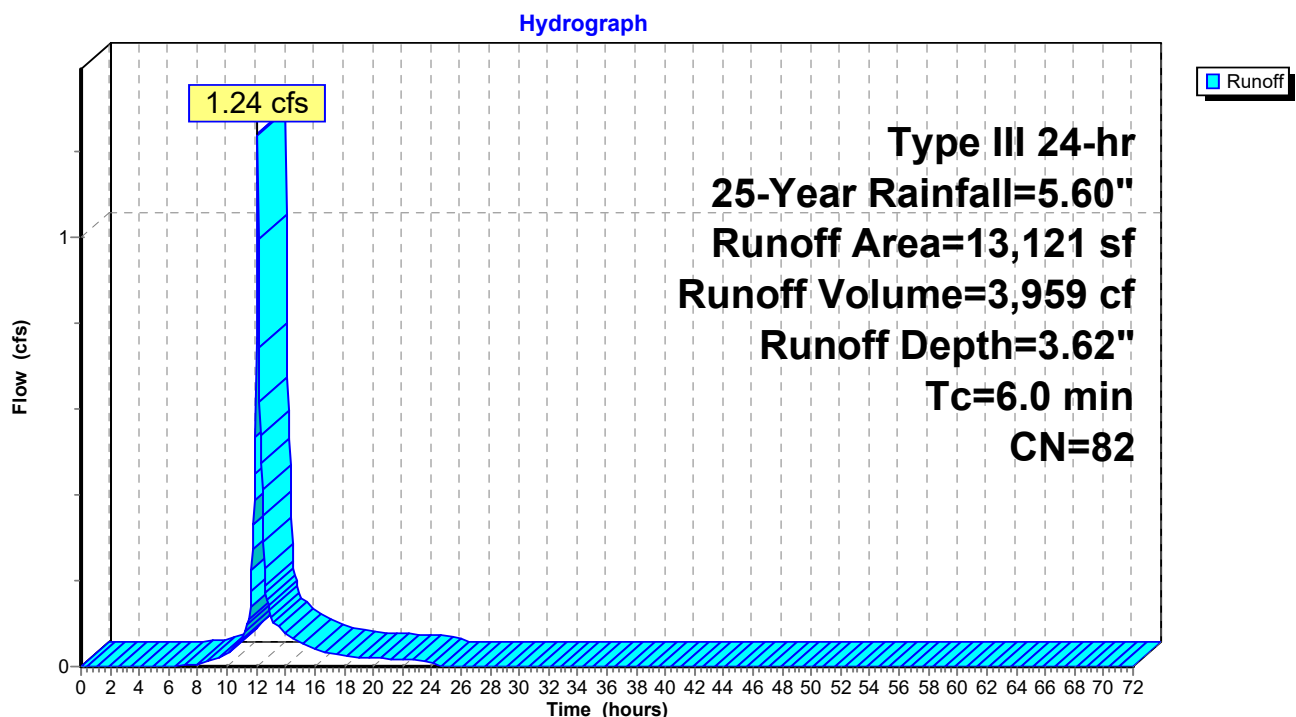
Runoff = 1.24 cfs @ 12.09 hrs, Volume= 3,959 cf, Depth= 3.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.60"

Area (sf)	CN	Description
8,766	74	>75% Grass cover, Good, HSG C
4,355	98	Paved parking, HSG C
13,121	82	Weighted Average
8,766	74	66.81% Pervious Area
4,355	98	33.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 16S: POST (7)



Summary for Subcatchment 18S: POST (5)

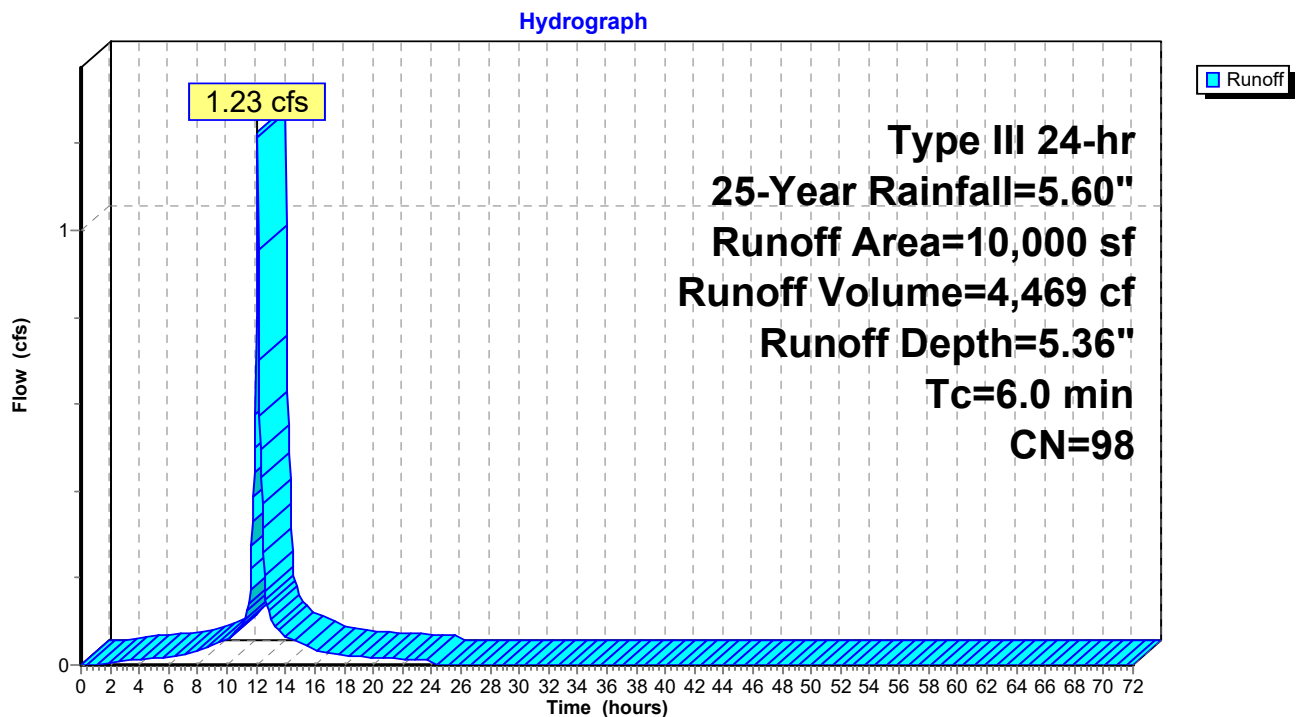
Runoff = 1.23 cfs @ 12.09 hrs, Volume= 4,469 cf, Depth= 5.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.60"

Area (sf)	CN	Description
10,000	98	Roofs, HSG C
10,000	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 18S: POST (5)



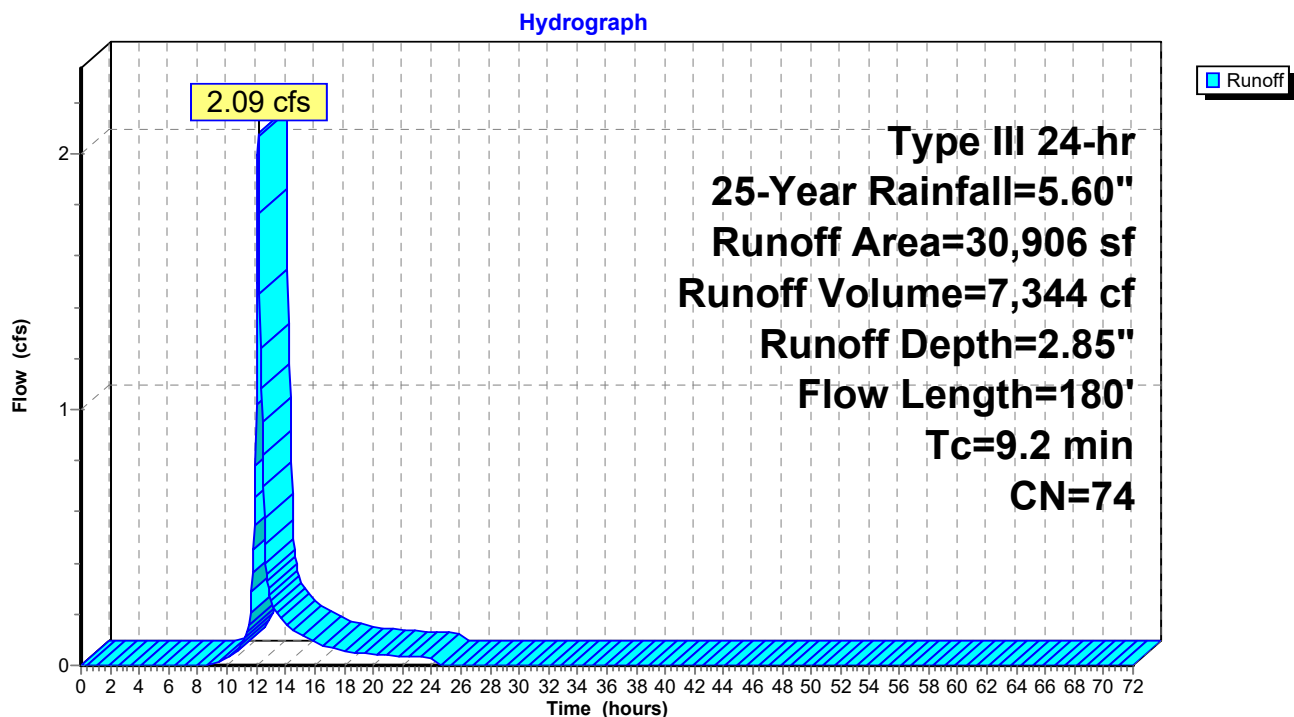
Summary for Subcatchment 19S: POST (6)

Runoff = 2.09 cfs @ 12.14 hrs, Volume= 7,344 cf, Depth= 2.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.60"

Area (sf)	CN	Description
30,906	74	>75% Grass cover, Good, HSG C
30,906	74	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	50	0.0200	0.10		Sheet Flow, Grass: Dense n= 0.240 P2= 3.40"
0.9	93	0.0600	1.71		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.3	37	0.1200	2.42		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
9.2	180	Total			

Subcatchment 19S: POST (6)

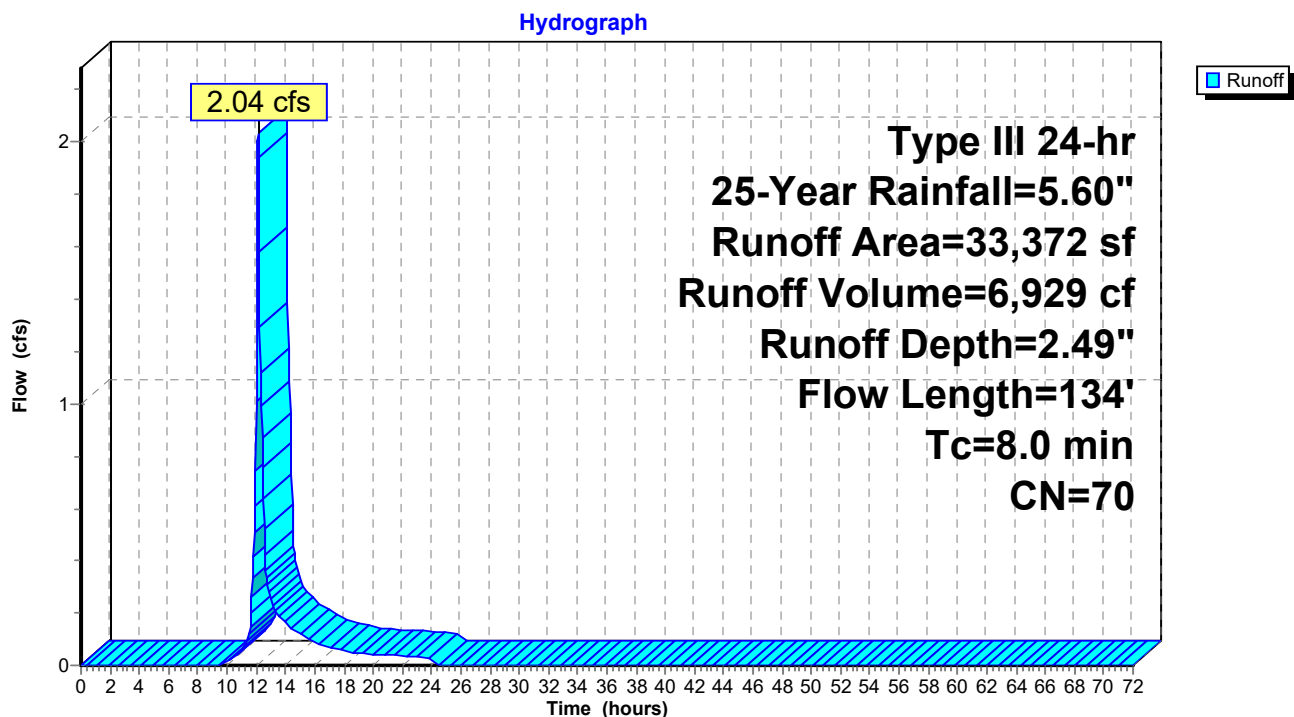
Summary for Subcatchment 20S: POST (8)

Runoff = 2.04 cfs @ 12.12 hrs, Volume= 6,929 cf, Depth= 2.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.60"

Area (sf)	CN	Description
1,400	74	>75% Grass cover, Good, HSG C
31,972	70	Woods, Good, HSG C
33,372	70	Weighted Average
33,372	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0800	0.12		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
1.1	84	0.0700	1.32		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
8.0	134	Total			

Subcatchment 20S: POST (8)

Summary for Pond 21P: Basin #2

Inflow Area = 13,121 sf, 33.19% Impervious, Inflow Depth = 9.98" for 25-Year event
 Inflow = 3.14 cfs @ 12.26 hrs, Volume= 10,913 cf
 Outflow = 2.98 cfs @ 12.34 hrs, Volume= 10,913 cf, Atten= 5%, Lag= 4.8 min
 Discarded = 0.10 cfs @ 12.34 hrs, Volume= 3,058 cf
 Primary = 0.86 cfs @ 12.34 hrs, Volume= 2,117 cf
 Tertiary = 2.02 cfs @ 12.34 hrs, Volume= 5,738 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 76.54' @ 12.34 hrs Surf.Area= 1,727 sf Storage= 1,513 cf

Plug-Flow detention time= 28.4 min calculated for 10,905 cf (100% of inflow)
 Center-of-Mass det. time= 28.4 min (813.3 - 784.9)

Volume	Invert	Avail.Storage	Storage Description
#1	75.50'	4,644 cf	stage (Conic) Listed below

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
75.50	1,179	0	0	1,179
76.00	1,429	651	651	1,437
76.50	1,702	782	1,433	1,719
77.00	1,988	922	2,354	2,015
77.50	2,288	1,068	3,422	2,326
78.00	2,603	1,222	4,644	2,653

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	76.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height
#3	Primary	76.00'	9.0" Vert. Orifice/Grate C= 0.600

Discarded OutFlow Max=0.10 cfs @ 12.34 hrs HW=76.54' (Free Discharge)

↑**1=Exfiltration** (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.86 cfs @ 12.34 hrs HW=76.54' (Free Discharge)

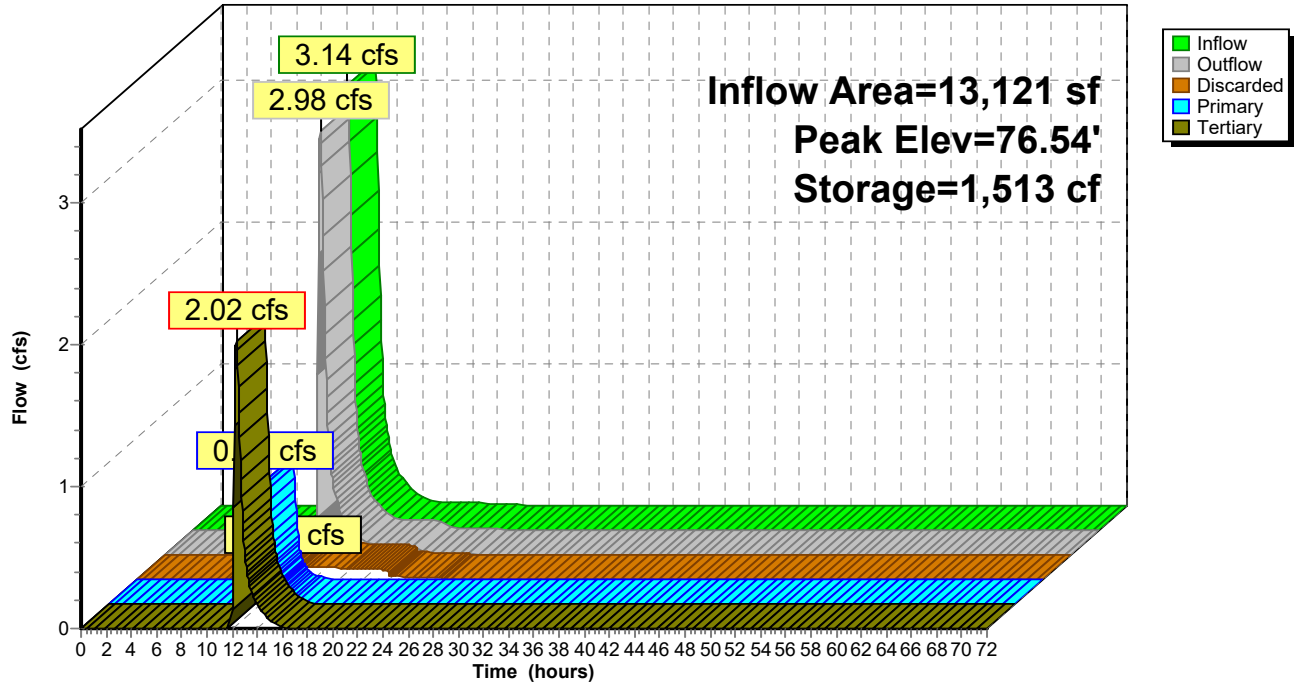
↑**3=Orifice/Grate** (Orifice Controls 0.86 cfs @ 2.51 fps)

Tertiary OutFlow Max=2.02 cfs @ 12.34 hrs HW=76.54' (Free Discharge)

↑**2=Sharp-Crested Rectangular Weir** (Weir Controls 2.02 cfs @ 2.57 fps)

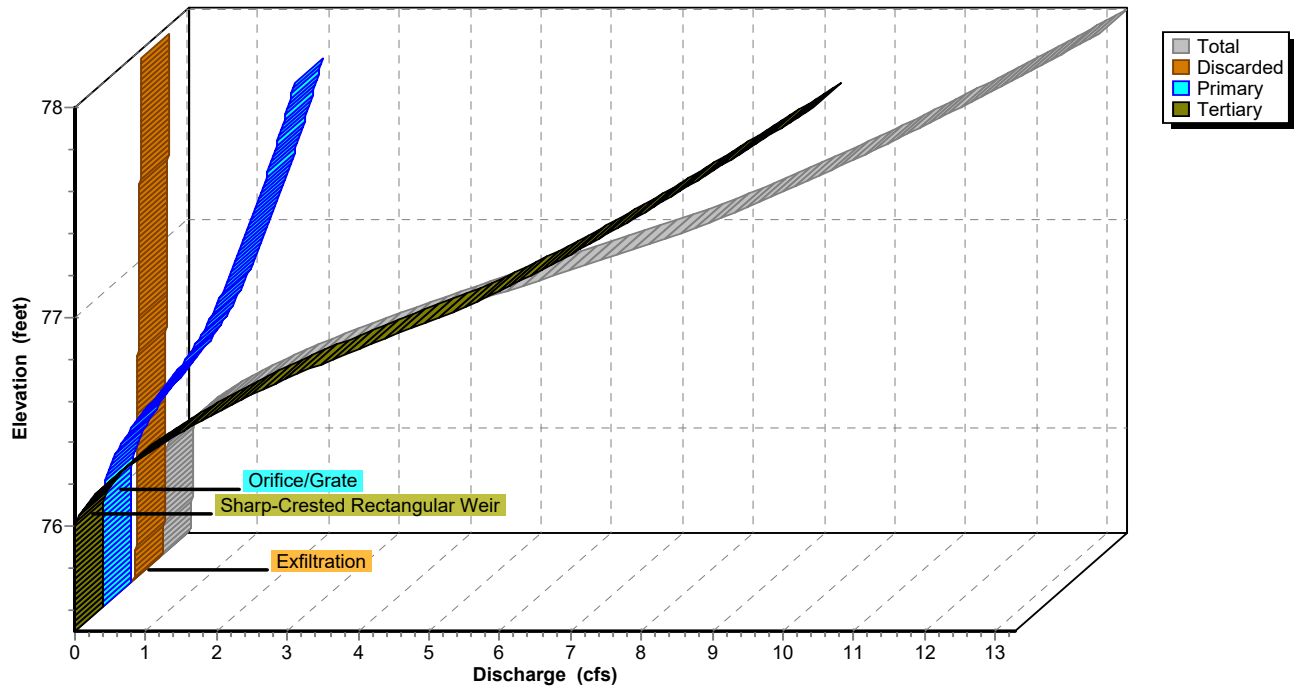
Pond 21P: Basin #2

Hydrograph



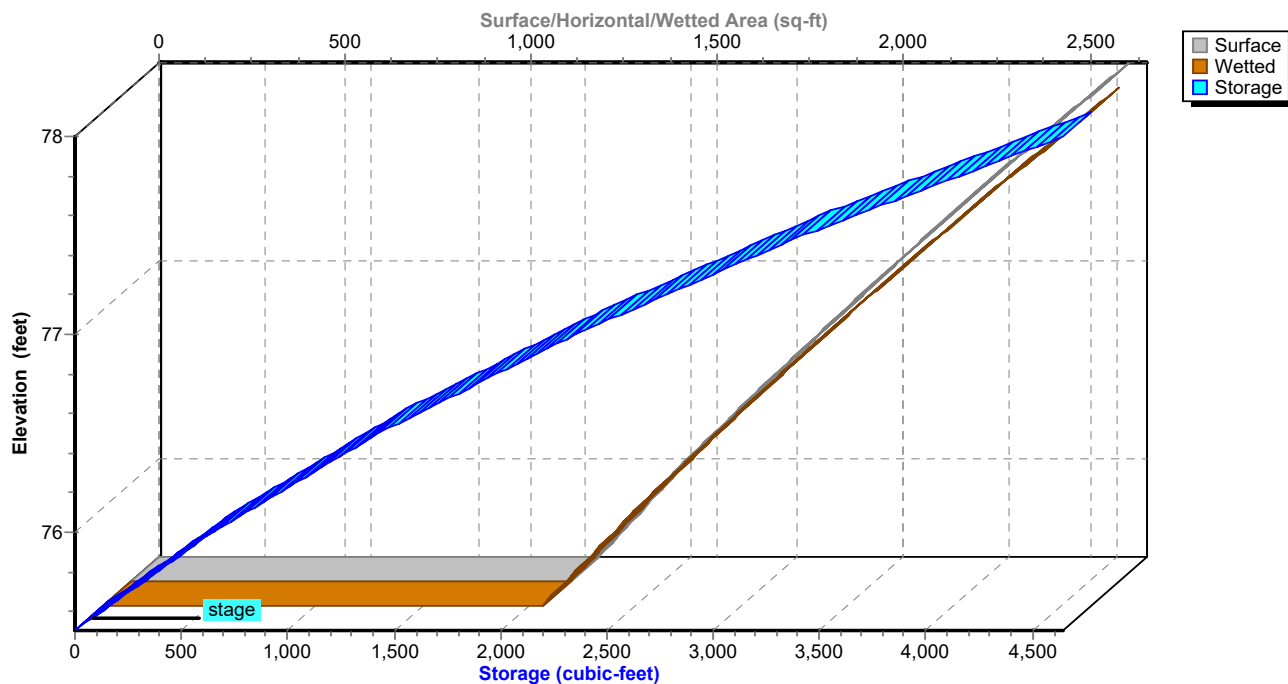
Pond 21P: Basin #2

Stage-Discharge



Pond 21P: Basin #2

Stage-Area-Storage



Summary for Pond 24P: Basin #1

Inflow Area = 58,410 sf, 47.09% Impervious, Inflow Depth = 3.40" for 25-Year event
 Inflow = 5.19 cfs @ 12.11 hrs, Volume= 16,573 cf
 Outflow = 2.75 cfs @ 12.29 hrs, Volume= 16,573 cf, Atten= 47%, Lag= 10.6 min
 Discarded = 0.19 cfs @ 12.29 hrs, Volume= 9,619 cf
 Tertiary = 2.56 cfs @ 12.29 hrs, Volume= 6,954 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 78.63' @ 12.29 hrs Surf.Area= 3,446 sf Storage= 4,588 cf

Plug-Flow detention time= 92.5 min calculated for 16,561 cf (100% of inflow)
 Center-of-Mass det. time= 92.5 min (878.4 - 785.9)

Volume	Invert	Avail.Storage	Storage Description
#1	77.10'	9,844 cf	stage (Conic) Listed below

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
77.10	2,446	0	0	2,446
78.00	3,036	2,462	2,462	3,059
79.00	3,682	3,354	5,816	3,737
80.00	4,385	4,028	9,844	4,475

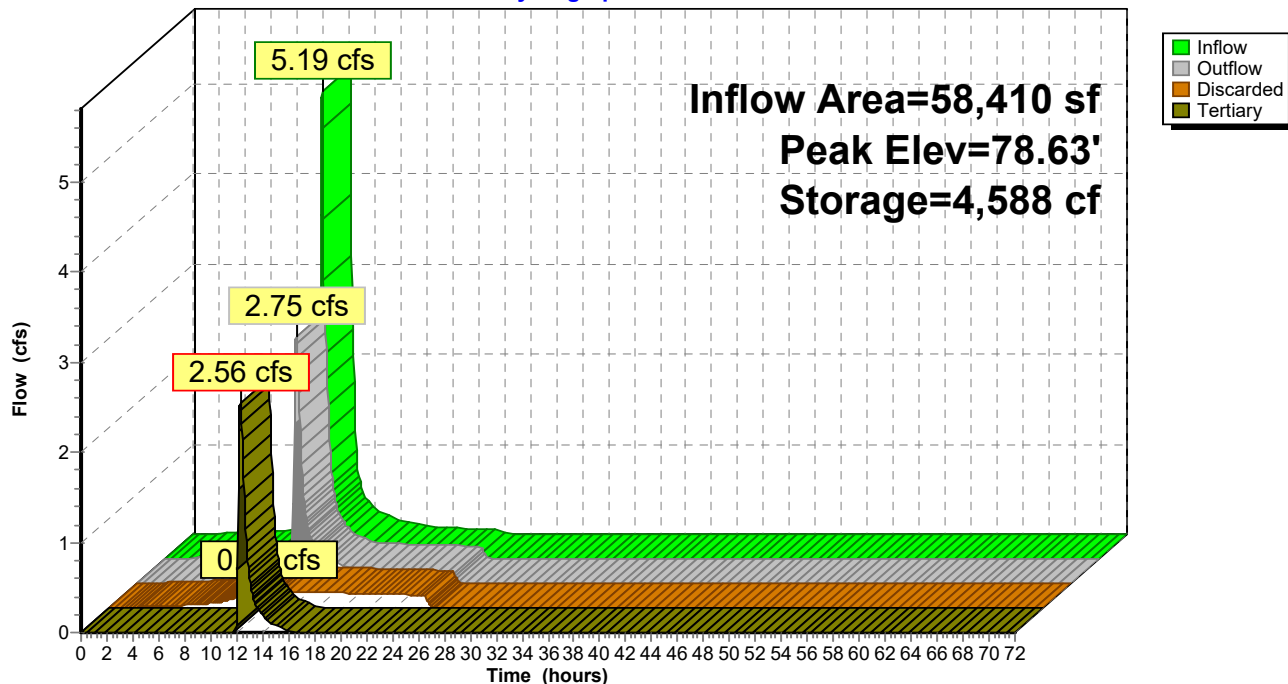
Device	Routing	Invert	Outlet Devices
#1	Discarded	77.10'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	78.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height

Discarded OutFlow Max=0.19 cfs @ 12.29 hrs HW=78.63' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Tertiary OutFlow Max=2.55 cfs @ 12.29 hrs HW=78.63' (Free Discharge)
 ↑ **2=Sharp-Crested Rectangular Weir** (Weir Controls 2.55 cfs @ 2.80 fps)

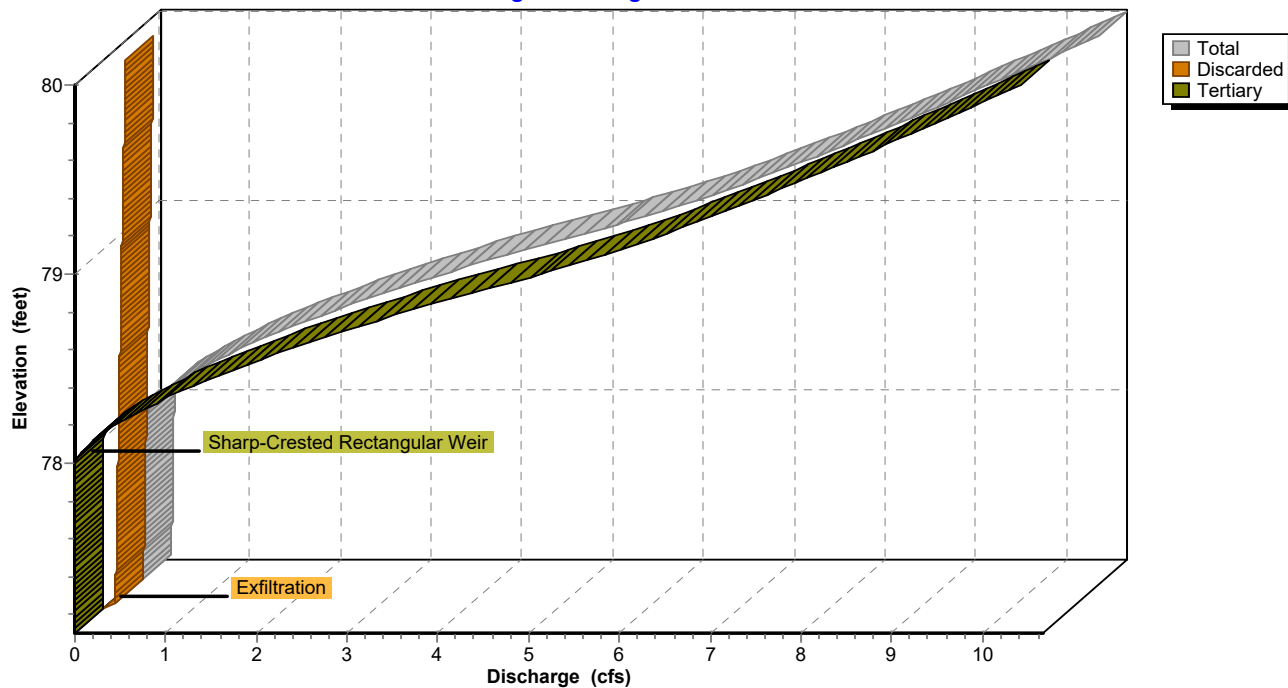
Pond 24P: Basin #1

Hydrograph



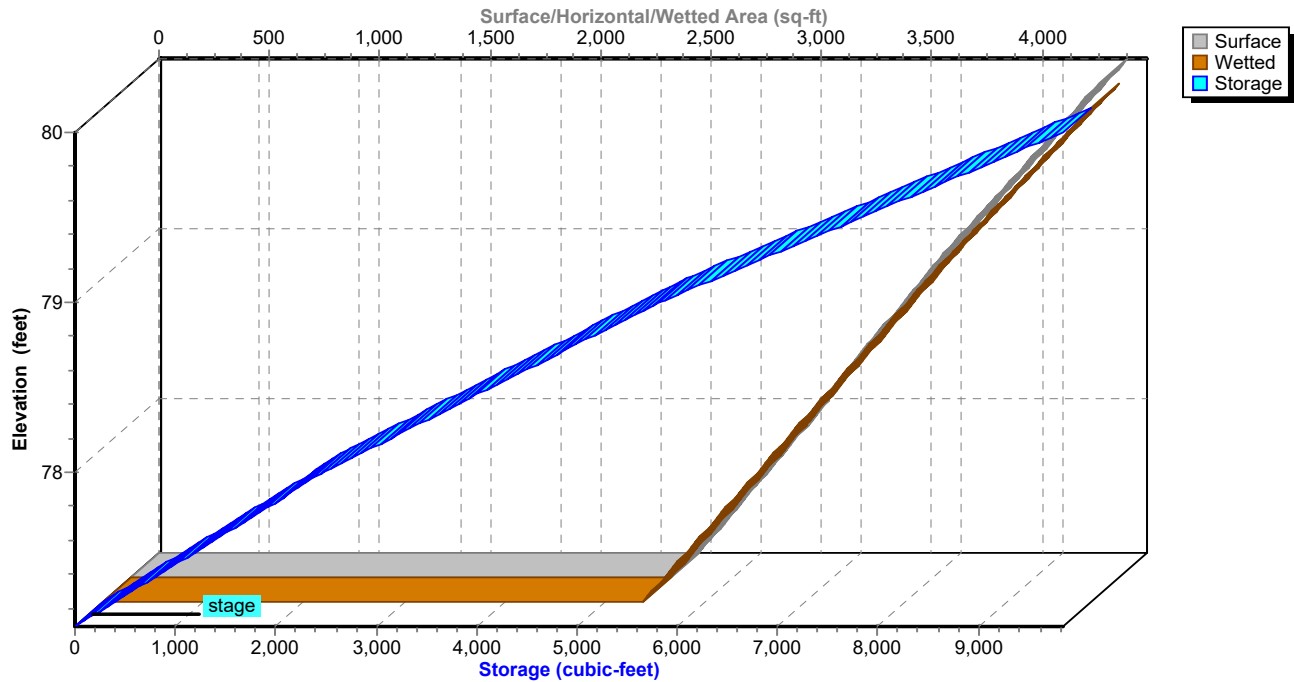
Pond 24P: Basin #1

Stage-Discharge



Pond 24P: Basin #1

Stage-Area-Storage



Summary for Pond 25P: Subsurface infiltration#1

Inflow Area = 10,000 sf, 100.00% Impervious, Inflow Depth = 5.36" for 25-Year event
 Inflow = 1.23 cfs @ 12.09 hrs, Volume= 4,469 cf
 Outflow = 1.11 cfs @ 12.13 hrs, Volume= 4,469 cf, Atten= 9%, Lag= 2.7 min
 Discarded = 0.05 cfs @ 12.13 hrs, Volume= 3,062 cf
 Primary = 1.06 cfs @ 12.13 hrs, Volume= 1,407 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 83.73' @ 12.13 hrs Surf.Area= 461 sf Storage= 1,229 cf

Plug-Flow detention time= 156.5 min calculated for 4,466 cf (100% of inflow)
 Center-of-Mass det. time= 156.6 min (902.8 - 746.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	79.00'	637 cf	12.80'W x 36.00'L x 5.50'H Field A 2,534 cf Overall - 942 cf Embedded = 1,592 cf x 40.0% Voids
#2A	80.00'	710 cf	Concrete Galley 4x4x4 x 16 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 2 Rows of 8 Chambers
		1,346 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	79.00'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	83.00'	8.0" Vert. Orifice/Grate C= 0.600

Discarded OutFlow Max=0.05 cfs @ 12.13 hrs HW=83.71' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=1.03 cfs @ 12.13 hrs HW=83.71' (Free Discharge)
 ↑ **2=Orifice/Grate** (Orifice Controls 1.03 cfs @ 2.94 fps)

Pond 25P: Subsurface infiltration#1 - Chamber Wizard Field A

Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

8 Chambers/Row x 4.00' Long = 32.00' Row Length +24.0" End Stone x 2 = 36.00' Base Length

2 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 12.80' Base Width

12.0" Base + 48.0" Chamber Height + 6.0" Cover = 5.50' Field Height

16 Chambers x 44.3 cf = 709.5 cf Chamber Storage

16 Chambers x 58.9 cf = 942.0 cf Displacement

2,534.4 cf Field - 942.0 cf Chambers = 1,592.4 cf Stone x 40.0% Voids = 637.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,346.5 cf = 0.031 af

Overall Storage Efficiency = 53.1%

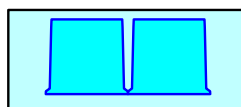
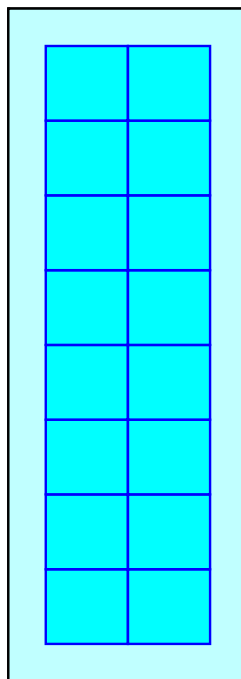
Overall System Size = 36.00' x 12.80' x 5.50'

16 Chambers @ \$ 300.00 /ea = \$ 4,800.00

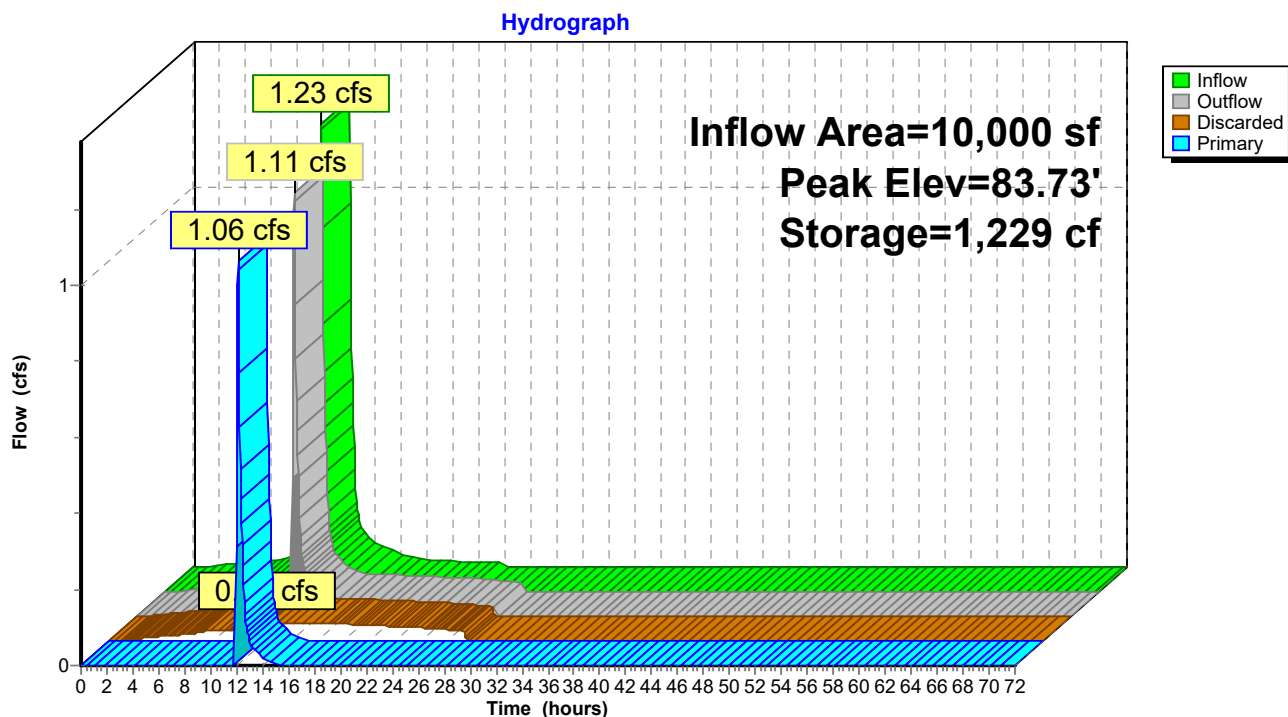
93.9 cy Field Excavation @ \$ 10.00 /cy = \$ 938.67

59.0 cy Stone @ \$ 30.00 /cy = \$ 1,769.31

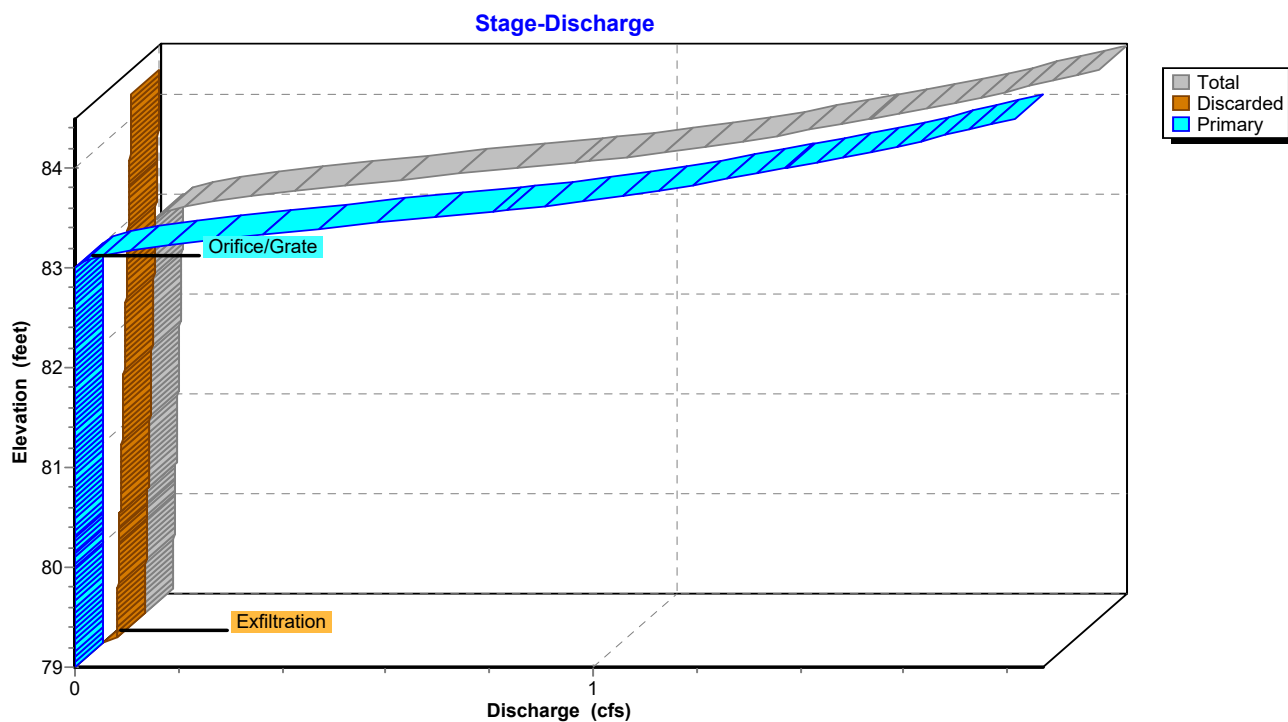
Total Cost = \$ 7,507.98



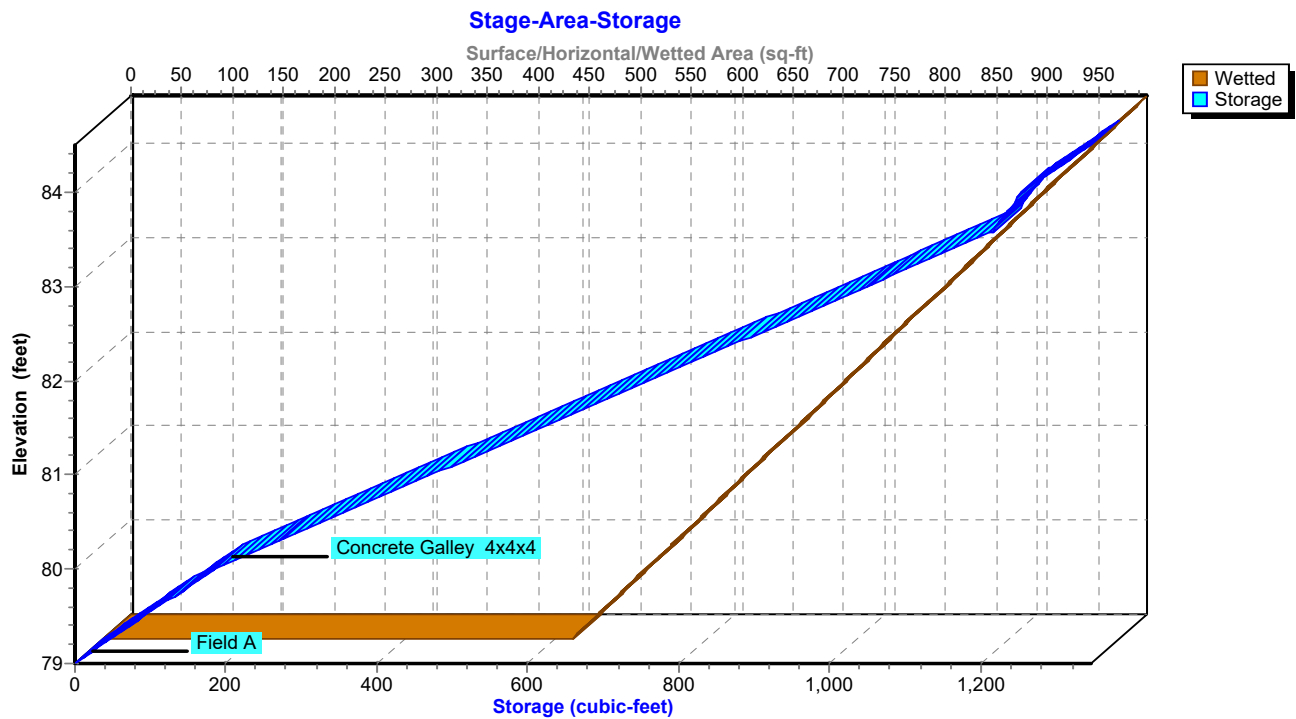
Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Summary for Pond 27P: Subsurface infiltration#2

[43] Hint: Has no inflow (Outflow=Zero)

Volume	Invert	Avail.Storage	Storage Description
#1A	75.50'	2,584 cf	21.60'W x 88.00'L x 6.00'H Field A 11,405 cf Overall - 4,946 cf Embedded = 6,459 cf x 40.0% Voids
#2A	76.50'	3,725 cf	Concrete Galley 4x4x4 x 84 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 4 Rows of 21 Chambers
#3	81.50'	15 cf	Custom Stage Data (Prismatic) Listed below (Recalc) -Impervious
		6,324 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.50	6	0	0
84.00	6	15	15

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	80.00'	6.0" Vert. Orifice/Grate C= 0.600
#3	Primary	78.75'	6.0" Vert. Orifice/Grate C= 0.600
#4	Primary	78.10'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)↑**1=Exfiltration** (Controls 0.00 cfs)**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)↑**2=Orifice/Grate** (Controls 0.00 cfs)↑**3=Orifice/Grate** (Controls 0.00 cfs)↑**4=Orifice/Grate** (Controls 0.00 cfs)

Pond 27P: Subsurface infiltration#2 - Chamber Wizard Field A

Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

21 Chambers/Row x 4.00' Long = 84.00' Row Length +24.0" End Stone x 2 = 88.00' Base Length

4 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 21.60' Base Width

12.0" Base + 48.0" Chamber Height + 12.0" Cover = 6.00' Field Height

84 Chambers x 44.3 cf = 3,725.0 cf Chamber Storage

84 Chambers x 58.9 cf = 4,945.6 cf Displacement

11,404.8 cf Field - 4,945.6 cf Chambers = 6,459.2 cf Stone x 40.0% Voids = 2,583.7 cf Stone Storage

Chamber Storage + Stone Storage = 6,308.6 cf = 0.145 af

Overall Storage Efficiency = 55.3%

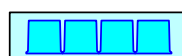
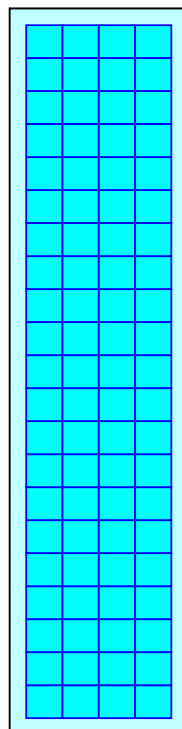
Overall System Size = 88.00' x 21.60' x 6.00'

84 Chambers @ \$ 300.00 /ea = \$ 25,200.00

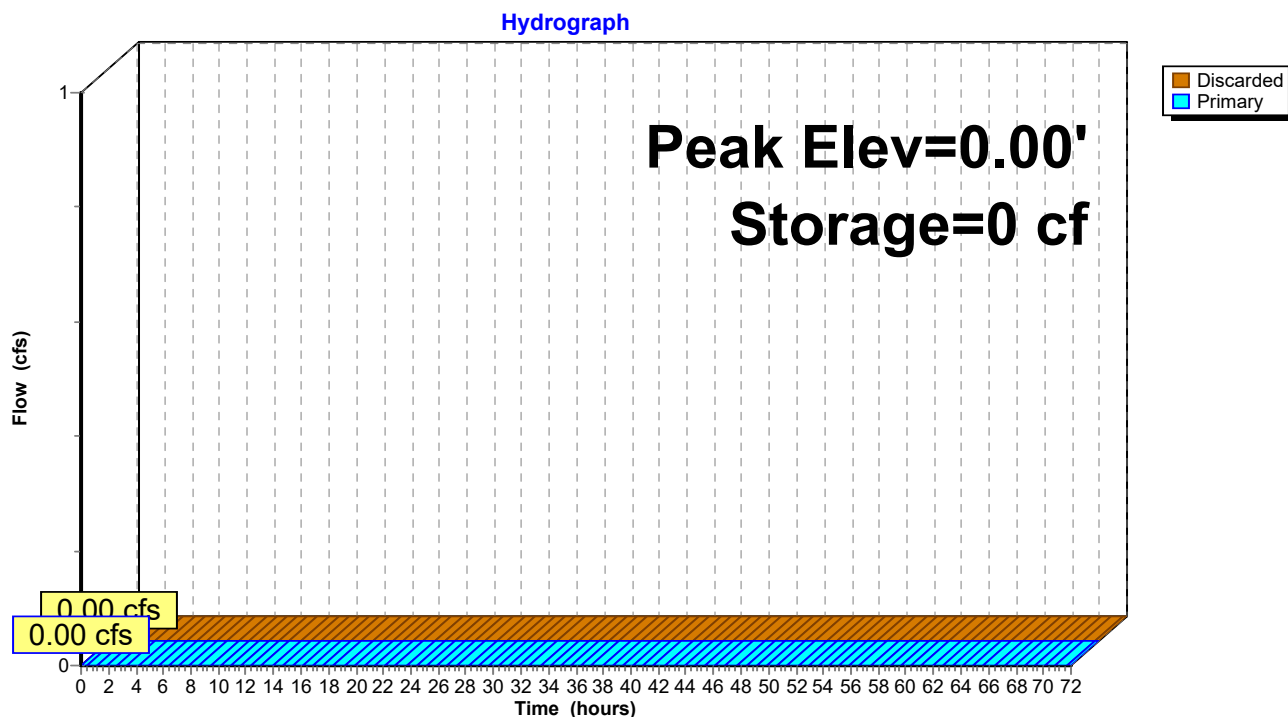
422.4 cy Field Excavation @ \$ 10.00 /cy = \$ 4,224.00

239.2 cy Stone @ \$ 30.00 /cy = \$ 7,176.88

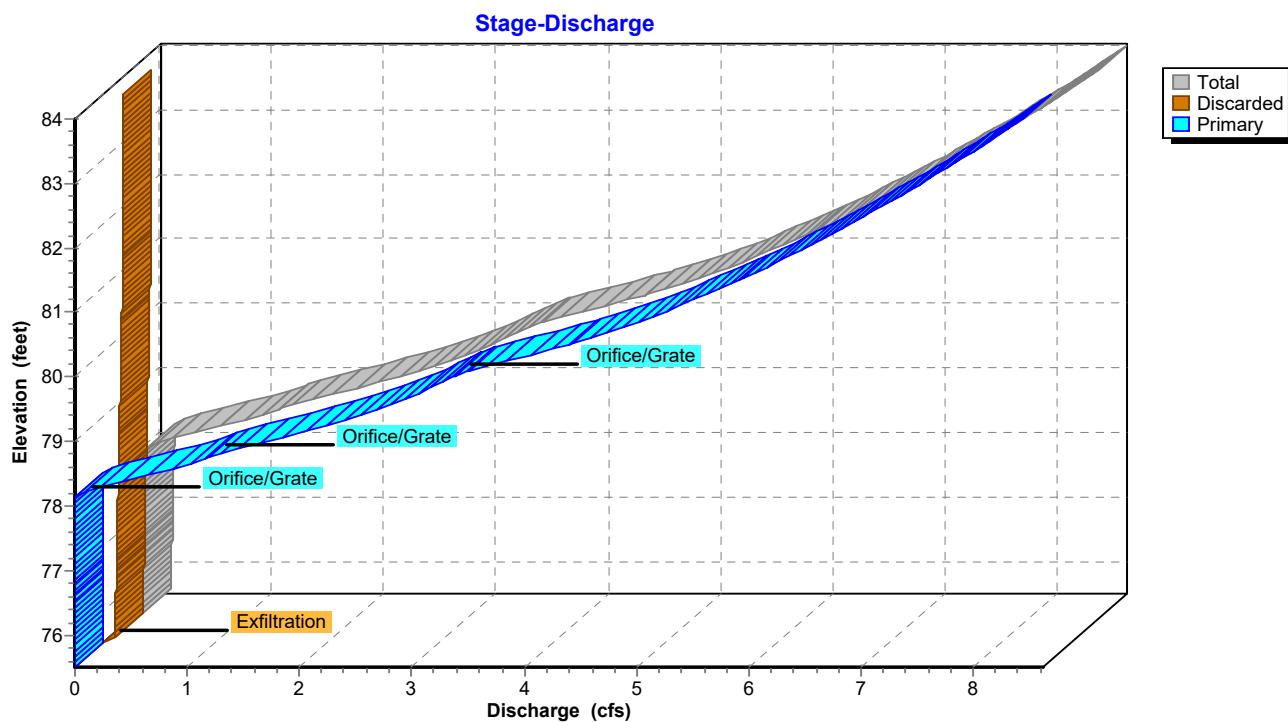
Total Cost = \$ 36,600.88



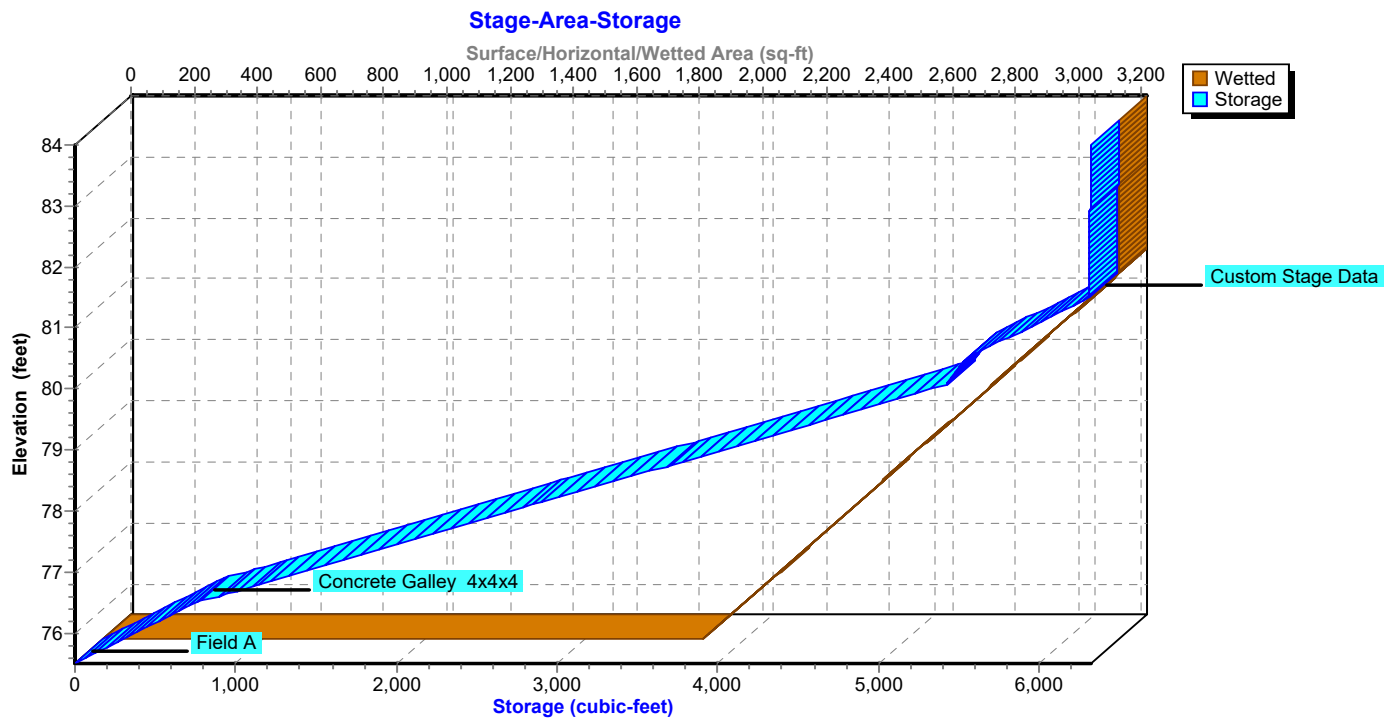
Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2

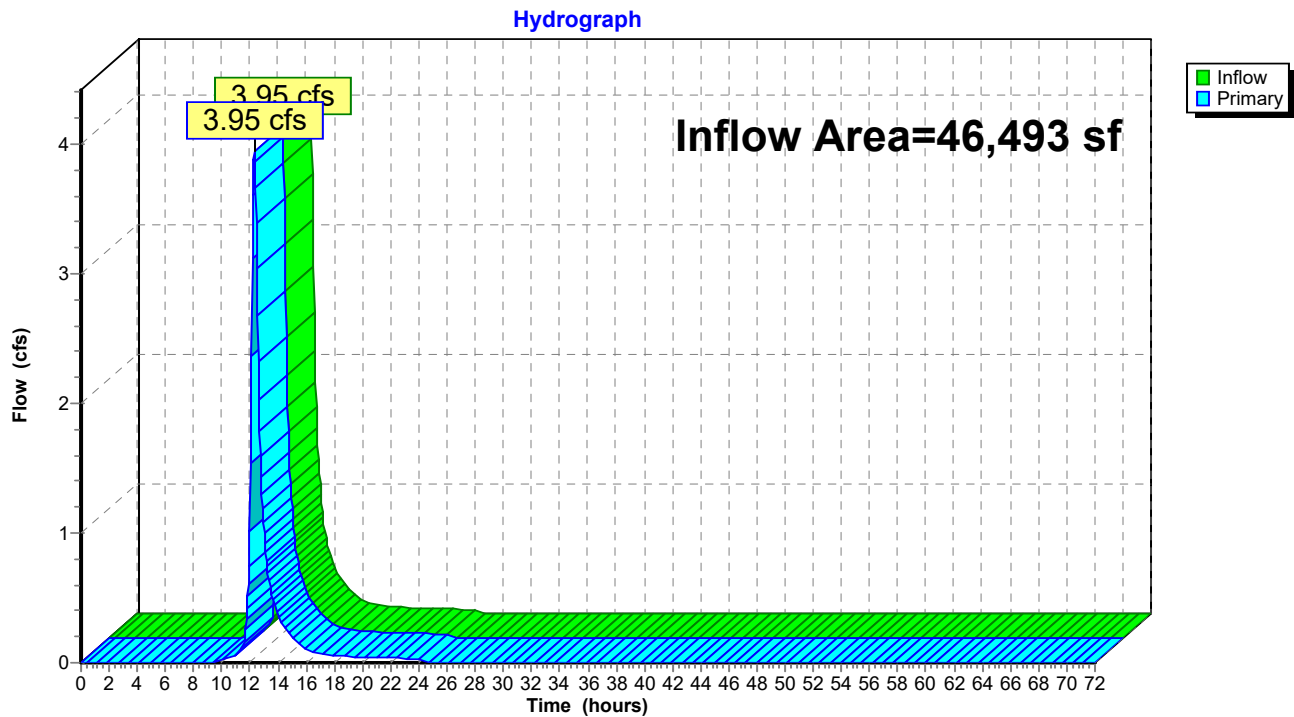


Summary for Link 23L: (Combined Link)

Inflow Area = 46,493 sf, 9.37% Impervious, Inflow Depth = 3.82" for 25-Year event
 Inflow = 3.95 cfs @ 12.30 hrs, Volume= 14,784 cf
 Primary = 3.95 cfs @ 12.30 hrs, Volume= 14,784 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Link 23L: (Combined Link)



737WashingtonSt - 10000 sf office

Type III 24-hr 100-Year Rainfall=7.00"

Prepared by {enter your company name here}

Printed 10/29/2021

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Page 71

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment13S: POST (4) Runoff Area=17,504 sf 100.00% Impervious Runoff Depth=6.76"
 Tc=6.0 min CN=98 Runoff=2.69 cfs 9,862 cf

Subcatchment15S: PRE (3) Runoff Area=97,629 sf 0.00% Impervious Runoff Depth=3.62"
 Flow Length=382' Tc=9.7 min CN=70 Runoff=8.26 cfs 29,438 cf

Subcatchment16S: POST (7) Runoff Area=13,121 sf 33.19% Impervious Runoff Depth=4.92"
 Tc=6.0 min CN=82 Runoff=1.67 cfs 5,375 cf

Subcatchment18S: POST (5) Runoff Area=10,000 sf 100.00% Impervious Runoff Depth=6.76"
 Tc=6.0 min CN=98 Runoff=1.54 cfs 5,634 cf

Subcatchment19S: POST (6) Runoff Area=30,906 sf 0.00% Impervious Runoff Depth=4.04"
 Flow Length=180' Tc=9.2 min CN=74 Runoff=2.96 cfs 10,410 cf

Subcatchment20S: POST (8) Runoff Area=33,372 sf 0.00% Impervious Runoff Depth=3.62"
 Flow Length=134' Tc=8.0 min CN=70 Runoff=2.99 cfs 10,063 cf

Pond 21P: Basin #2 Peak Elev=76.80' Storage=1,987 cf Inflow=5.46 cfs 17,130 cf
 Discarded=0.11 cfs 3,574 cf Primary=1.39 cfs 3,675 cf Tertiary=3.65 cfs 9,881 cf Outflow=5.15 cfs 17,130 cf

Pond 24P: Basin #1 Peak Elev=78.92' Storage=5,543 cf Inflow=6.94 cfs 22,557 cf
 Discarded=0.21 cfs 10,802 cf Tertiary=4.51 cfs 11,755 cf Outflow=4.71 cfs 22,557 cf

Pond 25P: Subsurface infiltration#1 Peak Elev=84.05' Storage=1,263 cf Inflow=1.54 cfs 5,634 cf
 Discarded=0.05 cfs 3,350 cf Primary=1.42 cfs 2,284 cf Outflow=1.47 cfs 5,634 cf

Pond 27P: Subsurface infiltration#2 Peak Elev=0.00' Storage=0 cf
 Discarded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

Link 23L: (Combined Link) Inflow=6.99 cfs 23,618 cf
 Primary=6.99 cfs 23,618 cf

Total Runoff Area = 202,532 sf Runoff Volume = 70,783 cf Average Runoff Depth = 4.19"
84.27% Pervious = 170,673 sf 15.73% Impervious = 31,859 sf

Summary for Subcatchment 13S: POST (4)

Runoff = 2.69 cfs @ 12.09 hrs, Volume= 9,862 cf, Depth= 6.76"

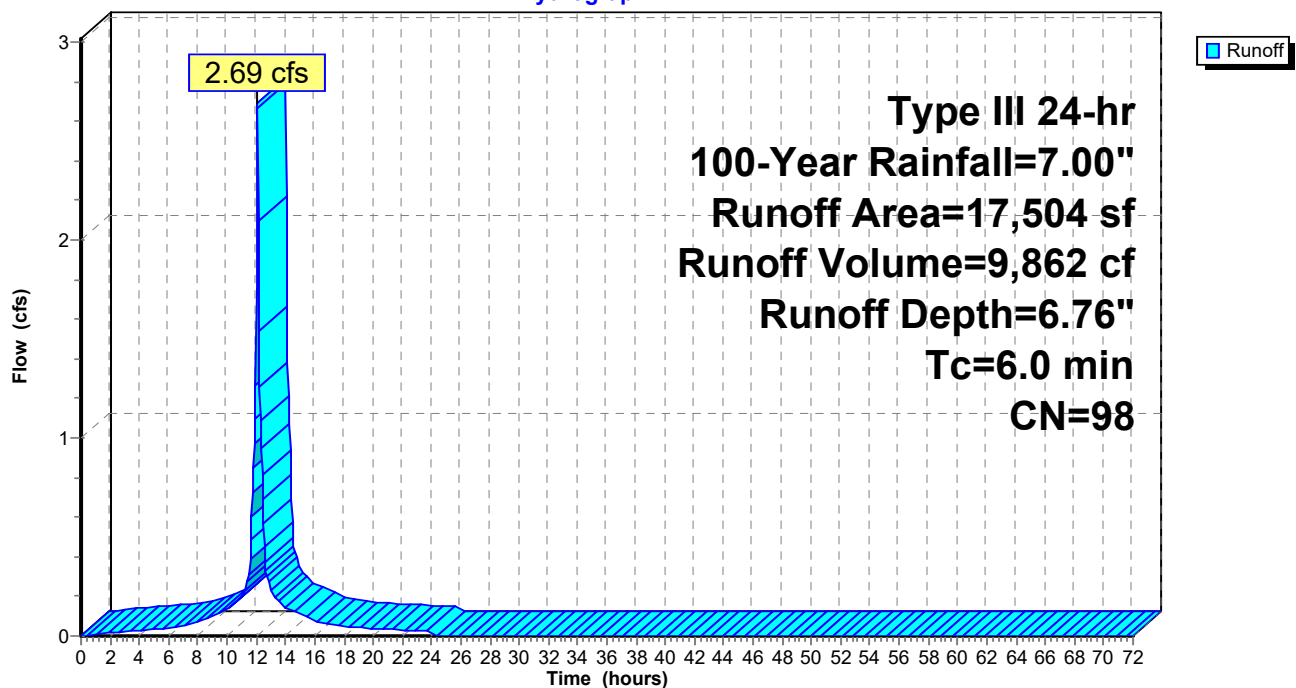
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.00"

Area (sf)	CN	Description
17,504	98	Paved parking, HSG C
17,504	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, runoff from pavement

Subcatchment 13S: POST (4)

Hydrograph



Summary for Subcatchment 15S: PRE (3)

Runoff = 8.26 cfs @ 12.14 hrs, Volume= 29,438 cf, Depth= 3.62"

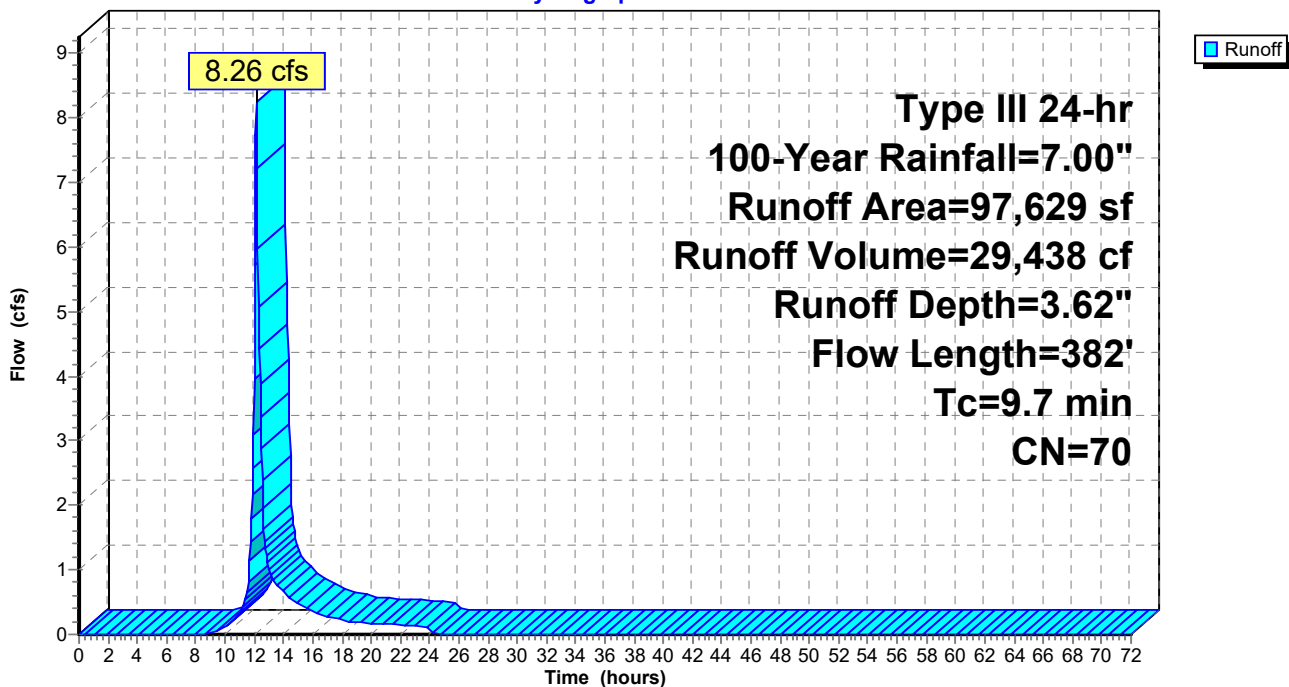
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.00"

Area (sf)	CN	Description
97,629	70	Woods, Good, HSG C
97,629	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.2000	0.17		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
2.6	138	0.0320	0.89		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.8	146	0.0710	1.33		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.5	48	0.0850	1.46		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
9.7	382	Total			

Subcatchment 15S: PRE (3)

Hydrograph



Summary for Subcatchment 16S: POST (7)

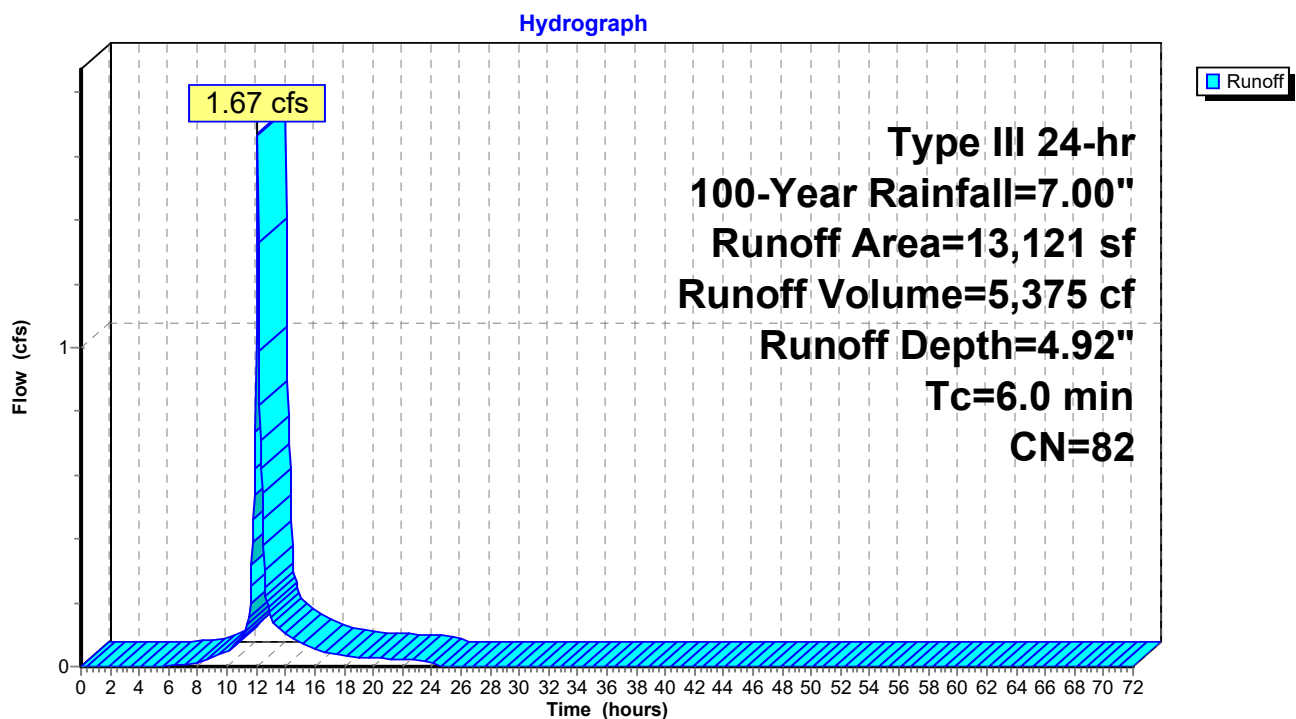
Runoff = 1.67 cfs @ 12.09 hrs, Volume= 5,375 cf, Depth= 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.00"

Area (sf)	CN	Description
8,766	74	>75% Grass cover, Good, HSG C
4,355	98	Paved parking, HSG C
13,121	82	Weighted Average
8,766	74	66.81% Pervious Area
4,355	98	33.19% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 16S: POST (7)



Summary for Subcatchment 18S: POST (5)

Runoff = 1.54 cfs @ 12.09 hrs, Volume= 5,634 cf, Depth= 6.76"

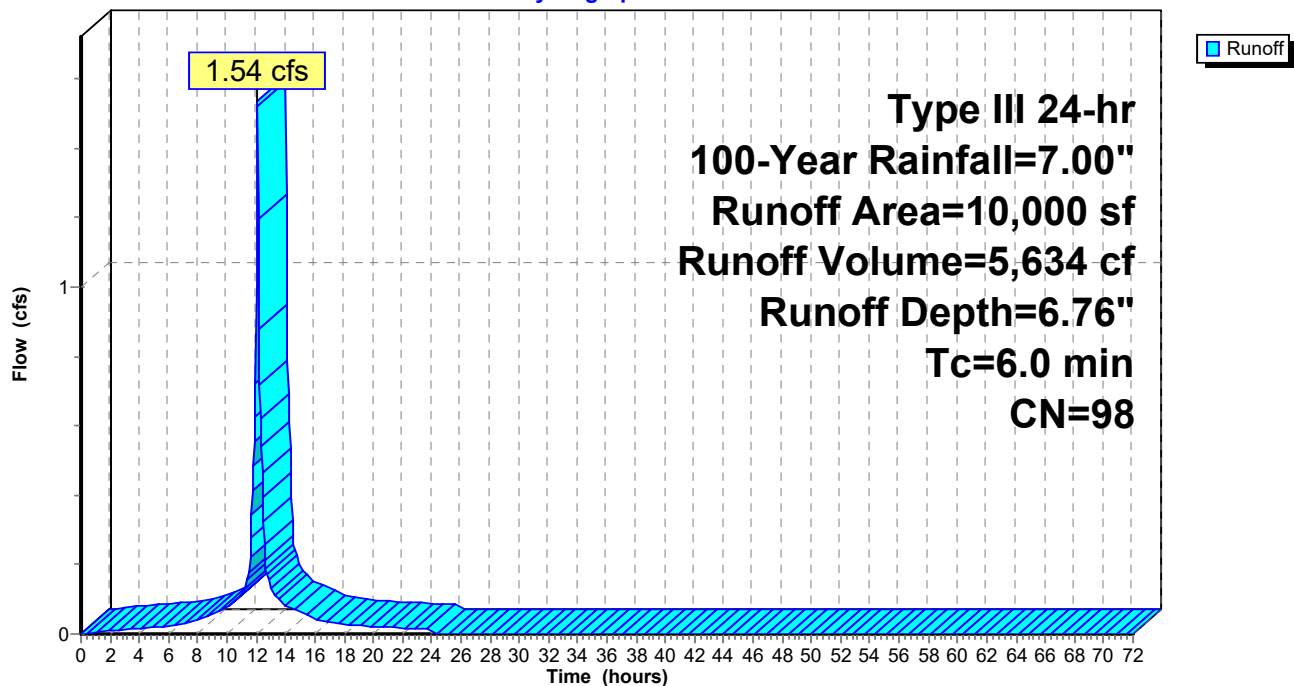
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.00"

Area (sf)	CN	Description
10,000	98	Roofs, HSG C
10,000	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Estimate

Subcatchment 18S: POST (5)

Hydrograph



Summary for Subcatchment 19S: POST (6)

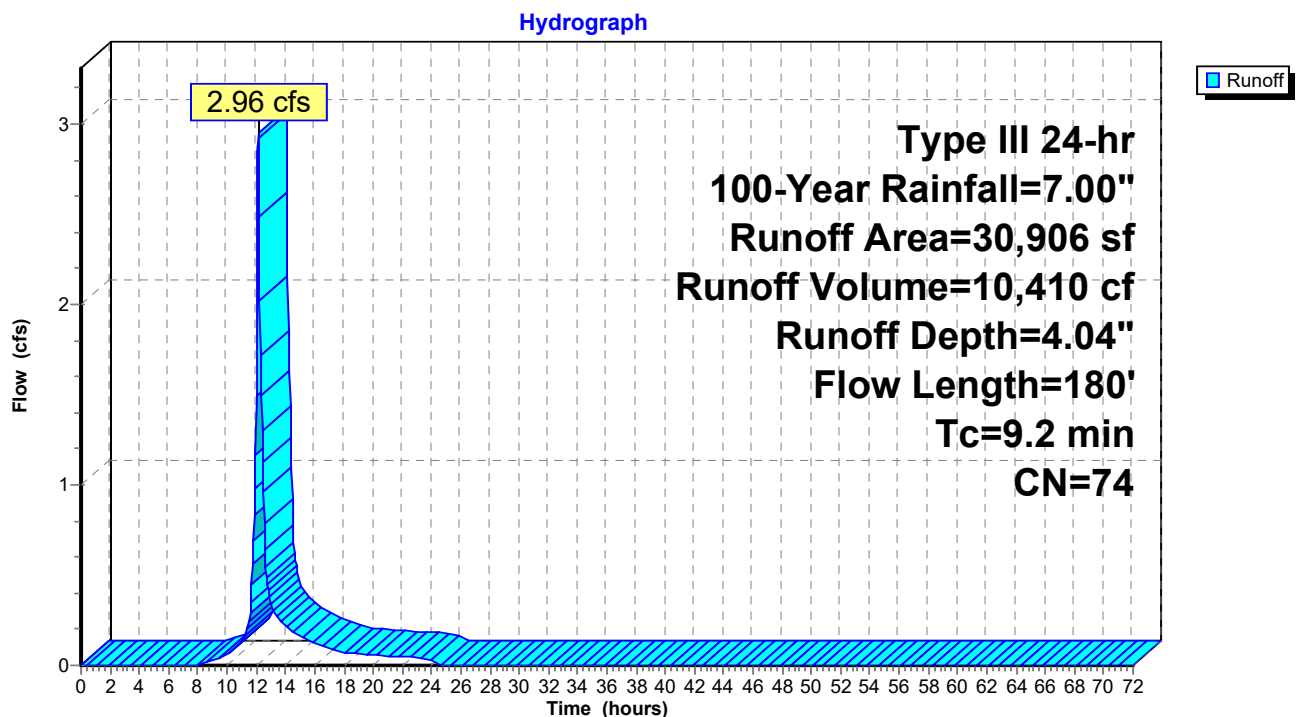
Runoff = 2.96 cfs @ 12.13 hrs, Volume= 10,410 cf, Depth= 4.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.00"

Area (sf)	CN	Description
30,906	74	>75% Grass cover, Good, HSG C
30,906	74	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	50	0.0200	0.10		Sheet Flow, Grass: Dense n= 0.240 P2= 3.40"
0.9	93	0.0600	1.71		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.3	37	0.1200	2.42		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
9.2	180	Total			

Subcatchment 19S: POST (6)



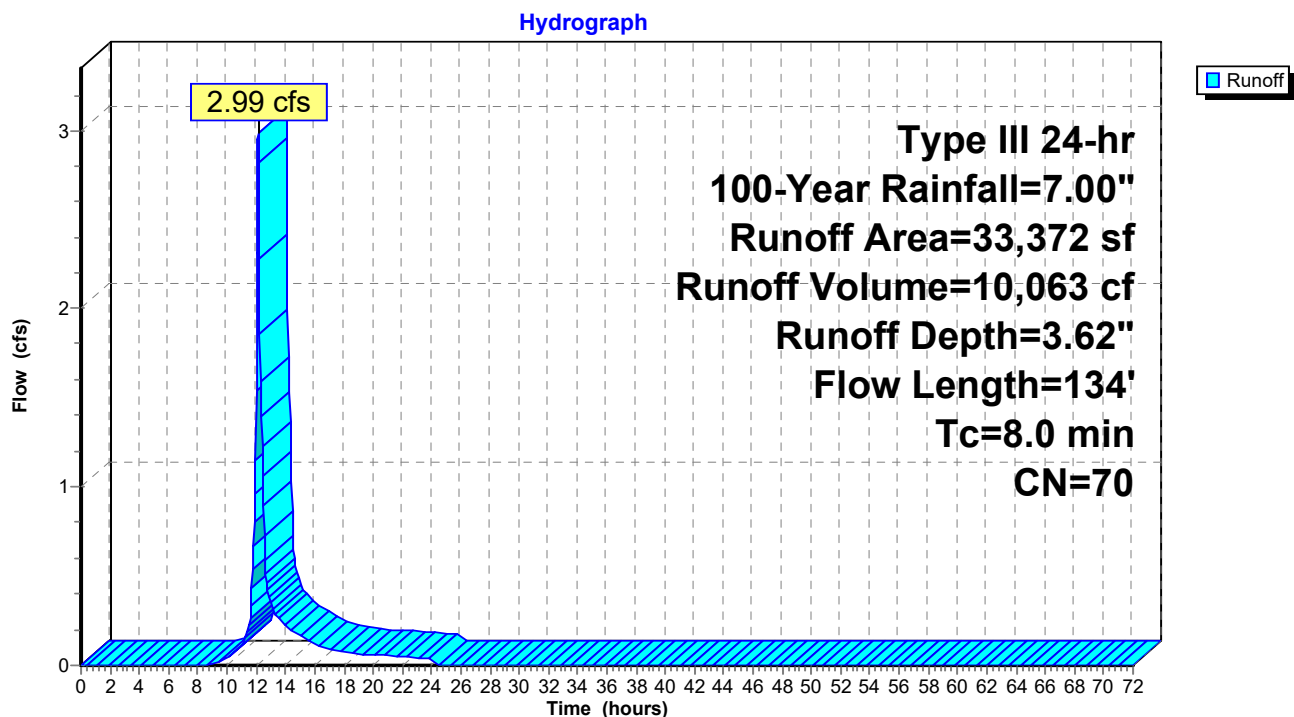
Summary for Subcatchment 20S: POST (8)

Runoff = 2.99 cfs @ 12.12 hrs, Volume= 10,063 cf, Depth= 3.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-Year Rainfall=7.00"

Area (sf)	CN	Description
1,400	74	>75% Grass cover, Good, HSG C
31,972	70	Woods, Good, HSG C
33,372	70	Weighted Average
33,372	70	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	50	0.0800	0.12		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.40"
1.1	84	0.0700	1.32		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
8.0	134	Total			

Subcatchment 20S: POST (8)

Summary for Pond 21P: Basin #2

Inflow Area = 13,121 sf, 33.19% Impervious, Inflow Depth = 15.67" for 100-Year event
 Inflow = 5.46 cfs @ 12.20 hrs, Volume= 17,130 cf
 Outflow = 5.15 cfs @ 12.27 hrs, Volume= 17,130 cf, Atten= 6%, Lag= 4.3 min
 Discarded = 0.11 cfs @ 12.27 hrs, Volume= 3,574 cf
 Primary = 1.39 cfs @ 12.27 hrs, Volume= 3,675 cf
 Tertiary = 3.65 cfs @ 12.27 hrs, Volume= 9,881 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 76.80' @ 12.27 hrs Surf.Area= 1,874 sf Storage= 1,987 cf

Plug-Flow detention time= 22.6 min calculated for 17,118 cf (100% of inflow)
 Center-of-Mass det. time= 22.6 min (802.5 - 780.0)

Volume	Invert	Avail.Storage	Storage Description
#1	75.50'	4,644 cf	stage (Conic) Listed below

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
75.50	1,179	0	0	1,179
76.00	1,429	651	651	1,437
76.50	1,702	782	1,433	1,719
77.00	1,988	922	2,354	2,015
77.50	2,288	1,068	3,422	2,326
78.00	2,603	1,222	4,644	2,653

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	76.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height
#3	Primary	76.00'	9.0" Vert. Orifice/Grate C= 0.600

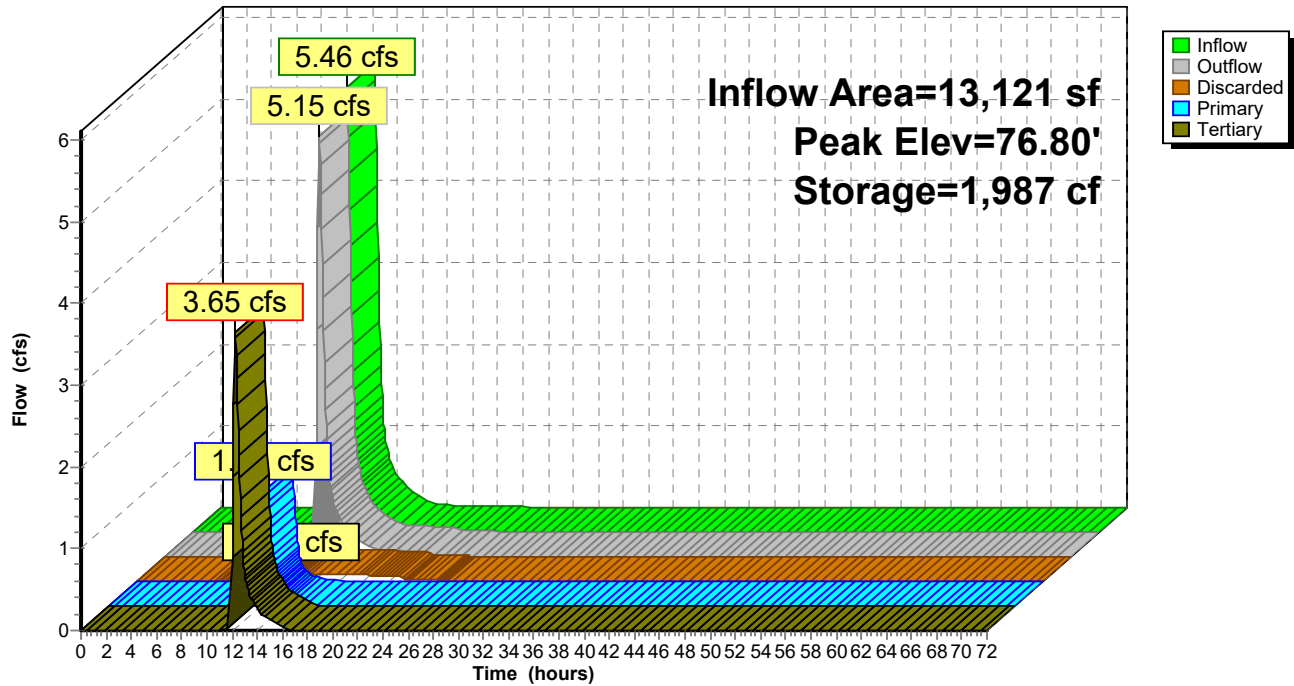
Discarded OutFlow Max=0.11 cfs @ 12.27 hrs HW=76.80' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=1.38 cfs @ 12.27 hrs HW=76.80' (Free Discharge)
 ↑ **3=Orifice/Grate** (Orifice Controls 1.38 cfs @ 3.12 fps)

Tertiary OutFlow Max=3.62 cfs @ 12.27 hrs HW=76.80' (Free Discharge)
 ↑ **2=Sharp-Crested Rectangular Weir** (Weir Controls 3.62 cfs @ 3.20 fps)

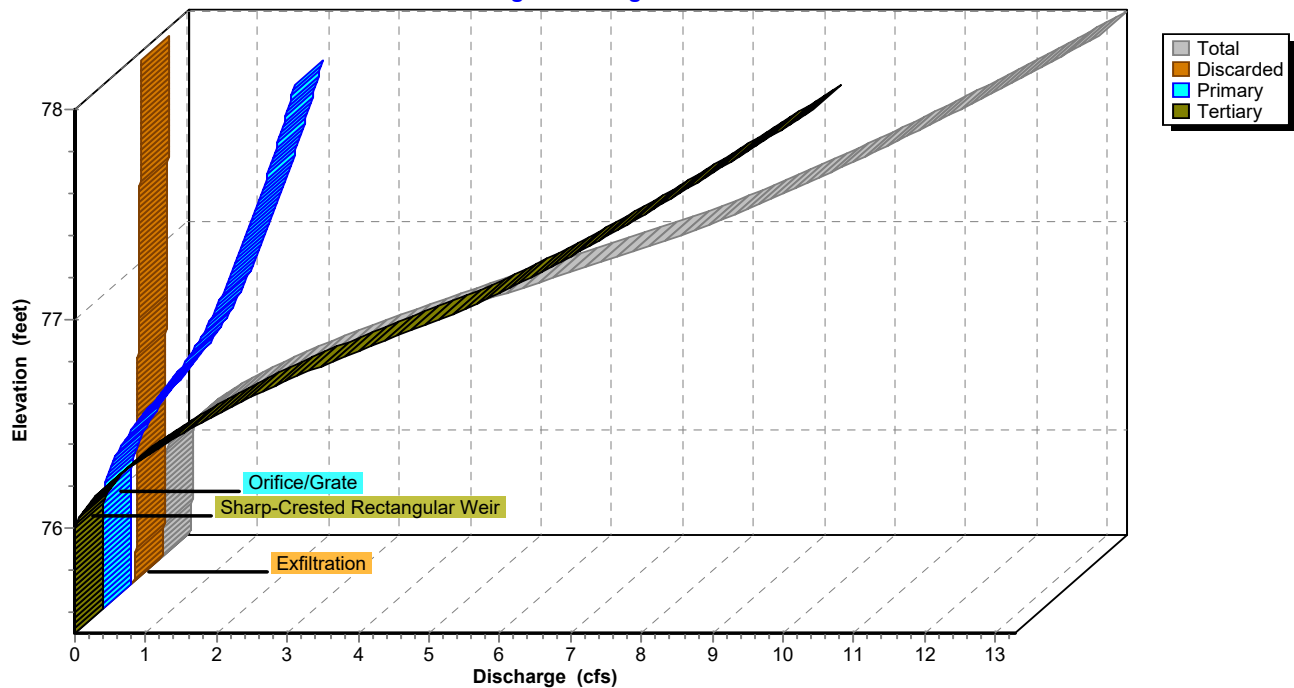
Pond 21P: Basin #2

Hydrograph



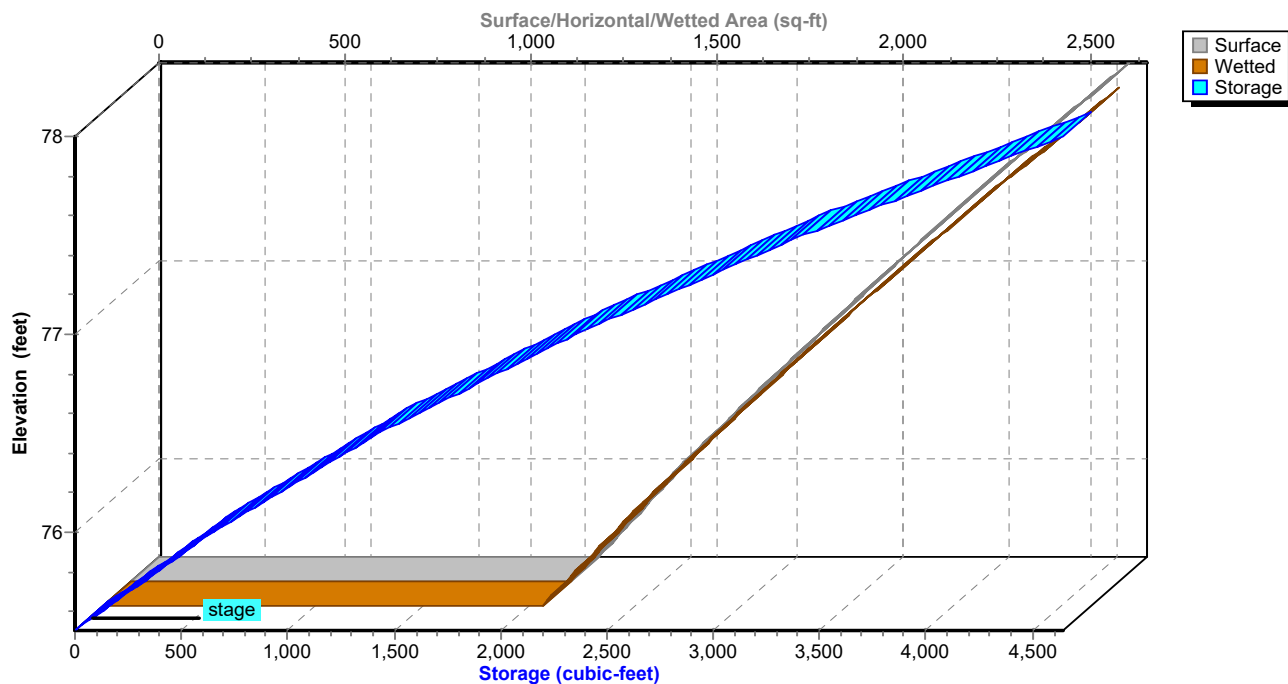
Pond 21P: Basin #2

Stage-Discharge



Pond 21P: Basin #2

Stage-Area-Storage



Summary for Pond 24P: Basin #1

Inflow Area = 58,410 sf, 47.09% Impervious, Inflow Depth = 4.63" for 100-Year event
 Inflow = 6.94 cfs @ 12.11 hrs, Volume= 22,557 cf
 Outflow = 4.71 cfs @ 12.22 hrs, Volume= 22,557 cf, Atten= 32%, Lag= 6.7 min
 Discarded = 0.21 cfs @ 12.22 hrs, Volume= 10,802 cf
 Tertiary = 4.51 cfs @ 12.22 hrs, Volume= 11,755 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 78.92' @ 12.22 hrs Surf.Area= 3,629 sf Storage= 5,543 cf

Plug-Flow detention time= 81.9 min calculated for 22,541 cf (100% of inflow)
 Center-of-Mass det. time= 81.9 min (863.4 - 781.5)

Volume	Invert	Avail.Storage	Storage Description
#1	77.10'	9,844 cf	stage (Conic) Listed below

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
77.10	2,446	0	0	2,446
78.00	3,036	2,462	2,462	3,059
79.00	3,682	3,354	5,816	3,737
80.00	4,385	4,028	9,844	4,475

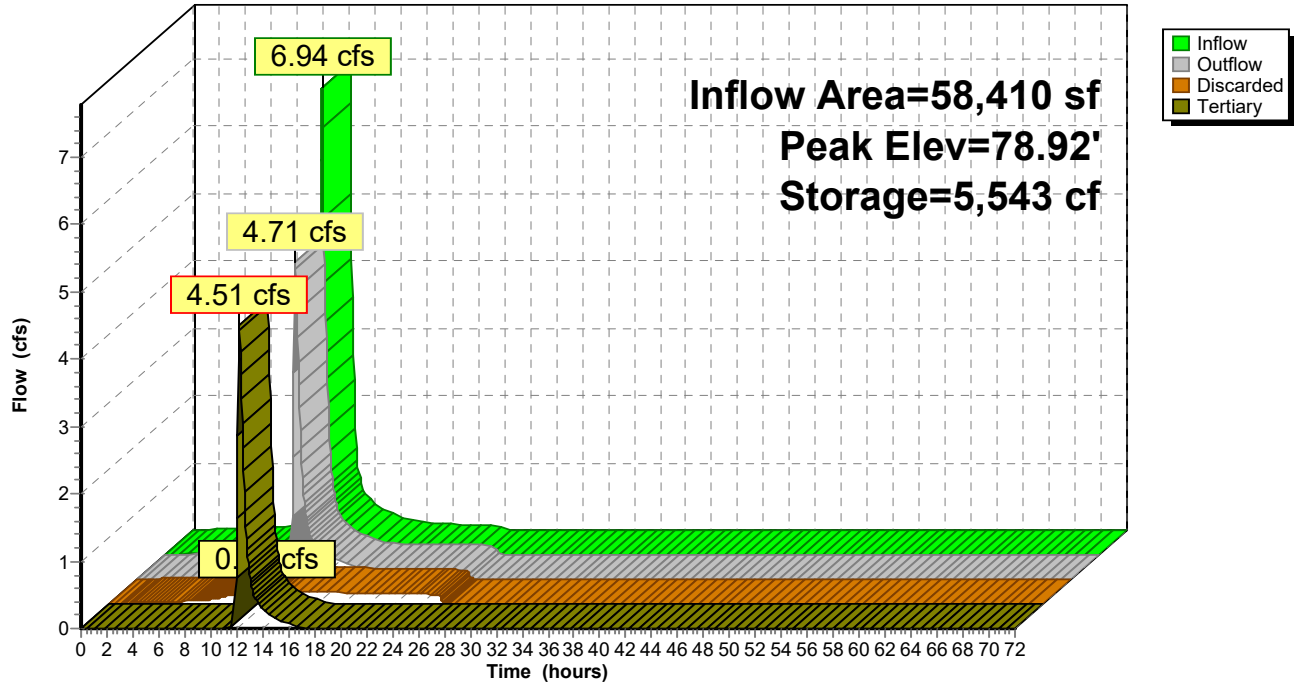
Device	Routing	Invert	Outlet Devices
#1	Discarded	77.10'	2.410 in/hr Exfiltration over Wetted area
#2	Tertiary	78.00'	1.5' long x 1.00' rise Sharp-Crested Rectangular Weir 1 End Contraction(s) 1.0' Crest Height

Discarded OutFlow Max=0.21 cfs @ 12.22 hrs HW=78.91' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.21 cfs)

Tertiary OutFlow Max=4.44 cfs @ 12.22 hrs HW=78.91' (Free Discharge)
 ↑ **2=Sharp-Crested Rectangular Weir** (Weir Controls 4.44 cfs @ 3.47 fps)

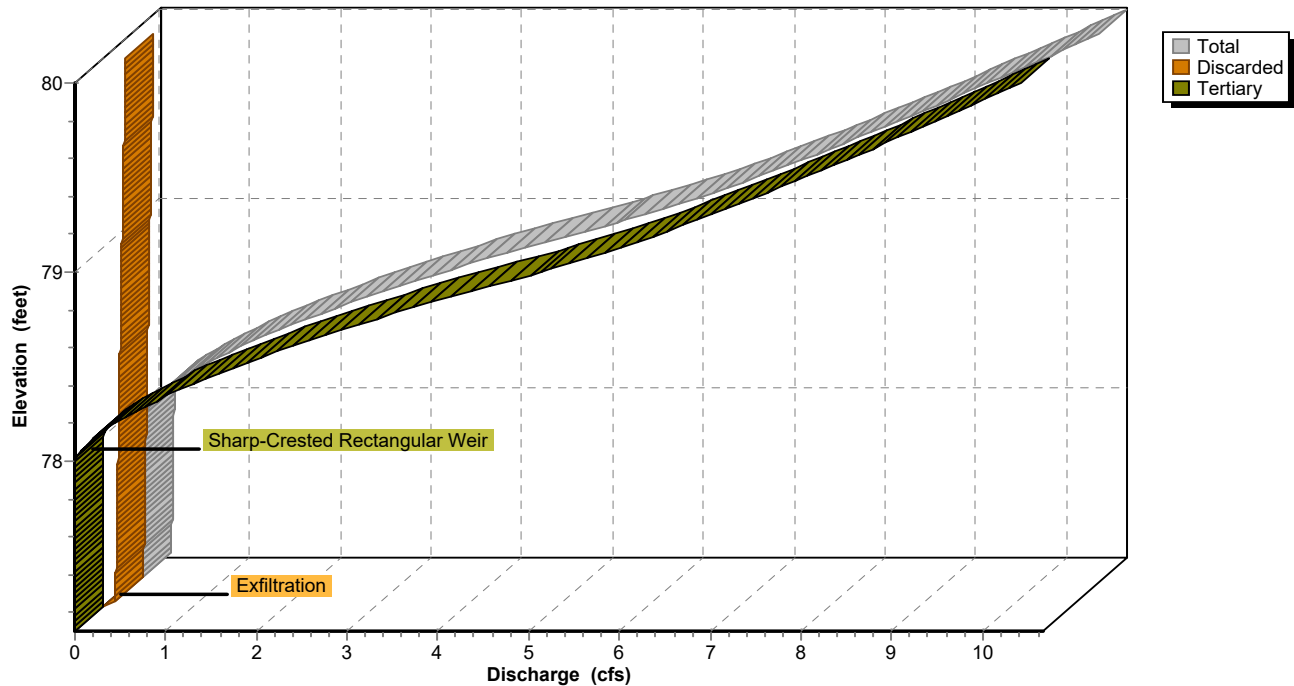
Pond 24P: Basin #1

Hydrograph

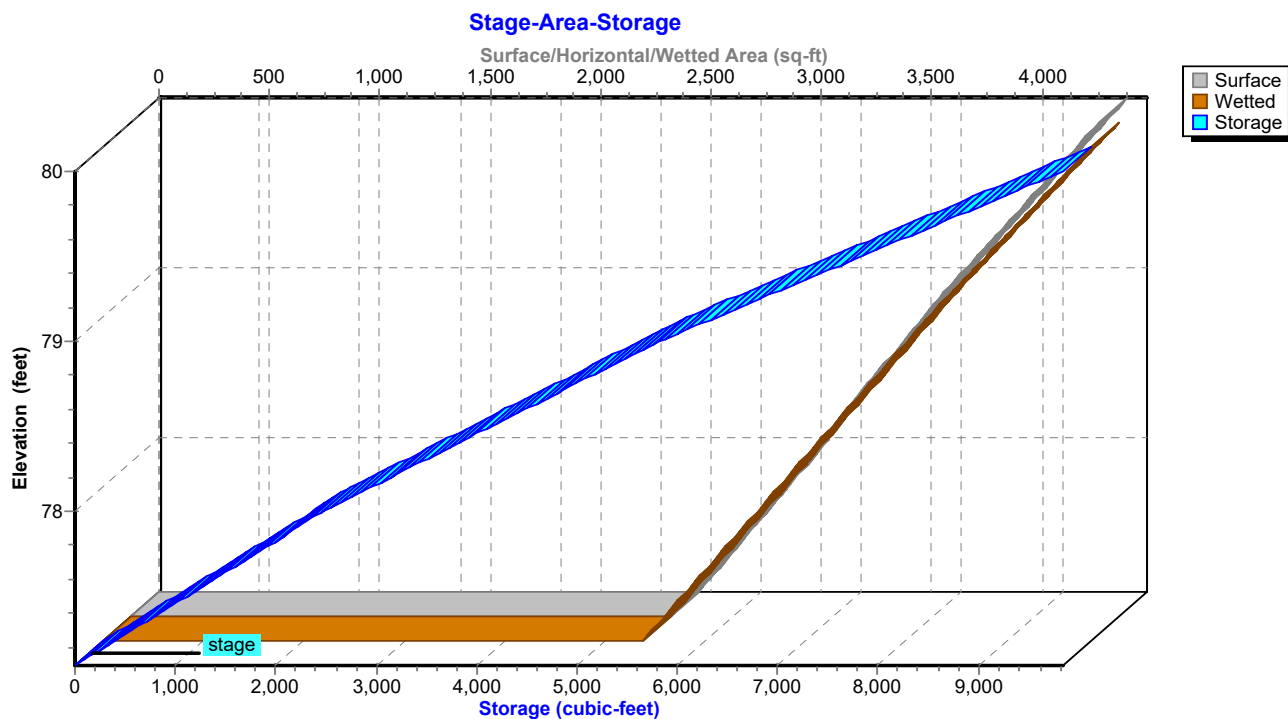


Pond 24P: Basin #1

Stage-Discharge



Pond 24P: Basin #1



Summary for Pond 25P: Subsurface infiltration#1

Inflow Area = 10,000 sf, 100.00% Impervious, Inflow Depth = 6.76" for 100-Year event
 Inflow = 1.54 cfs @ 12.09 hrs, Volume= 5,634 cf
 Outflow = 1.47 cfs @ 12.11 hrs, Volume= 5,634 cf, Atten= 4%, Lag= 1.5 min
 Discarded = 0.05 cfs @ 12.11 hrs, Volume= 3,350 cf
 Primary = 1.42 cfs @ 12.11 hrs, Volume= 2,284 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Peak Elev= 84.05' @ 12.11 hrs Surf.Area= 461 sf Storage= 1,263 cf

Plug-Flow detention time= 140.8 min calculated for 5,630 cf (100% of inflow)
 Center-of-Mass det. time= 140.9 min (883.8 - 743.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	79.00'	637 cf	12.80'W x 36.00'L x 5.50'H Field A 2,534 cf Overall - 942 cf Embedded = 1,592 cf x 40.0% Voids
#2A	80.00'	710 cf	Concrete Galley 4x4x4 x 16 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 2 Rows of 8 Chambers
		1,346 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	79.00'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	83.00'	8.0" Vert. Orifice/Grate C= 0.600

Discarded OutFlow Max=0.05 cfs @ 12.11 hrs HW=84.02' (Free Discharge)
 ↑ **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=1.39 cfs @ 12.11 hrs HW=84.02' (Free Discharge)
 ↑ **2=Orifice/Grate** (Orifice Controls 1.39 cfs @ 3.99 fps)

Pond 25P: Subsurface infiltration#1 - Chamber Wizard Field A**Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)**

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

8 Chambers/Row x 4.00' Long = 32.00' Row Length +24.0" End Stone x 2 = 36.00' Base Length

2 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 12.80' Base Width

12.0" Base + 48.0" Chamber Height + 6.0" Cover = 5.50' Field Height

16 Chambers x 44.3 cf = 709.5 cf Chamber Storage

16 Chambers x 58.9 cf = 942.0 cf Displacement

2,534.4 cf Field - 942.0 cf Chambers = 1,592.4 cf Stone x 40.0% Voids = 637.0 cf Stone Storage

Chamber Storage + Stone Storage = 1,346.5 cf = 0.031 af

Overall Storage Efficiency = 53.1%

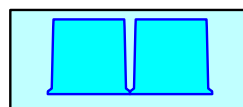
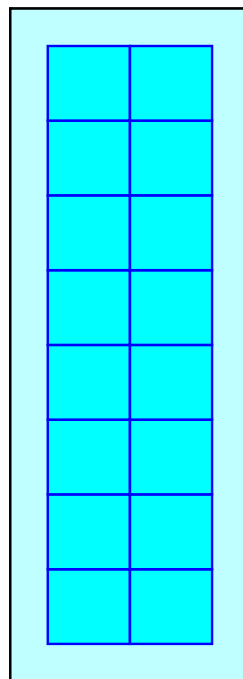
Overall System Size = 36.00' x 12.80' x 5.50'

16 Chambers @ \$ 300.00 /ea = \$ 4,800.00

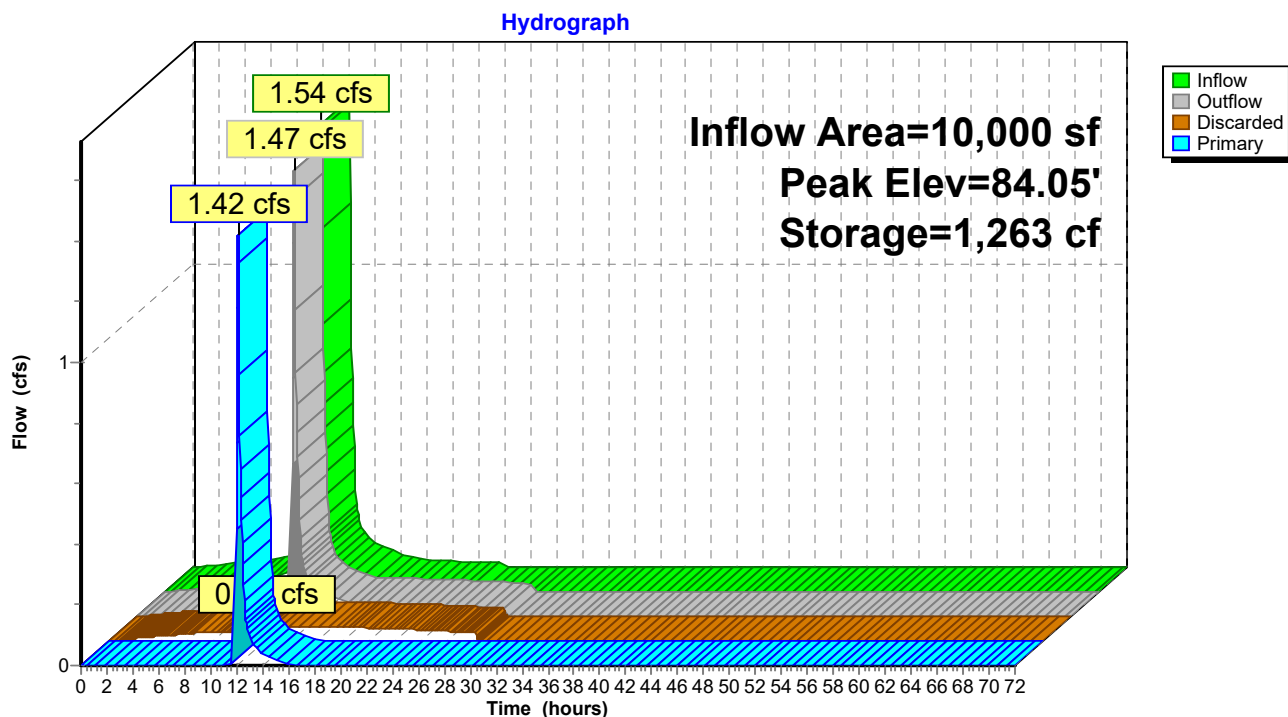
93.9 cy Field Excavation @ \$ 10.00 /cy = \$ 938.67

59.0 cy Stone @ \$ 30.00 /cy = \$ 1,769.31

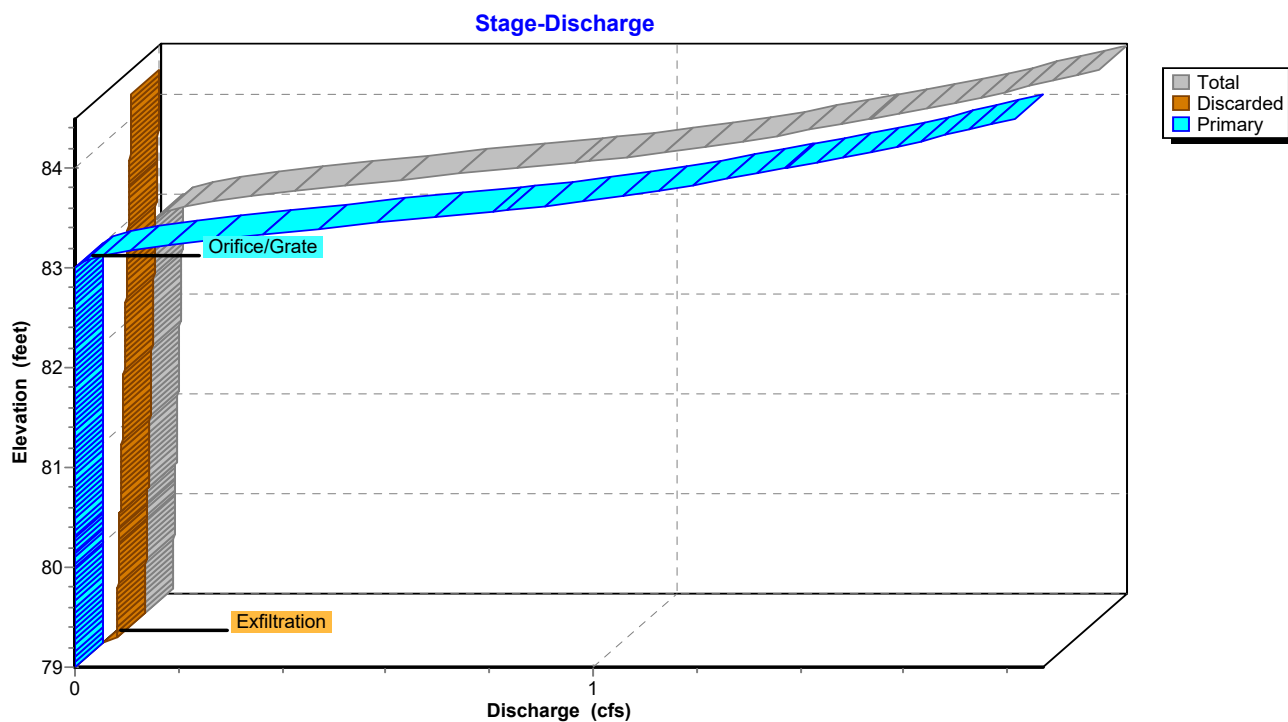
Total Cost = \$ 7,507.98



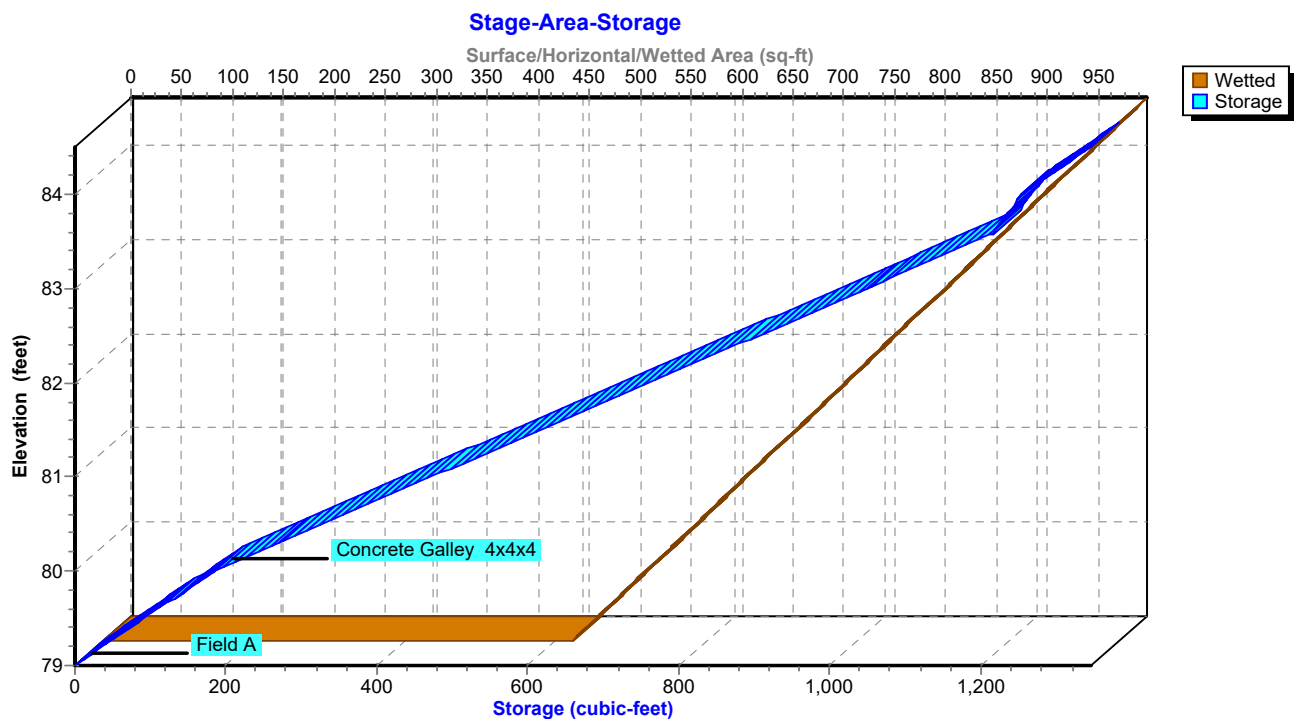
Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Pond 25P: Subsurface infiltration#1



Summary for Pond 27P: Subsurface infiltration#2

[43] Hint: Has no inflow (Outflow=Zero)

Volume	Invert	Avail.Storage	Storage Description
#1A	75.50'	2,584 cf	21.60'W x 88.00'L x 6.00'H Field A 11,405 cf Overall - 4,946 cf Embedded = 6,459 cf x 40.0% Voids
#2A	76.50'	3,725 cf	Concrete Galley 4x4x4 x 84 Inside #1 Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf 4 Rows of 21 Chambers
#3	81.50'	15 cf	Custom Stage Data (Prismatic) Listed below (Recalc) -Impervious
		6,324 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.50	6	0	0
84.00	6	15	15

Device	Routing	Invert	Outlet Devices
#1	Discarded	75.50'	2.410 in/hr Exfiltration over Wetted area
#2	Primary	80.00'	6.0" Vert. Orifice/Grate C= 0.600
#3	Primary	78.75'	6.0" Vert. Orifice/Grate C= 0.600
#4	Primary	78.10'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**1=Exfiltration** (Controls 0.00 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**2=Orifice/Grate** (Controls 0.00 cfs)

↑**3=Orifice/Grate** (Controls 0.00 cfs)

↑**4=Orifice/Grate** (Controls 0.00 cfs)

Pond 27P: Subsurface infiltration#2 - Chamber Wizard Field A

Chamber Model = Concrete Galley 4x4x4 (Concrete Galley, UCPI 4x4x4 Galley or equivalent)

Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf

Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf

21 Chambers/Row x 4.00' Long = 84.00' Row Length +24.0" End Stone x 2 = 88.00' Base Length

4 Rows x 52.8" Wide + 24.0" Side Stone x 2 = 21.60' Base Width

12.0" Base + 48.0" Chamber Height + 12.0" Cover = 6.00' Field Height

84 Chambers x 44.3 cf = 3,725.0 cf Chamber Storage

84 Chambers x 58.9 cf = 4,945.6 cf Displacement

11,404.8 cf Field - 4,945.6 cf Chambers = 6,459.2 cf Stone x 40.0% Voids = 2,583.7 cf Stone Storage

Chamber Storage + Stone Storage = 6,308.6 cf = 0.145 af

Overall Storage Efficiency = 55.3%

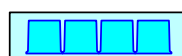
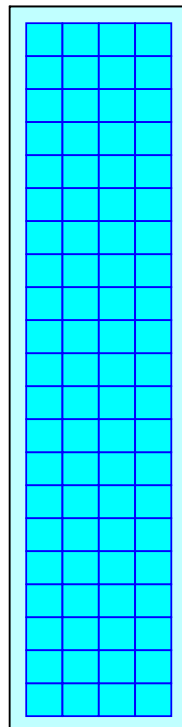
Overall System Size = 88.00' x 21.60' x 6.00'

84 Chambers @ \$ 300.00 /ea = \$ 25,200.00

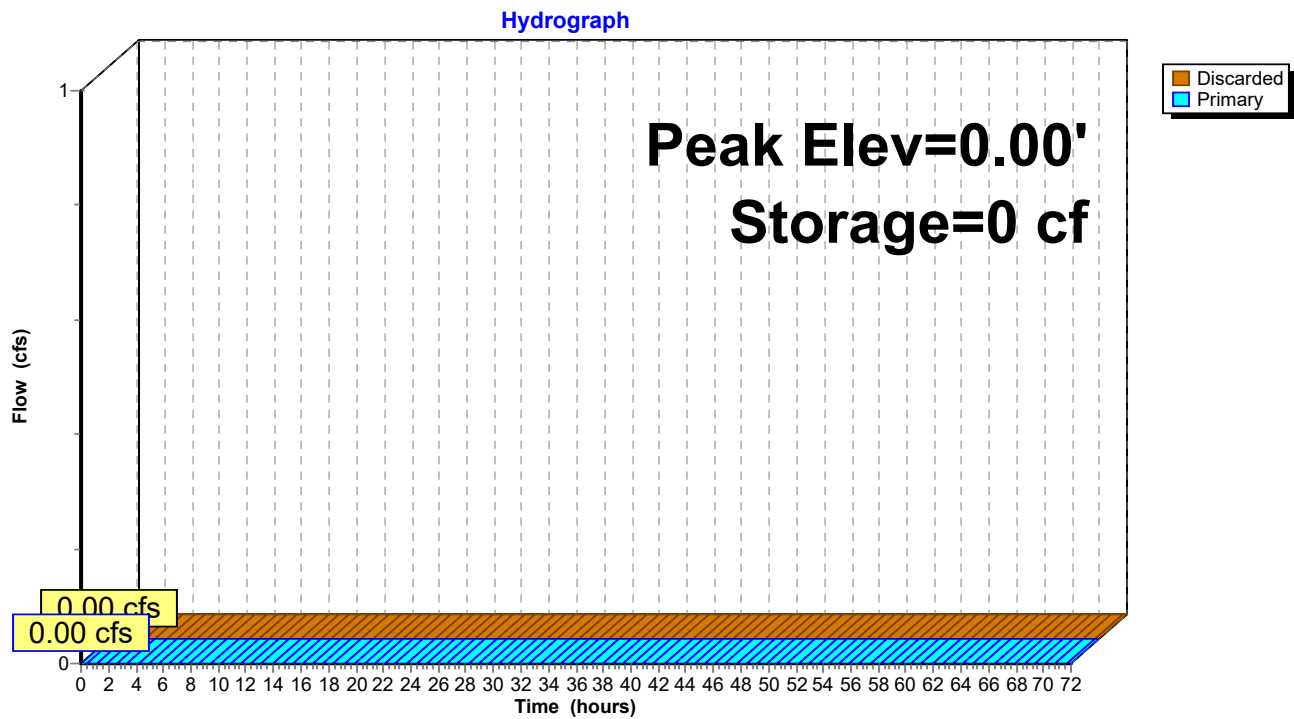
422.4 cy Field Excavation @ \$ 10.00 /cy = \$ 4,224.00

239.2 cy Stone @ \$ 30.00 /cy = \$ 7,176.88

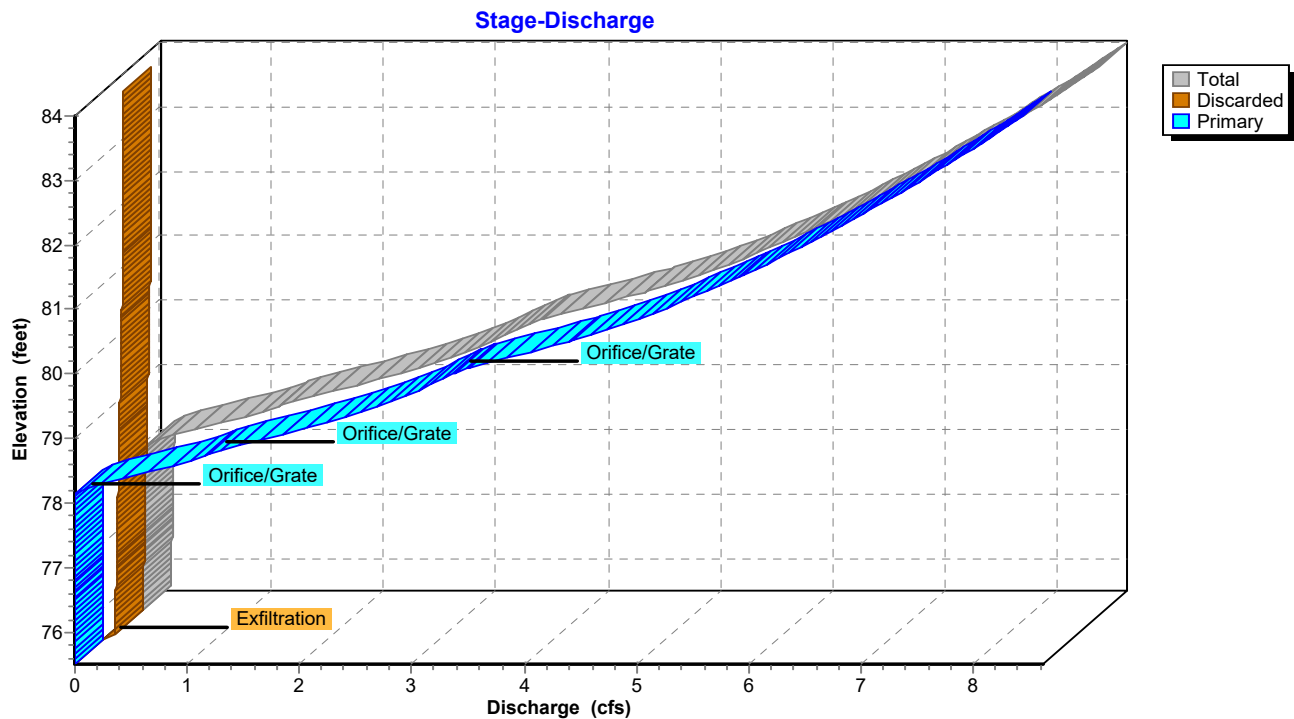
Total Cost = \$ 36,600.88



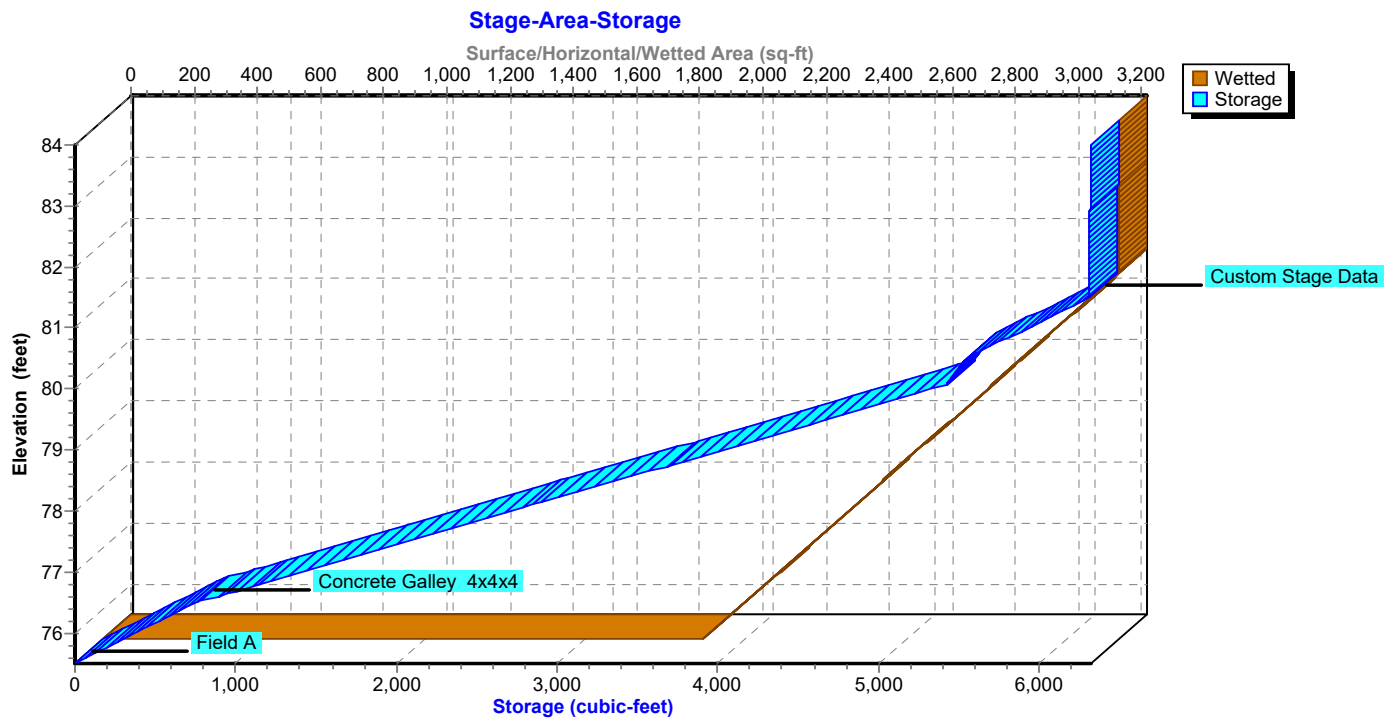
Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2



Pond 27P: Subsurface infiltration#2

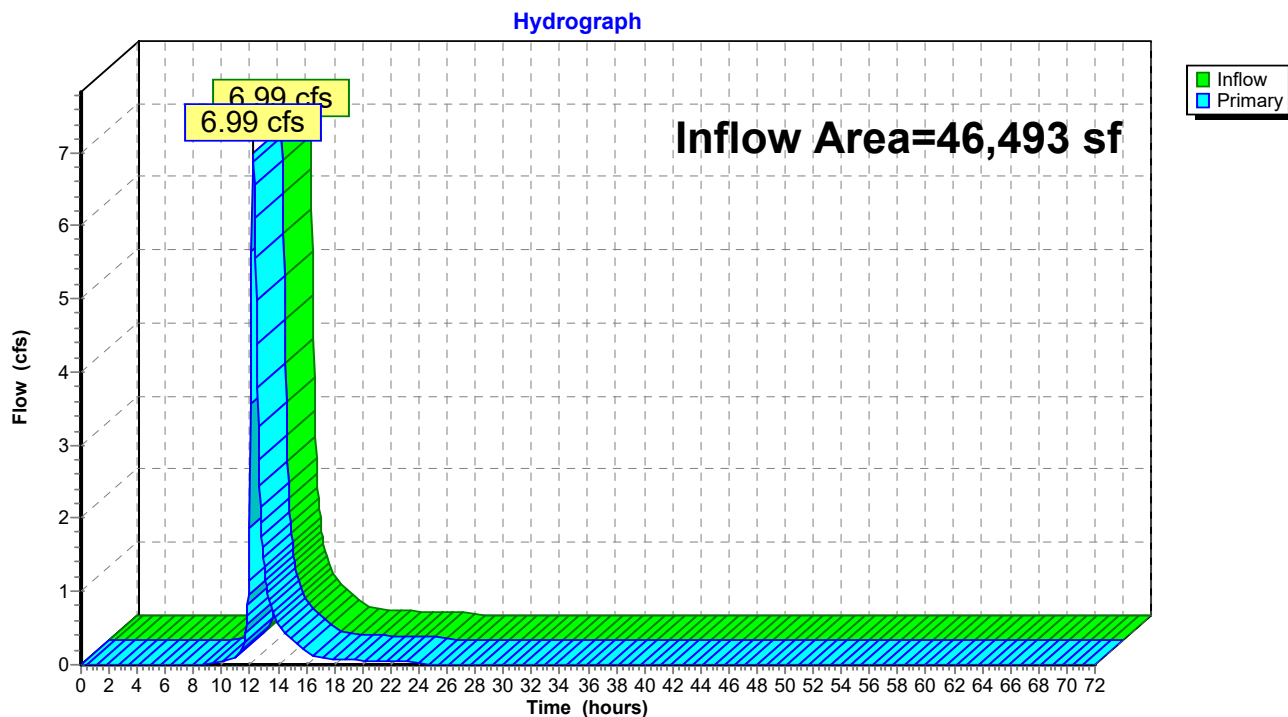


Summary for Link 23L: (Combined Link)

Inflow Area = 46,493 sf, 9.37% Impervious, Inflow Depth = 6.10" for 100-Year event
 Inflow = 6.99 cfs @ 12.21 hrs, Volume= 23,618 cf
 Primary = 6.99 cfs @ 12.21 hrs, Volume= 23,618 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Link 23L: (Combined Link)



Section II

Stormwater Compliance



Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



october 29, 2021

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- ☒ New development
- ☐ Redevelopment
- ☐ Mix of New Development and Redevelopment



Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☒ No disturbance to any Wetland Resource Areas
- ☒ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☐ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
 - ☐ Credit 1
 - ☐ Credit 2
 - ☐ Credit 3
- ☐ Use of "country drainage" versus curb and gutter conveyance and pipe
- ☐ Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- ☐ Treebox Filter
- ☐ Water Quality Swale
- ☐ Grass Channel
- ☐ Green Roof
- ☐ Other (describe): _____

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- ☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- ☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- ☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- ☒ Soil Analysis provided.
- ☒ Required Recharge Volume calculation provided.
- ☐ Required Recharge volume reduced through use of the LID site Design Credits.
- ☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - ☒ Static
 - ☐ Simple Dynamic
 - ☐ Dynamic Field¹
- ☒ Runoff from all impervious areas at the site discharging to the infiltration BMP.
- ☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- ☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- ☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
 - ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
 - ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- ☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- ☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- ☒ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- ☒ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- ☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - ☒ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - ☐ is within the Zone II or Interim Wellhead Protection Area
 - ☐ is near or to other critical areas
 - ☒ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - ☐ involves runoff from land uses with higher potential pollutant loads.
 - ☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - ☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- ☒ The BMP is sized (and calculations provided) based on:
 - ☒ The ½" or 1" Water Quality Volume or
 - ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- ☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- ☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- ☒ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- ☐ The NPDES Multi-Sector General Permit does **not** cover the land use.
- ☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- ☒ All exposure has been eliminated.
- ☐ All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- ☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- ☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- ☐ Critical areas and BMPs are identified in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- ☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - ☐ Limited Project
 - ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - ☐ Bike Path and/or Foot Path
 - ☐ Redevelopment Project
 - ☐ Redevelopment portion of mix of new and redevelopment.
- ☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- ☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- ☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- ☐ The project is **not** covered by a NPDES Construction General Permit.
- ☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- ☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- ☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - ☒ Name of the stormwater management system owners;
 - ☒ Party responsible for operation and maintenance;
 - ☒ Schedule for implementation of routine and non-routine maintenance tasks;
 - ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
 - ☒ Description and delineation of public safety features;
 - ☒ Estimated operation and maintenance budget; and
 - ☒ Operation and Maintenance Log Form.
- ☐ The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- ☐ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- ☒ An Illicit Discharge Compliance Statement is attached;
- ☐ NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

◆ **Standard 1: No New Untreated Discharges**

The proposed development proposes no new stormwater conveyances that discharge untreated stormwater off-site or cause down gradient erosion.

◆ **Standard 2: Peak Rate Attenuation**

The overall site analysis demonstrates that the stormwater management system has been designed so that the post-development peak discharge rates do not exceed the pre-development discharge rate for the 2-year, 10-year, 25-year & 100-year 24-hour storm events.

◆ **Standard 3: Recharge**

Required Recharge Volume

$$R_v = F \times \text{Total Impervious Area}$$

The site is comprised of Group C Soils, according to NRCS soils Mapping

F (Target Depth Factor) = 0.25"

Required	(Impervious SF)	(1/12)	Req DWQ	Prop (CF)	Prop (CF)	
WQV Basin #1	17504	0.0833	1	1459	2593	provided at el=78.00 (lowest outlet)
WQV Basin #2	4355	0.0833	1	363	651	provided at el=76.0 (lowest outlet)
WQV Roof system	5000	0.0833	1	417	1254	provided at el=83.00 (lowest outlet)
WQV Total	26859	0.083333	1	2238	4498	

Drawdown Within 72 Hours

$$\text{Time}_{\text{drawdown}} = \frac{R_v}{(K)(\text{Bottom Area})}$$

Where:

R_v = Storage Volume (required recharge volume)

K = Saturated Hydraulic Conductivity For "Static" and "Simple Dynamic" Methods, use Rawls Rate (see Table 2.3.3). For "Dynamic Field" Method, use 50% of the in-situ saturated hydraulic conductivity.

Bottom Area = Bottom Area of Recharge Structure

$$\text{Infiltration Basin\#1: } \text{Time}_{\text{drawdown}} = \frac{2593 \text{ CF}}{(2.41'')(1'/12'')(2447 \text{ SF})} = 5.3 \text{ hours} < 72 \text{ hours}$$

$$\text{Infiltration Basin\#2: } \text{Time}_{\text{drawdown}} = \frac{651 \text{ CF}}{(2.41'')(1'/12'')(1179 \text{ SF})} = 2.8 \text{ hours} < 72 \text{ hours}$$

$$\text{Subsurface Basin \#1: } \text{Time}_{\text{drawdown}} = \frac{1254 \text{ CF}}{(2.41'')(1'/12'')(483 \text{ SF})} = 12.9 \text{ hours} < 72 \text{ hours}$$

$$\text{Sediment forebay Basin \#1: } \text{Time}_{\text{drawdown}} = \frac{73 \text{ CF}}{(2.41'')(1'/12'')(57 \text{ SF})} = 6.3 \text{ hours} < 72 \text{ hours}$$

$$\text{Sediment forebay Subsurface\#2: } \text{Time}_{\text{drawdown}} = \frac{67 \text{ CF}}{(2.41'')(1'/12'')(241 \text{ SF})} = 1.3 \text{ hours} < 72 \text{ hours}$$

Mounding Analysis

“Mounding analysis is required when the vertical separation from the bottom of an exfiltration system to seasonal high groundwater is less than four (4) feet and the recharge system is proposed to attenuate the peak discharge from a 10-year or higher 24-hour storm (e.g., 10-year, 25-year, 50-year, or 100-year 24-hour storm). In such cases, the mounding analysis must demonstrate that the Required Recharge Volume (e.g., infiltration basin storage) is fully dewatered within 72 hours (so the next storm can be stored for exfiltration). The mounding analysis must also show that the groundwater mound that forms under the recharge system will not break out above the land or water surface of a wetland (e.g., it doesn’t increase the water sheet elevation in a Bordering Vegetated Wetland, Salt Marsh, or Land Under Water within the 72-hour evaluation period).”

“The Hantush¹ or other equivalent method may be used to conduct the mounding analysis. The Hantush method predicts the maximum height of the groundwater mound beneath a rectangular or circular recharge area. It assumes unconfined groundwater flow, and that a linear relation exists between the water table elevation and water table decline rate. It results in a water table recession hydrograph depicting exponential decline. The Hantush method is available in proprietary software and free on-line calculators on the Web in automated format. If the analysis indicates the mound will prevent the infiltration BMP from fully draining within the 72-hour period, an iterative process must be employed to determine an alternative design that drains within the 72-hour period.”

Mounding analysis is required when the vertical separation from the bottom of the exfiltration system to seasonal high groundwater is less than 4 ft. The mounding analysis must show that the groundwater mound that forms under the recharge system will not breakout above the land.

Mounding

Infiltration	Bottom El	Groundwater el	Separation (FT)
Basin #1	77.10	73.1 (TH D-2, 12-13-07)	4.00 (mounding <u>not</u> required)
Infiltration	75.50	73.50 (TH D-1, 4/19/21)	2.00 (mounding required)
Basin#2			
Subsurface #1	79.00	76.55 (TH D-1, 4/19/21)	2.45 (mounding required)

¹ Hantush 1967 – See Reference for Standard 3.

Recharge Rate (Rr) = (volume retained below low outlet/WQV)/ (bottom area)

$$\begin{aligned} \text{Rr Infiltration Basin \#2} &= 652 / 1179 = 0.55 \\ \text{Rr Subsurface Basin \#1} &= 1254 / 483 = 2.59 \end{aligned}$$

Basin #2 using the Hantush Method, 1179 s.f. bottom area, volume = 652, recharge infiltration rate of 0.55 ft/day and an initial saturated thickness of 50 feet, we calculated a groundwater mounding of 1.1 feet. Groundwater separation = 2 ft mounding will not breakout above the land or interfere with infiltration

Subsurface Basin #1 using the Hantush Method, 483 s.f. bottom area, volume = 1254, recharge infiltration rate of 2.59 ft/day and an initial saturated thickness of 50 feet, we calculated a groundwater mounding of 1.8 feet. Groundwater separation = 2.45 ft mounding will not breakout above the land or interfere with infiltration

◆ Standard 4: Water Quality

Water Quality Treatment Volume

$$V_{WQ} = D_{WQ} \times A_{IMP}$$

D_{WQ} = *Water Quality Depth*: **one-inch** for land use with a higher potential pollutant load, within an area with an **infiltration rate greater than 2.4 inches per hour**, within a Zone II or Interim Wellhead Protection Area, or near or to another critical area; one-half-inch for all other areas.

$$A_{IMP} = \text{Impervious Area}$$

$$D_{WQ} = 1''$$

Required	(Impervious SF)	(1/12)	DWQ	Req (CF)	Prop (CF)	
WQV Basin#1	17504	0.0833	1	1459	2593	provided at el=78.00 (lowest outlet)
WQV Basin #2	4355	0.0833	1	363	651	provided at el=76.0 (lowest outlet)
WQV Roof system	5000	0.0833	1	417	1254	provided at el=83.00 (lowest outlet)
WQV Total	26859	0.083333	1	2238	4498	
		0.1 inch/ imp acre	Required			
Forbay Sizing	(Impervious SF)	(1/12)	Volume			Provided Volume cf provided at
		0.08333				el=78
Forebay Basin #1	17504	3	0.1	146 CF	204	cf provided at el=76.25
		0.08333				
Forebay Basin #2	4355	3	0.1	36 CF	67	

◆ **Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLS)**

The proposed office use proposed on this site is not considered a land use with higher potential pollutant loads.

◆ **Standard 6: Critical Areas**

The site is not located within an aquifer protection Zone II or Interim Wellhead Protection Area.

◆ **Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable**

The project is not a redevelopment project.

◆ **Standard 8: Construction Period Pollution Prevention and Erosion and Sediment Control Plan**

An operation and maintenance plan has been prepared and Erosion and sediment controls are detailed within the site plan.

◆ **Standard 9: Operation and Maintenance Plan**

See O&M plan attached hereto.

◆ **Standard 10: Prohibition of Illicit Discharges**

"All illicit discharges to the stormwater management system are prohibited."

This statement is intended to meet Standard #10 of the Stormwater Management requirements

Illicit discharges to the stormwater management system are discharges that are not entirely comprised of stormwater.

Except for the potential for deliberate criminal act of discharge by an unauthorized entity for which the property owner has no control, there are to be no illicit discharges into the stormwater system.

Applicant\Owner

The site is not located within an aquifer protection Zone II or Interim Wellhead Protection Area.

◆ **Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable**

The project is not a redevelopment project.

◆ **Standard 8: Construction Period Pollution Prevention and Erosion and Sediment Control Plan**

An operation and maintenance plan has been prepared and Erosion and sediment controls are detailed within the site plan.

◆ **Standard 9: Operation and Maintenance Plan**

See O&M plan attached hereto.

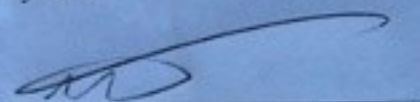
◆ **Standard 10: Prohibition of Illicit Discharges**

"All illicit discharges to the stormwater management system are prohibited."

This statement is intended to meet Standard #10 of the Stormwater Management requirements

Illicit discharges to the stormwater management system are discharges that are not entirely comprised of stormwater.

Except for the potential for deliberate criminal act of discharge by an unauthorized entity for which the property owner has no control, there are to be no illicit discharges into the stormwater system.



Applicant/Owner

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: Infiltration Basin #1

TSS Removal Calculation Worksheet	B	C	D	E	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
	Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75
		0.00	0.75	0.00	0.75
		0.00	0.75	0.00	0.75
		0.00	0.75	0.00	0.75
		0.00	0.75	0.00	0.75

Total TSS Removal =

25%

Separate Form Needs to
be Completed for Each
Outlet or BMP Train

Project: 120-365
Prepared By: KG
Date: 10/29/2021

*Equals remaining load from previous BMP (E)
which enters the BMP

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: Infiltration Basin#1Pretreatment

TSS Removal Calculation Worksheet	B	C	D	E	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
	Sediment Forebay	0.25	1.00	0.25	0.75
	Sediment Forebay	0.25	0.75	0.19	0.56
		0.00	0.56	0.00	0.56
		0.00	0.56	0.00	0.56
		0.00	0.56	0.00	0.56

Total TSS Removal =

44%

Separate Form Needs to
be Completed for Each
Outlet or BMP Train

Project: 120-365
Prepared By: KG
Date: 6/1/2021

*Equals remaining load from previous BMP (E)
which enters the BMP

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: Infiltration Basin #1 & 2

TSS Removal Calculation Worksheet	B	C	D	E	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
	Infiltration Basin	0.80	1.00	0.80	0.20
		0.00	0.20	0.00	0.20
		0.00	0.20	0.00	0.20
		0.00	0.20	0.00	0.20
		0.00	0.20	0.00	0.20

Total TSS Removal =

80%

Separate Form Needs to
be Completed for Each
Outlet or BMP Train

Project: 20-365
Prepared By: KG
Date: 10/29/2021

*Equals remaining load from previous BMP (E)
which enters the BMP

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: Subsurface System

TSS Removal Calculation Worksheet	B	C	D	E	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
	Subsurface Infiltration Structure	0.80	1.00	0.80	0.20
		0.00	0.20	0.00	0.20
		0.00	0.20	0.00	0.20
		0.00	0.20	0.00	0.20
		0.00	0.20	0.00	0.20

Total TSS Removal =

80%

Separate Form Needs to
be Completed for Each
Outlet or BMP Train

Project: 120-365

Prepared By: KG

Date: 10/29/2021

*Equals remaining load from previous BMP (E)
which enters the BMP

Non-automated TSS Calculation Sheet
must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

Mass. Dept. of Environmental Protection

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness is calculated. Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. **The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed** otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

Input Values		use consistent units (e.g. feet & days or inches & hours)		Conversion Table	
				inch/hour	feet/day
0.5500	R	Recharge (infiltration) rate (feet/day)		0.67	1.33
0.260	Sy	Specific yield, Sy (dimensionless, between 0 and 1)			
3.00	K	Horizontal hydraulic conductivity, Kh (feet/day)*		2.00	4.00
22.000	x	1/2 length of basin (x direction, in feet)			
25.000	y	1/2 width of basin (y direction, in feet)	hours	days	
1.000	t	duration of infiltration period (days)		36	1.50
50.000	hi(0)	initial thickness of saturated zone (feet)			
51.111	h(max)	maximum thickness of saturated zone (beneath center of basin at end of infiltration period)			
1.111	Δh(max)	maximum groundwater mounding (beneath center of basin at end of infiltration period)			

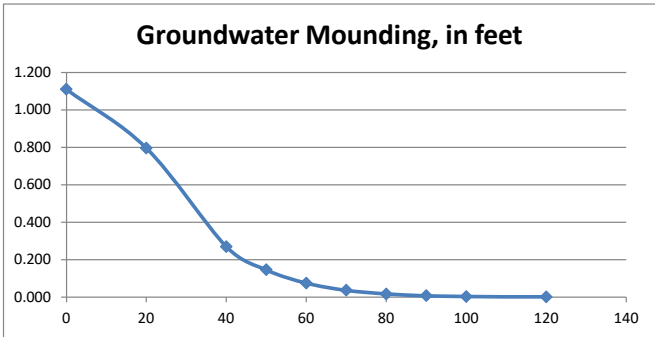
Ground-water Mounding, in feet

Distance from center of basin in x direction, in feet

1.111	0
0.797	20
0.270	40
0.146	50
0.075	60
0.037	70
0.017	80
0.008	90
0.004	100
0.001	120



Re-Calculate Now



Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

This spreadsheet will calculate the height of a groundwater mound beneath a stormwater infiltration basin. More information can be found in the U.S. Geological Survey Scientific Investigations Report 2010-5102 "Simulation of groundwater mounding beneath hypothetical stormwater infiltration basins".

The user must specify infiltration rate (R), specific yield (Sy), horizontal hydraulic conductivity (Kh), basin dimensions (x, y), duration of infiltration period (t), and the initial thickness of the saturated zone (hi(0), height of the water table if the bottom of the aquifer is the datum). For a square basin the half width equals the half length (x = y). For a rectangular basin, if the user wants the water-table changes perpendicular to the long side, specify x as the short dimension and y as the long dimension. Conversely, if the user wants the values perpendicular to the short side, specify y as the short dimension, x as the long dimension. All distances are from the center of the basin. Users can change the distances from the center of the basin at which water-table aquifer thickness are calculated. Cells highlighted in yellow are values that can be changed by the user. Cells highlighted in red are output values based on user-specified inputs. **The user MUST click the blue "Re-Calculate Now" button each time ANY of the user-specified inputs are changed** otherwise necessary iterations to converge on the correct solution will not be done and values shown will be incorrect. Use consistent units for all input values (for example, feet and days)

Input Values		use consistent units (e.g. feet & days or inches & hours)		Conversion Table	
				inch/hour	feet/day
2.5900	R	Recharge (infiltration) rate (feet/day)		0.67	1.33
0.260	Sy	Specific yield, Sy (dimensionless, between 0 and 1)			
3.00	K	Horizontal hydraulic conductivity, Kh (feet/day)*		2.00	4.00
18.500	x	1/2 length of basin (x direction, in feet)			
6.400	y	1/2 width of basin (y direction, in feet)	hours	days	
1.000	t	duration of infiltration period (days)		36	1.50
50.000	hi(0)	initial thickness of saturated zone (feet)			
51.852	h(max)	maximum thickness of saturated zone (beneath center of basin at end of infiltration period)			
1.852	Δh(max)	maximum groundwater mounding (beneath center of basin at end of infiltration period)			

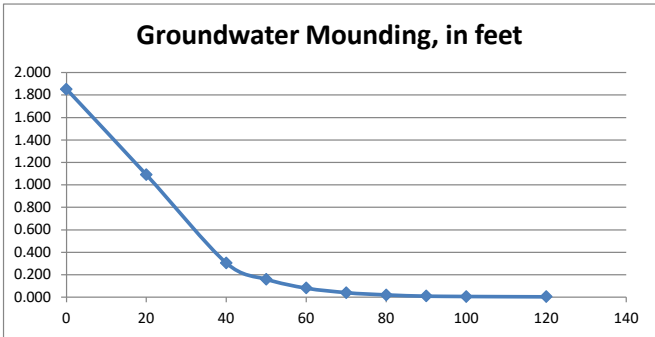
Ground-water Mounding, in feet

Distance from center of basin in x direction, in feet

1.852	0
1.091	20
0.304	40
0.160	50
0.081	60
0.040	70
0.020	80
0.010	90
0.006	100
0.004	120



Re-Calculate Now



Disclaimer

This spreadsheet solving the Hantush (1967) equation for ground-water mounding beneath an infiltration basin is made available to the general public as a convenience for those wishing to replicate values documented in the USGS Scientific Investigations Report 2010-5102 "Groundwater mounding beneath hypothetical stormwater infiltration basins" or to calculate values based on user-specified site conditions. Any changes made to the spreadsheet (other than values identified as user-specified) after transmission from the USGS could have unintended, undesirable consequences. These consequences could include, but may not be limited to: erroneous output, numerical instabilities, and violations of underlying assumptions that are inherent in results presented in the accompanying USGS published report. The USGS assumes no responsibility for the consequences of any changes made to the spreadsheet. If changes are made to the spreadsheet, the user is responsible for documenting the changes and justifying the results and conclusions.

Section III

Operation & Maintenance

OPERATION AND MAINTENANCE PLAN

PROPOSED DRAINAGE SYSTEM – DURING CONSTRUCTION

**715 Washington Street
Pembroke, Massachusetts**

Owner:

George Thibeault

599 Summer Street

Marshfield MA 02050

Contact: George Thibeault 781-789-7555

wildlandtimber@gmail.com

Party Responsible for Operation and Maintenance:

George Thibeault

599 Summer Street

Marshfield MA 02050

Contact: George Thibeault 781-789-7555

wildlandtimber@gmail.com

Source of Funding:

Operation and Maintenance of this stormwater management system will be the responsibility of the property owner to include its successor and/or assigns, as the same may appear on record with the appropriate register of deeds.

During Construction:

During periods of active construction the stormwater management system shall be inspected on a weekly basis and within 24 hours of a storm event of greater than ½". Maintenance tasks shall be performed monthly or after significant rainfall events of 1" of rain or greater. During construction, silt-laden runoff shall be prevented from entering the drainage system and off-site properties. Temporary swales shall be constructed as needed during construction to direct runoff to sediment traps. Infiltration systems shall not be placed in service until after the installation of base course pavement and vegetative stabilization of the areas contributing to the systems.

During dewatering operations, all water pumped from the dewatering shall be directed to a "dirt bag" pumped sediment removal system (or approved equal) as manufactured by ACF Environmental. The unit shall be placed on a crushed stone blanket. Disposal of such "dirt bag" shall occur when the device is full and can no longer effectively filter sediment or allow water to pass at a reasonable flow rate. Disposal of this unit shall be the responsibility of the contractor and shall be as directed by the owner in accordance with applicable local, state, and federal guidelines and regulations.

Stabilized construction entrances shall be placed at the entrances and shall consist of 1½" to 2" stone and be constructed as shown on the approved plans.

All erosion and sedimentation control measures shall be in place prior to the commencement of any site work or earthwork operations, and shall be maintained during construction, and shall remain in place until all site work is complete and ground cover is established.

Heavy equipment shall not be used on basin bottoms.

All exposed soils not to be paved shall be stabilized as soon as practical. Seed mixes shall only be applied during appropriate periods as recommended by the seed supplier, typically May 1 to October 15. Any exposed soils that cannot be stabilized by vegetation during these dates shall be stabilized with hay bales, hay mulch, check dams, jute netting or other acceptable means.

Once each structure is in place, it should be maintained in accordance with the procedures described in the post-construction Operations and Maintenance Plan.

During dry periods where dust is created by construction activities the following control measures should be implemented.

- Sprinkling – The contractor may sprinkle the ground along haul roads and traffic areas until moist.
- Vegetative cover – Areas that are not expected to be disturbed regularly may be stabilized with vegetative cover.
- Mulch – Mulching can be used as a quick and effective means of dust control in recently disturbed areas.
- Spray on chemical soil treatments may be utilized. Application rates shall conform to manufacturers recommendations.

For additional information, refer to Performance, Standards and Guidelines for Stormwater Management in Massachusetts, published by the Department of Environmental Protection.

STORMWATER MANAGEMENT
BEST MANAGEMENT PRACTICES
INSPECTION SCHEDULE AND EVALUATION CHECKLIST – CONSTRUCTION PHASE

PROJECT LOCATION: 715 Washington Street, Pembroke
 Latest Revision: 10/29/2021

Stormwater Control Manager: _____

Stamp

Best Management Practice	Inspection Frequency (1)	Date Inspected	Inspector	Minimum Maintenance and Key Items to Check	Cleaning/Repair Needed yes/no List items	Date of Cleaning/Repair	Performed By	Water Level in Detention System
Silt socks & swales and silt traps	After every major storm event							
Dewatering Operations	Daily-during actual dewatering							
Temporary Construction Entrance	Daily or as needed.							

(1) Refer to the Massachusetts Stormwater Management, Volume Two: Stormwater Technical Handbook for recommendations regarding frequency for inspection and maintenance of specific BMPs.

Limited or no use of sodium chloride salts, fertilizers or pesticides recommended. Slow release fertilizer recommended.
 Other notes:(Include deviations from: Con Com Order of Conditions, PB Approval, Construction Sequence and Approved Plan)

OPERATION AND MAINTENANCE PLAN
PROPOSED DRAINAGE SYSTEM – POST CONSTRUCTION
715 Washington Street
Pembroke, Massachusetts

Owner:

George Thibeault

599 Summer Street

Marshfield MA 02050

Contact: George Thibeault 781-789-7555

wildlandtimber@gmail.com

Party Responsible for Operation and Maintenance:

After construction is complete the owner will be the party responsible for operation and maintenance of the drainage system. When the property is conveyed, the new owner will be the party responsible for operation and maintenance.

Source of Funding:

Operation and Maintenance of this stormwater management system will be the responsibility of the owner. The estimated annual budget for the operation and maintenance of the stormwater system is \$1000.

Schedule for Inspection and Maintenance:

Deep Sump Catch Basins

Deep sump catch basins shall become part of the roadway system and shall be inspected after every major storm event during construction and cleaned when sediment exceeds 18” depth. After construction when all slopes have been stabilized, basins shall be cleaned a minimum of twice per year. Disposal of the accumulated sediment shall be in accordance with applicable local, state, and federal guidelines and regulations.

Sediment Forebays (at grade)

At a minimum, At a minimum, **the sediment forebays shall be inspected after every significant rainfall events and cleaned of any accumulated debris**, inspect sediment forebays monthly and clean them out at least four times per year. Stabilize the floor and sidewalls of the sediment forebay before making it operational. When mowing grasses, keep the grass height lower than 6 inches, check for signs of riling and gulying and repair as needed. After removing sediment, replace any vegetation damaged during the clean-out by re-seeding or sodding. When re-seeding, incorporate practices such as hydro seeding with a tackifier, blanket or similar practice to ensure that no scour occurs in the forebay, while the seeds germinate and develop roots.

Any sediment removed from the infiltration systems should be disposed of in accordance with Town, State and Federal Regulations.

Infiltration Basin

The Infiltration BMP’s should be inspected on a quarterly basis: additional inspections should be scheduled during the first few months to make sure the vegetation is established adequately and also following major storm events. Additional inspections are required following any storm event that exceeds 2.5 inches in 24-hour period (the one-year frequency storm). Evidence of

standing water for more than 48 hours following a storm would indicate possible failure of the infiltration surface. In that case, a qualified professional engineer should be retained to assess the cause of failure and recommend corrective action, which should be immediately implemented to restore the function of the system. The basin should be inspected for slope integrity, soil moisture, vegetative health, soil stability, soil compaction, soil erosion, ponding and sedimentation. The basin should be mowed twice per year.

Regular maintenance tasks include mowing, watering, and weed and pest control. Only organic fertilizers, weed and pest control will be utilized.

Sediment and debris should be removed manually, at least twice per year, before the vegetation is impacted adversely. Periodic mowing (Twice per year) may be required to maintain the dense growth of vegetation. Care should be taken to protect basin from snow removal procedures and off street parking.

Subsurface Drainage Systems Maintenance Schedule

Activity Time of Year Frequency

Inspect Inlets and access manholes twice per year. Remove any debris that might clog the system.

After construction, the systems should be inspected for standing water 1-2 days after any significant rainfall exceeding 1" of rainfall in 24 hours or major storm event. If the system is continuing to hold standing water after 2 days the owner should have it inspected and repaired. The systems should also be inspected to verify whether infiltration function has been lost. If infiltration capacity has become degraded, it should be restored under the direction of a qualified professional.

The subsurface systems should be inspected twice per year and at least once per year by a drainage system professional to ensure that the system is operating as intended. The owner shall implement and pay for the inspector's recommendations.

Lawn Fertilization

Lawn fertilizer shall be slow release and limited to 3 lbs per 1000 s.f. per year.

Stormwater Contamination Prevention

Exterior storage of hazardous materials including deicing chemicals, fertilizers, herbicides, pesticides, and other hazardous materials is prohibited. All hazardous materials are to be stored inside of the buildings no exterior storage of hazardous materials is allowed. Individual storage unit users shall be notified of the prohibition of illicit discharges to the stormwater management system.

Illicit Discharges

Illicit discharges to the stormwater management system are discharges that are not entirely comprised of stormwater. Illicit discharges are prohibited from the stormwater management system and the stormwater management system shall be inspected for illicit discharges annually.

The following is a list of discharges that are allowed under the EPA Construction General Permit (CGP) provided that appropriate stormwater controls are designed, installed, and maintained:

- a. Stormwater discharges, including stormwater runoff, snowmelt runoff, and surface runoff and drainage, associated with construction activity under 40 CFR §122.26(b)(14) or § 122.26(b)(15)(i);
- b. Stormwater discharges designated by EPA as needing a permit under 40 CFR § 122.26(a)(1)(v) or §122.26(b)(15)(ii);

c. Stormwater discharges from construction support activities (*e.g., concrete or asphalt batch plants, equipment staging yards, material storage areas, excavated material disposal areas, borrow areas*) provided:

- i. The support activity is directly related to the construction site required to have permit coverage for stormwater discharges;
- ii. The support activity is not a commercial operation, nor does it serve multiple unrelated construction projects;
- iii. The support activity does not continue to operate beyond the completion of the construction activity at the project it supports; and
- iv. Stormwater controls are implemented in accordance with Part 2 of the CGP and, if applicable, Part 3 of the CGP, for discharges from the support activity areas.

The following non-stormwater discharges from your construction activity, provided that, with the exception of water used to control dust and to irrigate areas to be vegetatively stabilized, these discharges are not routed to areas of exposed soil on your site and you comply with any applicable requirements for these discharges in Part 2 of the CGP:

- i. Discharges from emergency fire-fighting activities;
- ii. Fire hydrant flushings;
- iii. Landscape irrigation;
- iv. Water used to wash vehicles and equipment, provided that there is no discharge of soaps, solvents, or detergents used for such purposes;
- v. Water used to control dust;
- vi. Potable water including uncontaminated water line flushings;
- vii. Routine external building washdown that does not use detergents;
- viii. Pavement wash waters provided spills or leaks of toxic or hazardous materials have not occurred (unless all spill material has been removed) and where detergents are not used. You are prohibited from directing pavement wash waters directly into any surface water, storm drain inlet, or stormwater conveyance, unless the conveyance is connected to a sediment basin, sediment trap, or similarly effective control;
- ix. Uncontaminated air conditioning or compressor condensate;
- x. Uncontaminated, non-turbid discharges of ground water or spring water;
- xi. Foundation or footing drains where flows are not contaminated with process materials such as solvents or contaminated ground water; and
- xii. Construction dewatering water that has been treated by an appropriate control under Part 2.1.3.4 of the CGP; and
- e. Discharges of stormwater listed above in Parts a, b, and c, or authorized nonstormwater discharges in Part d above, commingled with a discharge authorized by a different NPDES permit and/or a discharge that does not require NPDES permit authorization.

Snow Removal and De-icing

Snow removal will be the responsibility of the Owner. Snow will be plowed from Parking areas and driveways and shoveled or removed with a snow blower from walkways. Snow will be stored along roadways and walkways as shown on the Site Plan. If additional stockpiling area is needed, excess snow will be removed from the site with proper off-site disposal. Snow shall be stockpiled in areas where melting will be directed through the drainage systems and not directly to the wetlands. Stockpiling within any infiltration areas is prohibited.

Inspections

Yearly inspections of the stormwater management system shall be performed and an Inspection Schedule and Evaluation Checklist shall be maintained by the Owner and made available to regulatory officials if requested. Copies of the receipts for cleaning of the systems shall also be maintained.

The Owner shall be responsible to secure the services of a Licensed Engineer on an on-going basis. The inspector shall review the project with respect to the following:

- Proper installation and performance of the Stormwater Management System.
- Review of the controls to determine any damaged or ineffective controls.
- Corrective actions.

The Engineer shall prepare, stamp and submit, to the Owner, a report documenting the findings and should request the required maintenance or repair for the pollution prevention controls when the inspector finds that it is necessary for the control to be effective (see attached Inspection Schedule and Evaluation Checklist). The inspector shall notify the Owner to make the changes.

The owner and/or their employees responsible for the O&M of the stormwater management system shall be trained annually. Records of trained individuals shall be kept and submitted to the town with the check list. The records shall indicate the latest training date.

The attached inspection form shall be retained and kept available for a minimum of three years.

For additional information, refer to Performance, Standards and Guidelines for Stormwater Management in Massachusetts, published by the Department of Environmental Protection

Definition of Major Storm Event

For the purposes of this operation and maintenance plan a major storm event should be defined as a rainfall of such intensity or duration that causes observable movement of sediment on the roadway or site. It is the intent of this plan to prevent this sediment from entering the drainage system. Prior to stabilization of the site this may occur more frequently with less intense storms. As the site is stabilized with ground cover the movement of sediment will only occur during more severe storms.

For additional information, refer to Performance Standards and Guidelines for Stormwater Management in Massachusetts, published by the Department of Environmental Protection.

STORMWATER MANAGEMENT
BEST MANAGEMENT PRACTICES

INSPECTION SCHEDULE AND EVALUATION CHECKLIST – POST CONSTRUCTION PHASE

PROJECT LOCATION: 715 Washington Street, Pembroke

Latest Revision 10/29/2021

Best Management Practice	Inspection Frequency (1)	Date Inspected	Inspector	Minimum Maintenance and Key Items to Check	Cleaning/Repair Needed yes/no List items	Date of Cleaning/Repair	Performed By	Water Level in Drainage System
*Infiltration Basin	Twice per year							
Deep sump catch basins	Twice per year							
Subsurface System	Twice per year							
	Twice per year							
	Weekly							

*At a minimum, the sediment forebays shall be inspected after every significant rainfall events and cleaned of any accumulated debris

(1) Refer to the Massachusetts Stormwater Management, Volume Two: Stormwater Technical Handbook for recommendations regarding frequency for inspection and maintenance of specific BMPs.

(2) records shall be kept for a minimum of three years.

Limited or no use of sodium chloride salts, fertilizers or pesticides recommended. Slow release fertilizer recommended.

Other notes:(Include deviations from: Con Com Order of Conditions, PB Approval, Construction Sequence and Approved Plan)

Stormwater Control Manager: _____

Stamp

Deep Sump Catch Basin



Description: Deep sump catch basins, also known as oil and grease or hooded catch basins, are underground retention systems designed to remove trash, debris, and coarse sediment from stormwater runoff, and serve as temporary spill containment devices for floatables such as oils and greases.

Ability to meet specific standards

Standard	Description
2 - Peak Flow	Provides no peak flow attenuation
3 - Recharge	Provides no groundwater recharge
4 - TSS Removal	25% TSS removal credit when used for pretreatment. Because of their limited effectiveness and storage capacity, deep sump catch basins receive credit for removing TSS only if they are used for pretreatment and designed as off-line systems.
5 - Higher Pollutant Loading	Recommended as pretreatment BMP. Although provides some spill control capability, a deep sump catch basin may not be used in place of an oil grit separator or sand filter for land uses that have the potential to generate runoff with high concentrations of oil and grease such as: high-intensity-use parking lots, gas stations, fleet storage areas, vehicle and/or equipment maintenance and service areas.
6 - Discharges near or to Critical Areas	May be used as pretreatment BMP. not an adequate spill control device for discharges near or to critical areas.
7 - Redevelopment	Highly suitable.

Advantages/Benefits:

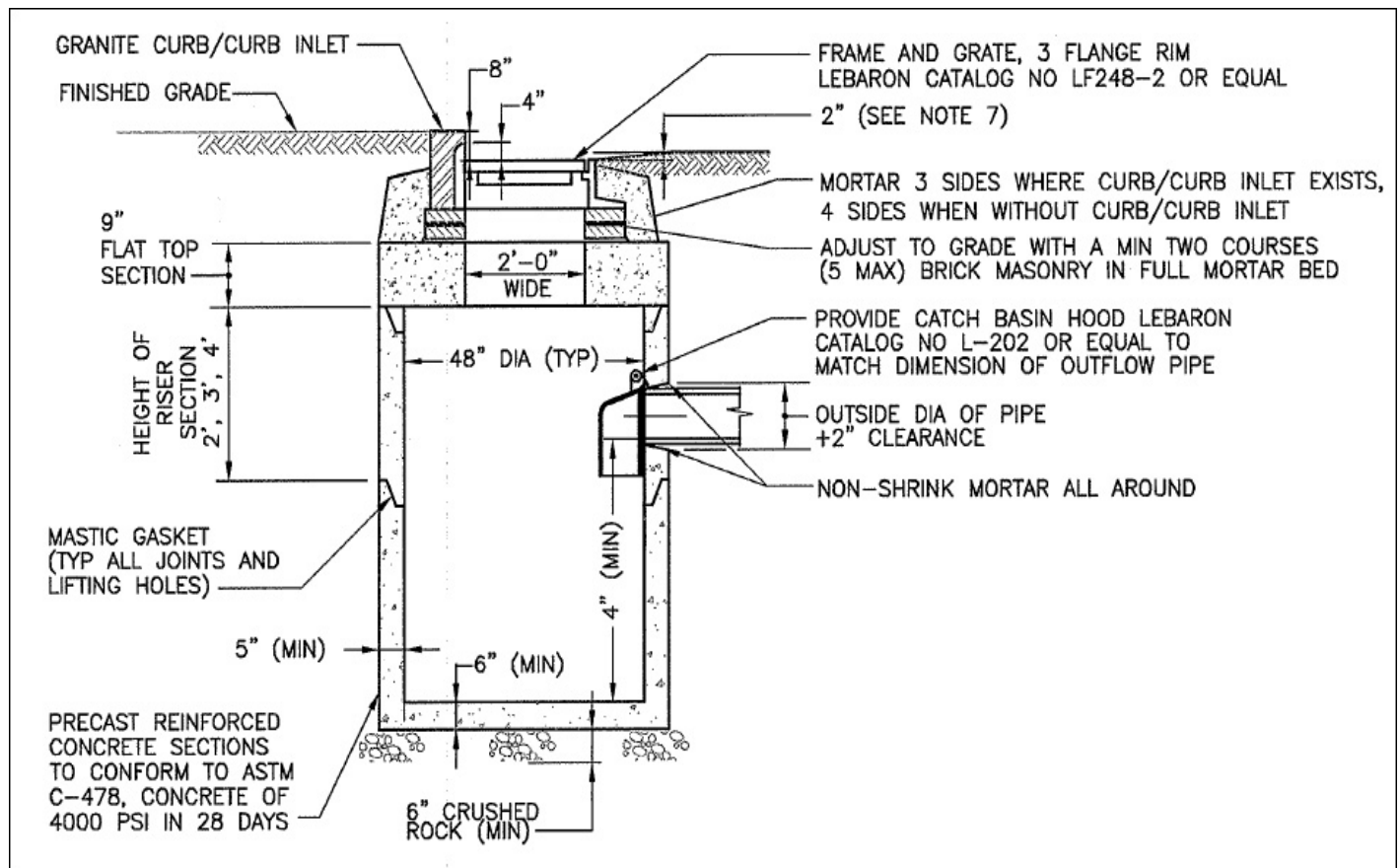
- Located underground, so limited lot size is not a deterrent.
- Compatible with subsurface storm drain systems.
- Can be used for retrofitting small urban lots where larger BMPs are not feasible.
- Provide pretreatment of runoff before it is delivered to other BMPs.
- Easily accessed for maintenance.
- Longevity is high with proper maintenance.

Disadvantages/Limitations:

- Limited pollutant removal.
- Expensive to install and maintain, resulting in high cost per unit area treated.
- No ability to control volume of stormwater
- Frequent maintenance is essential
- Requires proper disposal of trapped sediment and oil and grease
- Entrapment hazard for amphibians and other small animals

Pollutant Removal Efficiencies

- Total Suspended Solids (TSS) - 25% (for regulatory purposes)
- Nutrients (Nitrogen, phosphorus) - Insufficient data
- Metals (copper, lead, zinc, cadmium) - Insufficient data
- Pathogens (coliform, e coli) - Insufficient data



adapted from the University of New Hampshire

Maintenance

Activity	Frequency
Inspect units	Four times per year
Clean units	Four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin.

Special Features

All deep sump catch basins must include hoods. For MassHighway projects, consult the Stormwater Handbook for Highways and Bridges for hood requirements.

LID Alternative

Reduce Impervious Surface

Disconnect rooftop and non-rooftop runoff

Vegetated Filter Strip

Deep Sump Catch Basin

Suitable Applications

- Pretreatment
- Residential subdivisions
- Office
- Retail

Design Considerations

- The contributing drainage area to any deep sump catch basin should not exceed $\frac{1}{4}$ acre of impervious cover.
- Design and construct deep sump catch basins as off-line systems.
- Size the drainage area so that the flow rate does not exceed the capacity of the inlet grate.
- Divert excess flows to another BMP intended to meet the water quantity requirements (peak rate attenuation) or to a storm drain system. An off-line design enhances pollutant removal efficiency, because it prevents the resuspension of sediments in large storms.

Make the sump depth (distance from the bottom of the outlet pipe to the bottom of the basin) at least four feet times the diameter of the outlet pipe and more if the contributing drainage area has a high sediment load. The minimum sump depth is 4 feet. Double catch basins, those with 2 inlet grates, may require deeper sumps. Install the invert of the outlet pipe at least 4 feet from the bottom of the catch basin grate.

The inlet grate serves to prevent larger debris from entering the sump. To be effective, the grate must have a separation between the grates of one square inch or less. The inlet openings must not allow flows greater than 3 cfs to enter the deep sump catch basin. If the inlet grate is designed with a curb cut, the grate must reach the back of the curb cut to prevent bypassing. The inlet grate must be constructed of a durable material and fit tightly into the frame so it won't be dislodged by automobile traffic. The inlet grate must not be welded to the frame so that sediments may be easily removed. To facilitate maintenance, the inlet grate must be placed along the road shoulder or curb line rather than a traffic lane.

Note that within parking garages, the State Plumbing Code regulates inlet grates and other stormwater

management controls. Inlet grates inside parking garages are currently required to have much smaller openings than those described herein.

To receive the 25% removal credit, hoods must be used in deep sump catch basins. Hoods also help contain oil spills. MassHighway may install catch basins without hoods provided they are designed, constructed, operated, and maintained in accordance with the Mass Highway Stormwater Handbook.

Install the weep hole above the outlet pipe. Never install the weep hole in the bottom of the catch basin barrel.

Site Constraints

A proponent may not be able to install a deep sump catch basin because of:

- Depth to bedrock;
- High groundwater;
- Presence of utilities; or
- Other site conditions that limit depth of excavation because of stability.

Maintenance

Regular maintenance is essential. Deep sump catch basins remain effective at removing pollutants only if they are cleaned out frequently. One study found that once 50% of the sump volume is filled, the catch basin is not able to retain additional sediments.

Inspect or clean deep sump basins at least four times per year and at the end of the foliage and snow-removal seasons. Sediments must also be removed four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin. If handling runoff from land uses with higher potential pollutant loads or discharging runoff near or to a critical area, more frequent cleaning may be necessary.

Clamshell buckets are typically used to remove sediment in Massachusetts. However, vacuum trucks are preferable, because they remove more trapped sediment and supernatant than clamshells. Vacuuming is also a speedier process and is less likely to snap the cast iron hood within the deep sump catch basin.

Always consider the safety of the staff cleaning deep sump catch basins. Cleaning a deep sump catch basin within a road with active traffic or even within a parking lot is dangerous, and a police detail may be necessary to safeguard workers.

Although catch basin debris often contains concentrations of oil and hazardous materials such as petroleum hydrocarbons and metals, MassDEP classifies them as solid waste. Unless there is evidence that they have been contaminated by a spill or other means, MassDEP does not routinely require catch basin cleanings to be tested before disposal. Contaminated catch basin cleanings must be evaluated in accordance with the Hazardous Waste Regulations, 310 CMR 30.000, and handled as hazardous waste.

In the absence of evidence of contamination, catch basin cleanings may be taken to a landfill or other facility permitted by MassDEP to accept solid waste, without any prior approval by MassDEP. However, some landfills require catch basin cleanings to be tested before they are accepted.

With prior MassDEP approval, catch basin cleanings may be used as grading and shaping materials at landfills undergoing closure (see Revised Guidelines for Determining Closure Activities at Inactive Unlined Landfill Sites) or as daily cover at active landfills. MassDEP also encourages the beneficial reuse of catch basin cleanings whenever possible. A Beneficial Reuse Determination is required for such use.

MassDEP regulations prohibit landfills from accepting materials that contain free-draining liquids. One way to remove liquids is to use a hydraulic lift truck during cleaning operations so that the material can be decanted at the site. After loading material from several catch basins into a truck, elevate the truck so that any free-draining liquid can flow back into the structure. If there is no free water in the truck, the material may be deemed to be sufficiently dry. Otherwise the catch basin cleanings must undergo a Paint Filter Liquids Test. Go to www.Mass.gov/dep/recycle/laws/cafacts.doc for information on all of the MassDEP requirements pertaining to the disposal of catch basin cleanings.

Infiltration Basins



Description: Infiltration basins are stormwater runoff impoundments that are constructed over permeable soils. Pretreatment is critical for effective performance of infiltration basins. Runoff from the design storm is stored until it exfiltrates through the soil of the basin floor.

Ability to meet specific standards

Standard	Description
2 - Peak Flow	Can be designed to provide peak flow attenuation.
3 - Recharge	Provides groundwater recharge.
4 - TSS Removal	80% TSS removal, with adequate pretreatment
5 - Higher Pollutant Loading	May be used if 44% of TSS is removed with a pretreatment BMP prior to infiltration. For some land uses with higher potential pollutant loads, use an oil grit separator, sand filter or equivalent for pretreatment prior to discharge to the infiltration basin. Infiltration must be done in compliance with 314 CMR 5.00
6 - Discharges near or to Critical Areas	Highly recommended, especially for discharges near cold-water fisheries. Requires 44% removal of TSS prior to discharge to infiltration basin
7 - Redevelopment	Typically not an option due to land area constraints

Advantages/Benefits:

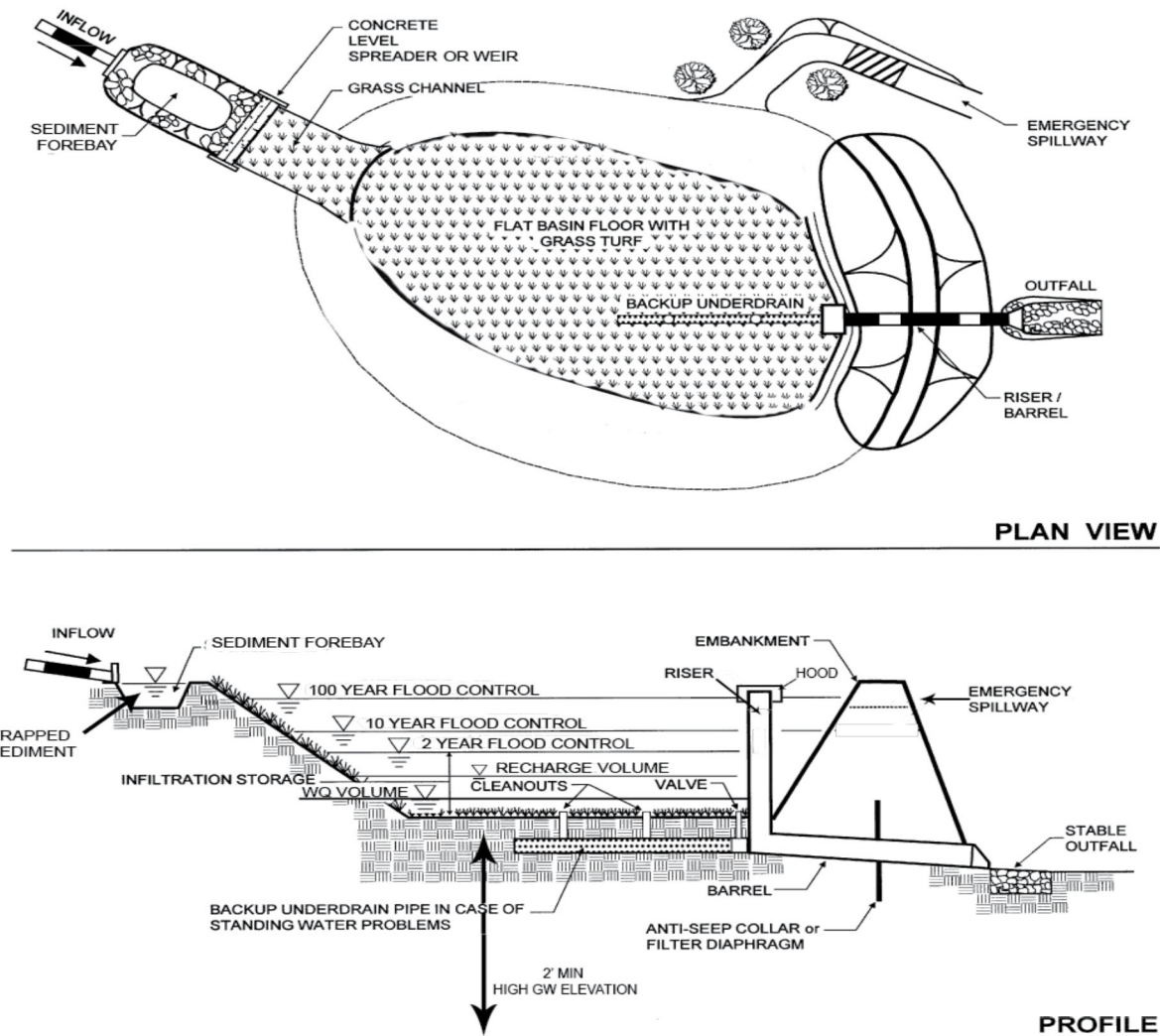
- Provides groundwater recharge.
- Reduces local flooding.
- Preserves the natural water balance of the site.
- Can be used for larger sites than infiltration trenches or structures.

Disadvantages/Limitations:

- High failure rates due to improper siting, inadequate pretreatment, poor design and lack of maintenance.
- Restricted to fairly small drainage areas.
- Not appropriate for treating significant loads of sediment and other pollutants.
- Requires frequent maintenance.
- Can serve as a “regional” stormwater treatment facility

Pollutant Removal Efficiencies

- | | |
|--|-----------------------|
| • Total Suspended Solids (TSS) | 80% with pretreatment |
| • Total Nitrogen | 50% to 60% |
| • Total Phosphorus | 60% to 70% |
| • Metals (copper, lead, zinc, cadmium) | 85% to 90% |
| • Pathogens (coliform, e coli) | 90% |



adapted from the Vermont Stormwater Manual

Maintenance

Activity	Frequency
Preventative maintenance	Twice a year
Inspect to ensure proper functioning	After every major storm during first 3 months of operation and twice a year thereafter and when there are discharges through the high outlet orifice.
Mow the buffer area, side slopes, and basin bottom if grassed floor; rake if stone bottom; remove trash and debris; remove grass clippings and accumulated organic matter	Twice a year
Inspect and clean pretreatment devices	Every other month recommended and at least twice a year and after every major storm event.

Special Features: High failure rate without adequate pretreatment and regular maintenance.

LID Alternative: Reduce impervious surfaces. Bioretention areas

Infiltration Basins

The following are variations of the infiltration basin design.

Full Exfiltration Basin Systems

These basin systems are sized to provide storage and exfiltration of the required recharge volume and treatment of the required water quality volume. They also attenuate peak discharges. Designs typically include an emergency overflow channel to discharge runoff volumes in excess of the design storm.

Partial or Off-line Exfiltration Basin Systems

Partial basin systems exfiltrate a portion of the runoff (usually the first flush or the first half inch), with the remaining runoff being directed to other BMPs. Flow splitters or weirs divert flows containing the first flush into the infiltration basin. This design is useful at sites where exfiltration cannot be achieved by downstream detention BMPs because of site condition limitations.

Applicability

The suitability of infiltration basins at a given site is restricted by several factors, including soils, slope, depth to water table, depth to bedrock, the presence of an impermeable layer, contributing

watershed area, proximity to wells, surface waters, and foundations. Generally, infiltration basins are suitable at sites with gentle slopes, permeable soils, relatively deep bedrock and groundwater levels, and a contributing watershed area of approximately 2 to 15 acres. Table IB.1 presents the recommended site criteria for infiltration basins.

Pollution prevention and pretreatment are particularly important at sites where infiltration basins are located. A pollution prevention program that separates contaminated and uncontaminated runoff is essential. Uncontaminated runoff can be infiltrated directly, while contaminated runoff must be collected and pretreated using an appropriate combination of BMPs and then rerouted to the infiltration basin. This approach allows uncontaminated stormwater to be infiltrated during and immediately after the storm and permits the infiltration of contaminated stormwater after an appropriate detention time. The Pollution Prevention and Source Control Plan required by Stormwater Standard 4 must take these factors into account. For land uses with higher potential pollutant loads, provide a bypass to divert contaminated stormwater from the infiltration basin in storms larger than the design storm.

Table IB.1 - Site Criteria for Infiltration Basins
1. The contributing drainage area to any individual infiltration basin should be restricted to 15 acres or less.
2. The minimum depth to the seasonal high water table, bedrock, and/or impermeable layer should be 2 ft. from the bottom of the basin.
3. The minimum infiltration rate is 0.17 inches per hour. Infiltration basins must be sized in accordance with the procedures set forth in Volume 3.
4. One soil sample for every 5000 ft. of basin area is recommended, with a minimum of three samples for each infiltration basin. Samples should be taken at the actual location of the proposed infiltration basin so that any localized soil conditions are detected.
5. Infiltration basins should not be used at sites where soil have 30% or greater clay content, or 40% or greater silt clay content.
6. Infiltration basins should not be placed over fill materials.
7. The following setback requirements should apply to infiltration basin installations: <ul style="list-style-type: none"> • Distance from any slope greater than 15% - Minimum of 50 ft. • Distance from any soil absorption system- Minimum of 50 ft. • Distance from any private well - Minimum of 100 ft., additional setback distance may be required depending on hydrogeological conditions. • Distance from any public groundwater drinking supply wells - Zone I radius, additional setback distance may be required depending on hydrogeological conditions. • Distance from any surface drinking water supply - Zone A • Distance from any surface water of the commonwealth (other than surface water supplies and their tributaries) - Minimum of 50 ft. • Distance from any building foundations including slab foundations without basements - Minimum of 10 ft. downslope and 100 ft. upslope.

Prior to pretreatment, implement the pollution prevention and source control program specified in the Pollution Prevention and Source Control Plan to reduce the concentration of pollutants in the discharge. Program components include careful management of snow and deicing chemicals, fertilizers, herbicides, and pest control. The Plan must prohibit snow disposal in the basin and include measures to prevent runoff of stockpiled snow from entering the basin. Stockpiled snow contains concentrations of sand and deicing chemicals. At industrial sites, keep raw materials and wastes from being exposed to precipitation. Select pretreatment BMPs that remove coarse sediments, oil and grease, and floatable organic and inorganic materials, and soluble pollutants.

Effectiveness

Infiltration basins are highly effective treatment systems that remove many contaminants, including TSS. However, infiltration basins are not intended to remove coarse particulate pollutants. Use a pretreatment device to remove them before they enter the basin. The pollutant removal efficiency of the basin depends on how much runoff is exfiltrated by the basin.

Infiltration basins can be made to control peak discharges by incorporating additional stages in the design. To do this, design the riser outlet structure or weir with multiple orifices, with the lowest orifice set to achieve storage of the full recharge volume required by Standard 3. Design the upper orifices using the same procedures as extended detention basins. The basins can also be designed to achieve exfiltration of storms greater than the required recharge volume. However, in such cases, make sure the soils are permeable enough to allow the basin to exfiltrate the entire volume in a 72-hour period. This may necessitate increasing the size of the floor area of the basin. Generally, it is not economically feasible to provide storage for large infrequent storms, such as the 100-year 24-hour storm.

Planning Considerations

Carefully evaluate sites before planning infiltration basins, including investigating soils, depth to bedrock, and depth to water table. Suitable parent soils should have a minimum infiltration rate of 0.17 inches per hour. Infiltration basin must be sized in accordance with the procedures set forth in Volume 3. The slopes of the contributing drainage area for the infiltration basin must be less than 5%.

Design

Infiltration basins are highly effective treatment and disposal systems when designed properly. The first step before design is providing source control and implementing pollution prevention measures to minimize sediment and other contaminants in runoff discharged to the infiltration basin. Next, consider the appropriate pretreatment BMPs.

Design pretreatment BMPs to pretreat runoff before stormwater reaches the infiltration basin. For Critical Areas, land uses with potentially higher pollutant loads, and soils with rapid infiltration rates (greater than 2.4 inches/hour), pretreatment must remove at least 44% of the TSS. Proponents may comply with this requirement by proposing two pretreatment BMPs capable of removing 25% TSS. However, the issuing authorities (i.e., Conservation Commissions or MassDEP) may require additional pretreatment for other constituents beyond TSS for land uses with higher potential pollutant loads. If the land use has the potential to generate stormwater runoff with high concentrations of oil and grease, treatment by an oil grit separator or equivalent is required before discharge to the infiltration basin.

For discharges from areas other than Critical Areas, land uses with potentially higher pollutant loads, and soils with rapid infiltration rates, MassDEP also requires some TSS pretreatment. Common pretreatment for infiltration basins includes aggressive street sweeping, deep sump catch basins, oil/grit separators, vegetated filter strips, water quality swales, or sediment forebays. Fully stabilize all land surfaces contributing drainage to the infiltration practice after construction is complete to reduce the amount of sediment in runoff that flows to the pretreatment devices.

Always investigate site conditions. Infiltration basins must have a minimum separation from seasonal high groundwater of at least 2 feet. Greater separation is necessary for bedrock. If there is bedrock on the site, conduct an analysis to determine the appropriate vertical separation. The greater the distance from the bottom of the basin media to the seasonal high groundwater elevation, the less likely the basin will fail to drain in the 72-hour period following precipitation.

Determine soil infiltration rates using samples collected at the proposed location of the basin. Take one soil boring or dig one test pit for every 5,000 feet

of basin area, with a minimum of three borings for each infiltration basin. Conduct the borings or test pits in the layer where infiltration is proposed. For example, if the A and B horizons are to be removed and the infiltration will be through the C horizon, conduct the borings or test pits through the C horizon. MassDEP requires that borings be at least 20 feet deep or extend to the depth of the limiting layer.

For each bore hole or test pit, evaluate the saturated hydraulic conductivity of the soil, depth to seasonal high groundwater, NRCS soil textural class, NRCS Hydrologic Soil Group, and the presence of fill materials in accordance with Volume 3. Never locate infiltration basins above fill. Never locate infiltration basins in Hydrologic Soil Group “D” soils. The minimum acceptable final soil infiltration rate is 0.17 inches per hour. Design the infiltration basin based on the soil evaluation set forth in Volume 3.

If the proposed basin is determined to be in Hydrologic Soil Group “C” soils, incorporate measures in the design to reduce the potential for clogging, such as providing more pretreatment or greater media depth to provide additional storage. Never use the results of a Title 5 percolation test to estimate a saturated hydraulic conductivity rate, because it tends to greatly overestimate the rate that water will infiltrate into the subsurface.

Estimate seasonal high groundwater based on soil mottles or through direct observation when borings are conducted in April or May, when groundwater levels are likely to be highest. If it is difficult to determine the seasonal high groundwater elevation from the borings or test pits, then use the Frimpter method developed by the USGS (Massachusetts/Rhode Island District Office) to estimate seasonal high groundwater. After estimating the seasonal high groundwater using the Frimpter method, re-examine the bore holes or test pits to determine if there are any field indicators that corroborate the Frimpter method estimate.

Stabilize inlet channels to prevent incoming flow velocities from reaching erosive levels, which can scour the basin floor. Riprap is an excellent inlet stabilizer. Design the riprap so it terminates in a broad apron, thereby distributing runoff more evenly over the basin surface to promote better infiltration.

At a minimum, size the basin to hold the required recharge volume. Determine the required recharge

volume using either the static or dynamic methods set forth in Volume 3. Remember that the required storage volume of an infiltration basin is the sum of the quantity of runoff entering the basin from the contributing area and the precipitation directly entering the basin. Include one foot of freeboard above the total of the required recharge volume and the direct precipitation volume to account for design uncertainty. When applying the dynamic method to size the basin, use only the bottom of the basin (i.e., do not include side wall exfiltration) for the effective infiltration area.

Design the infiltration basin to exfiltrate in no less than 72 hours. Consider only the basin floor as the effective infiltration area when determining whether the basin meets this requirement.

Design the basin floor to be as flat as possible to provide uniform ponding and exfiltration of the runoff. Design the basin floor to have as close to a 0% slope as possible. In no case shall the longitudinal slope exceed 1%. Enhanced deposition of sediment in low areas may clog the surface soils, resulting in reduced infiltration and wet areas. Design the side slopes of the basin to be no steeper than 3:1 (horizontal: vertical) to allow for proper vegetative stabilization, easier mowing, easier access, and better public safety.

For basins with a 1% longitudinal slope, it will be necessary to incorporate cells into the design, making sure that the depth of ponded water does not exceed 2 feet, because sloped basin floors cause water to move downhill, thereby decreasing the likelihood of infiltration. Make lateral slopes flat (i.e., 0% slope).

After the basin floor is shaped, place soil additives on the basin floor to amend the soil. The soil additives shall include compost, properly aged to kill any seed stock contained within the compost. Do not put biosolids in the compost. Mix native soils that were excavated from the A or B horizons to create the basin with the compost, and then scarify the native

materials and compost into the parent material using a chisel plow or rotary device to a depth of 12 inches. Immediately after constructing the basin, stabilize its bottom and side slopes with a dense turf of water-tolerant grass. Use low-maintenance, rapidly germinating grasses, such as fescues. The selected grasses must be capable of surviving in both wet and dry conditions. Do not use sod, which can prevent roots from directly contacting the underlying soil. During the first two months, inspect the newly established vegetation several times to determine if any remedial actions (e.g., reseeding, irrigating) are necessary.

Never plant trees or shrubs within the basin or on the impounding embankments as they increase the chance of basin failure due to root decay or subsurface disturbance. The root penetration and thatch formation of the turf helps to maintain and may even enhance the original infiltration capacity. Soluble nutrients are taken up by the turf for growth, improving the pollutant removal capacity. Dense turf will impede soil erosion and scouring of the basin floor.

In place of turf, use a basin liner of 6 to 12 inches of fill material, such as coarse sand. Clean and replace this material as needed. Do not use loose stone, riprap, and other irregular materials requiring hand removal of debris and weeds.

Design embankments and spillways to conform to the regulatory guidelines of the state's Office of Dam Safety (302 CMR 10.00). Design infiltration basins to be below surrounding grade to avoid issues related to potential embankment failure. All infiltration basins must have an emergency spillway capable of bypassing runoff from large storms without damage to the impounding structure. Design the emergency spillway to divert the storm associated with brimful conditions without impinging upon the structural integrity of the basin. The brimful condition could be the required recharge volume or a design storm (such as the 2-year, 10-year, or 100-year storm if the basin is designed to provide peak rate attenuation in addition to exfiltration). The storm associated with the brimful conditions should not include the one foot of freeboard required to account for design uncertainty. Design the emergency spillway to shunt water toward a location where the water will not damage wetlands or buildings. A common error is to direct the spillway

runoff toward an adjoining property not owned by an applicant. If the emergency spillway is designed to drain the emergency overflow toward an adjoining property, obtain a drainage easement and submit it to the Conservation Commission as part of the Wetlands NOI submission. Place vegetative buffers around the perimeter of the basin for erosion control and additional sediment and nutrient removal.

Monitoring wells: Install one monitoring well in the basin floor per every 5,000 square feet of basin floor. Make sure the monitoring well(s) extend 20 feet beneath the basin floor or to the limiting layer, whichever is higher.

Access: Include access in the basin design. The area at the top of the basin must provide unimpeded vehicular access around the entire basin perimeter. The access area shall be no less than 15 feet.

Inlet Structures: Place inlet structures at one longitudinal end of the basin, to maximize the flow path from the inlet to the overflow outlet. A common error is to design multiple inlet points around the entire basin perimeter.

Outlet structures: Infiltration basins must include an overflow outlet in addition to an emergency spillway. Whether using a single orifice or multiple orifices in the design, at a minimum, set the lowest orifice at or above the required recharge volume.

Drawdown device: Include a device to draw the basin down for maintenance purposes. If the basin includes multiple cells, include a drawdown device for each cell.

Fences: Do not place fences around basins located in Riverfront Areas, as required by 310 CMR 10.58(4)(d)1.d. to avoid impeding wildlife movement. In such cases, consider including a safety bench as part of the design.

Construction

Prior to construction, rope or fence off the area selected for the infiltration basin. Never allow construction equipment to drive across the area intended to serve as the infiltration basin.

Never use infiltration basins as temporary sediment traps for construction activities.

To limit smearing or compacting soils, never construct the basin in winter or when it is raining. Use light earth-moving equipment to excavate the infiltration basin because heavy equipment compacts the soils beneath the basin floor and side slopes and reduces infiltration capacity. Because some compaction of soils is inevitable during construction, add the required soil amendments and deeply till the basin floor with a rotary tiller or a disc harrow to a depth of 12 inches to restore infiltration rates after final grading.

Use proper erosion/sediment control during construction. Immediately following basin construction, stabilize the floor and side slopes of the basin with a dense turf of water-tolerant grass. Use low maintenance, rapidly germinating grasses, such as fescues. Do not sod the basin floor or side slopes. After the basin is completed, keep the basin roped or fenced off while construction proceeds on other parts of the site. Never direct construction period drainage to the infiltration basin. After construction is completed, do not direct runoff into the basin until the bottom and side slopes are fully stabilized.

Maintenance

Infiltration basins are prone to clogging and failure, so it is imperative to develop and implement aggressive maintenance plans and schedules. Installing the required pretreatment BMPs will significantly reduce maintenance requirements for the basin.

The Operation and Maintenance Plan required by Standard 9 must include inspections and preventive maintenance at least twice a year, and after every time drainage discharges through the high outlet orifice. The Plan must require inspecting the pretreatment BMPs in accordance with the minimal requirements specified for those practices and after every major storm event. A major storm event is defined as a storm that is equal to or greater than the 2-year, 24-hour storm (generally 2.9 to 3.6 inches in a 24-hour period, depending in geographic location in Massachusetts).

Once the basin is in use, inspect it after every major storm for the first few months to ensure it is stabilized and functioning properly and if necessary take corrective action. Note how long water remains standing in the basin after a storm; standing water within the basin 48 to 72 hours after a storm indicates that the infiltration capacity may

have been overestimated. If the ponding is due to clogging, immediately address the reasons for the clogging (such as upland sediment erosion, excessive compaction of soils, or low spots).

Thereafter, inspect the infiltration basin at least twice per year. Important items to check during the inspection include:

- Signs of differential settlement,
- Cracking,
- Erosion,
- Leakage in the embankments
- Tree growth on the embankments
- Condition of riprap,
- Sediment accumulation and
- The health of the turf.

At least twice a year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings and accumulated organic matter to prevent an impervious organic mat from forming. Remove trash and debris at the same time. Use deep tilling to break up clogged surfaces, and revegetate immediately.

Remove sediment from the basin as necessary, but wait until the floor of the basin is thoroughly dry. Use light equipment to remove the top layer so as to not compact the underlying soil. Deeply till the remaining soil, and revegetate as soon as possible. Inspect and clean pretreatment devices associated with basins at least twice a year, and ideally every other month.

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Sediment Forebays



Description: A sediment forebay is a post-construction practice consisting of an excavated pit, bermed area, or cast structure combined with a weir, designed to slow incoming stormwater runoff and facilitating the gravity separation of suspended solids. This practice is different from a sediment trap used as a construction period BMP.

Ability to meet specific standards

Standard	Description
2 - Peak Flow	Provides no peak flow attenuation
3 - Recharge	Provides no groundwater recharge
4 - TSS Removal	MassDEP requires a sediment forebay as pretreatment before stormwater is discharged to an extended dry detention basin, wet basin, constructed stormwater wetland or infiltration basin. No separate credit is given for the sediment forebay. For example, extended dry detention basins with sediment forebays receive a credit for 50% TSS removal. Wet basins and constructed stormwater wetlands with sediment forebays receive a credit for 80% TSS removal. When they provide pretreatment for other BMPs, sediment forebays receive a 25% TSS removal credit.
5 - Higher Pollutant Loading	Recommended as a pretreatment BMP
6 - Discharges near or to Critical Areas	Recommended as a pretreatment BMP
7 - Redevelopment	Usually not suitable due to land use constraints

Advantages/Benefits:

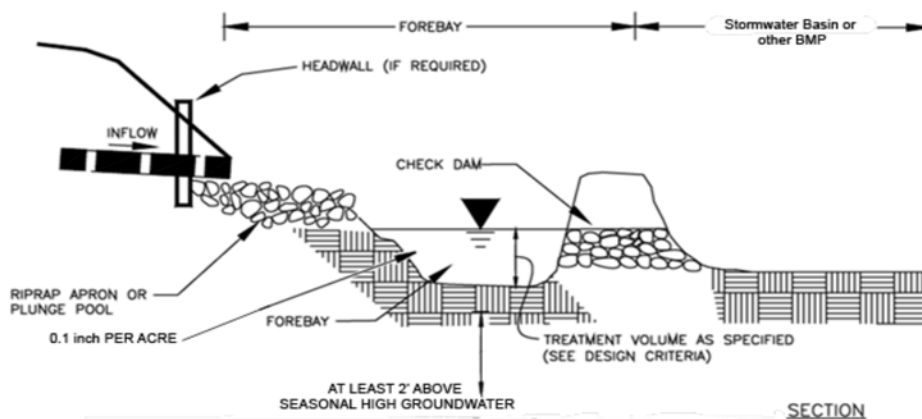
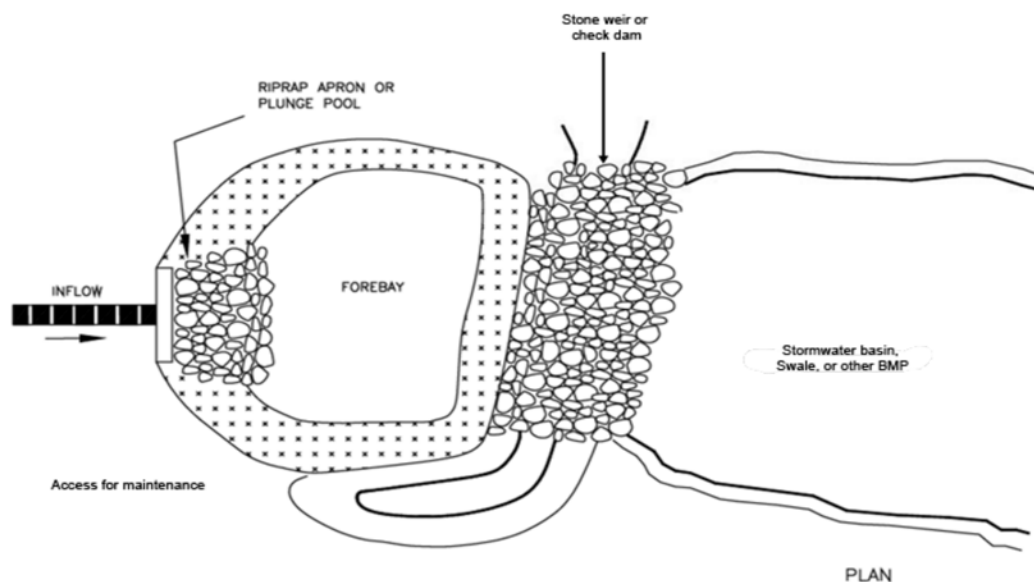
- Provides pretreatment of runoff before delivery to other BMPs.
- Slows velocities of incoming stormwater
- Easily accessed for sediment removal
- Longevity is high with proper maintenance
- Relatively inexpensive compared to other BMPs
- Greater detention time than proprietary separators

Disadvantages/Limitations:

- Removes only coarse sediment fractions
- No removal of soluble pollutants
- Provides no recharge to groundwater
- No control of the volume of runoff
- Frequent maintenance is essential

Pollutant Removal Efficiencies

- Total Suspended Solids (TSS) - 25%
- Nutrients (Nitrogen, phosphorus) - Insufficient data
- Metals (copper, lead, zinc, cadmium) - Insufficient data
- Pathogens (coliform, e coli) - Insufficient data



adapted from the Vermont Stormwater Handbook

Maintenance

Activity	Frequency
Inspect sediment forebays	Monthly
Clean sediment forebays	Four times per year and when sediment depth is between 3 to 6 feet.

Special Features

MassDEP requires a sediment forebay as pretreatment before discharging to a dry extended detention basin, wet basin, constructed stormwater wetland, or infiltration basin.

MassDEP uses the term sediment forebay for BMPs used to pretreat stormwater after construction is complete and the site is stabilized. MassDEP uses the term sediment trap to refer to BMPs used for erosion and sedimentation control during construction. For information on the design and construction of sediment traps used during construction, consult the Massachusetts Erosion and Sediment Control Guidelines for Urban and Suburban Areas: A Guide for Planners, Designers and Municipal Officials.

Sediment Forebays

Design

Sediment forebays are typically on-line units, designed to slow stormwater runoff and settle out sediment.

At a minimum, size the volume of the sediment forebay to hold 0.1-inch/impervious acre to pretreat the water quality volume.

When routing the 2-year and 10-year storms through the sediment forebay, design the forebay to withstand anticipated velocities without scouring.

A typical forebay is excavated below grade with earthen sides and a stone check dam.

Design elevated embankments to meet applicable safety standards.

Stabilize earth slopes and bottoms using grass seed mixes recommended by the NRCS and capable of resisting the anticipated shearing forces associated with velocities to be routed through the forebay. Use only grasses. Using other vegetation will reduce the storage volume in the forebay. Make sure that the selected grasses are able to withstand periodic inundation under water, and drought-tolerant during the summer. MassDEP recommends using a mix of grasses rather than relying upon a single grass species.

Alternatively, the bottom floor may be stabilized with concrete or stone to aid maintenance. Concrete floors or pads, or any hard bottom floor, greatly facilitate the removal of accumulated sediment.

When the bottom floor is vegetated, it may be necessary to remove accumulated sediment by hand, along with re-seeding or re-sodding grasses removed during maintenance.

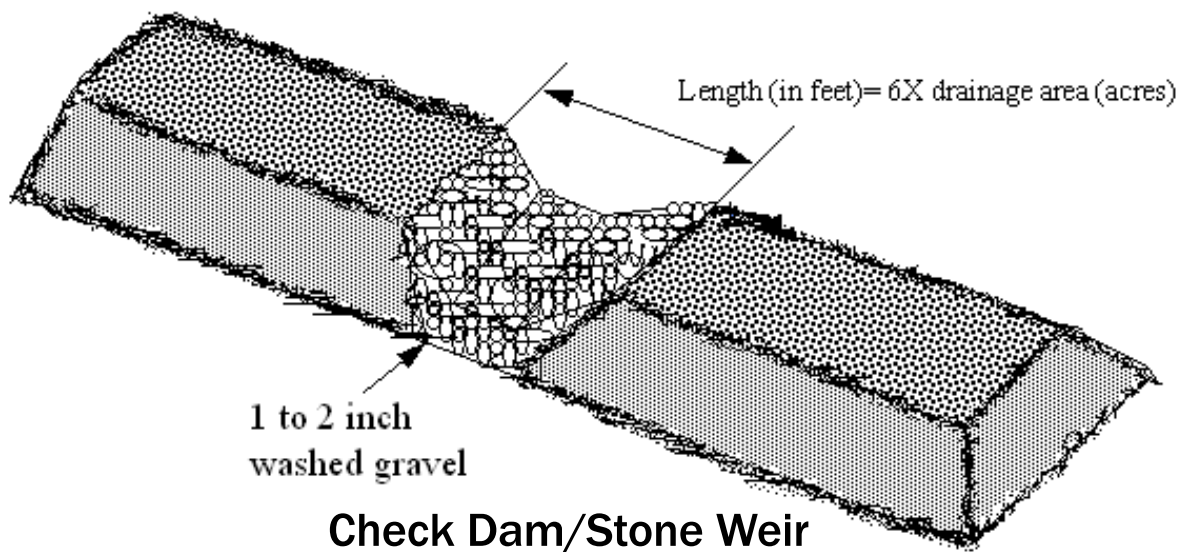
Design sediment forebays to make maintenance accessible and easy. If machinery is required to remove the sediment, carefully incorporate equipment access in the design. Sediment forebays may require excavation so concrete flooring may not always be appropriate.

Include sediment depth markers to simplify inspections. Sediment markers make it easy to determine when the sediment depth is between 3 and 6 feet and needs to be removed. Make the side slopes of sediment forebays no steeper than 3:1. Design the sediment forebay so that the discharge or outflow velocity can control the 2-year peak discharge without scour. Design the channel geometry to prevent erosion from the 2-year peak discharge.

Do not confuse post-construction sediment forebays with the sediment traps used as a construction-period control. Construction-period sediment control traps are sized larger than forebays, because there is a greater amount of suspended solids in construction period runoff. Construction-period sediment traps are sized based on drainage area and not impervious acre. Never use a construction-period sediment trap for post-construction drainage purposes unless it is first brought off-line, thoroughly cleaned (including check dam), and stabilized before being made re-operational.

Refer to the section of this chapter for information on the design of the check dam component of the sediment forebay. Set the minimum elevation of the check dam to hold a volume of 0.1-inch of runoff/impervious acre. Check dam elevations may be uniform or they may contain a weir (e.g., when the top of the check dam is set to the 2-year or 10-year storm, and the bottom of the weir is set to the top of the 0.1-inch/impervious acre volume). When a weir is included in a stone berm, make sure that the weir is able to hold its shape. Fabric or wire may be required.

Unless part of a wet basin, post construction sediment forebays must be designed to dewater between storms. Set the bottom of the forebay at a minimum of 2 feet above seasonal high groundwater, and place pervious material on the bottom floor to facilitate dewatering between storms. For design purposes, use 72 hours to evaluate dewatering, using the storm that produces either the ½ inch or 1-inch of runoff (water quality volume) in a 24-hour period. A stone check dam can act as a filter berm, allowing water to percolate through the check dam. Depending on the head differential, a stone check dam may allow greater dewatering than an earthen berm.



MassDEP Stormwater Handbook, 1996

Maintenance

Sediments and associated pollutants are removed only when sediment forebays are actually cleaned out, so regular maintenance is essential. Frequently removing accumulated sediments will make it less likely that sediments will be resuspended. At a minimum, inspect sediment forebays monthly and clean them out at least four times per year. Stabilize the floor and sidewalls of the sediment forebay before making it operational, otherwise the practice will discharge excess amounts of suspended

sediments. When mowing grasses, keep the grass height no greater than 6 inches. Set mower blades no lower than 3 to 4 inches. Check for signs of rilling and gullyng and repair as needed. After removing the sediment, replace any vegetation damaged during the clean-out by either reseeding or re-sodding. When reseeding, incorporate practices such as hydroseeding with a tackifier, blanket, or similar practice to ensure that no scour occurs in the forebay, while the seeds germinate and develop roots.

Subsurface Structures



Description: Subsurface structures are underground systems that capture runoff, and gradually infiltrate it into the groundwater through rock and gravel. There are a number of underground infiltration systems that can be installed to enhance groundwater recharge. The most common types include pre-cast concrete or plastic pits, chambers (manufactured pipes), perforated pipes, and galleys.

Ability to meet specific standards

Standard	Description
2 - Peak Flow	N/A
3 - Recharge	Provides groundwater recharge
4 - TSS Removal	80%
5 - Higher Pollutant Loading	May be used if 44% of TSS is removed with a pretreatment BMP prior to infiltration. Land uses with the potential to generate runoff with high concentrations of oil and grease require an oil grit separator or equivalent prior to discharge to the infiltration structure. Infiltration must be done in accordance with 314 CMR 5.00.
6 - Discharges near or to Critical Areas	Highly recommended
7 - Redevelopment	Suitable with pretreatment

Advantages/Benefits:

- Provides groundwater recharge
- Reduces downstream flooding
- Preserves the natural water balance of the site
- Can remove other pollutants besides TSS
- Can be installed on properties with limited space
- Useful in stormwater retrofit applications

Disadvantages/Limitations:

- Limited data on field performance
- Susceptible to clogging by sediment
- Potential for mosquito breeding due to standing water if system fails

Pollutant Removal Efficiencies

- | | |
|--|-------------------|
| • Total Suspended Solids (TSS) | 80% |
| • Nutrients (Nitrogen, phosphorus) | Insufficient data |
| • Metals (copper, lead, zinc, cadmium) | Insufficient data |
| • Pathogens (coliform, e coli) | Insufficient data |

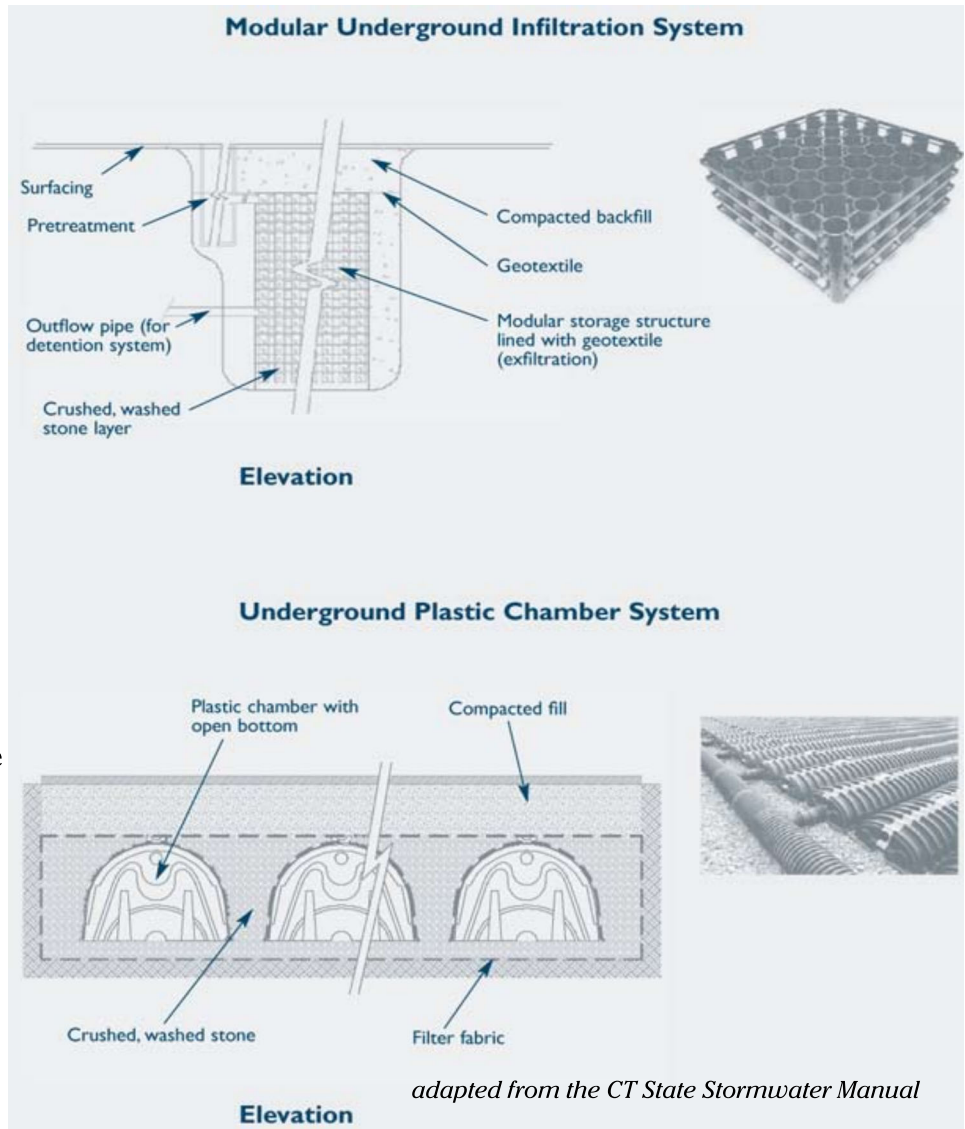
Subsurface Structures

There are different types of subsurface structures:

Infiltration Pit: A pre-cast concrete or plastic barrel with uniform perforations. The bottom of the pit should be closed with the lowest row of perforations at least 6 inches above the bottom, to serve as a sump. Infiltration pits typically include an observation well. The pits may be placed linearly, so that as the infiltrative surfaces in the first pit clog, the overflow moves to the second pit for exfiltration. Place an outlet near the top of the infiltration pit to accommodate emergency overflows. MassDEP provides recharge credit for storage below the emergency outflow invert. To make an infiltration pit, excavate the pit, wrap fabric around the barrel, place stone in the bottom of the pit, place the barrel in the pit, and then backfill stone around the barrel. Take a boring or dig an observation trench at the site of each proposed pit.

Chambers: These are typically manufactured pipes containing open bottoms and sometimes perforations. The chambers are placed atop a stone bed. Take the same number of borings or observation pits as for infiltration trenches. Do not confuse these systems with underground detention systems (UDS) that use similar chambers. UDS are designed to attenuate peak rates of runoff--not to recharge groundwater.

Perforated Pipes: In this system, pipes containing perforations are placed in a leaching bed, similar to a Title 5 soil absorption system (SAS). The pipes dose the leaching bed. Take the same number of borings or observation pits as for infiltration trenches. Perforated pipes by themselves do not constitute a stormwater recharge system and receive no credit pursuant to Stormwater Standard No. 3. Do not confuse recharge systems that use perforated pipes with perforated pipes installed to lower the water table or divert groundwater flows.



Galleys: Similar to infiltration pits. Some designs consist of concrete perforated rectangular vaults. Others are modular systems usually placed under parking lots. When the galley design consists of a single rectangular perforated vault, conduct one boring or observation trench per galley. When the galleys consist of interlocking modular units, take the same number of borings or observation pits as for infiltration trenches. Do not confuse these galleys with vaults storing water for purposes of underground detention, which do not contain perforations.

Applicability

Subsurface structures are constructed to store stormwater temporarily and let it percolate into the underlying soil. These structures are used for small drainage areas (typically less than 2 acres). They are feasible only where the soil is adequately permeable and the maximum water table and/or bedrock

elevation is sufficiently low. They can be used to control the quantity as well as quality of stormwater runoff, if properly designed and constructed. The structures serve as storage chambers for captured stormwater, while the soil matrix provides treatment.

Without adequate pretreatment, subsurface structures are not suitable for stormwater runoff from land uses or activities with the potential for high sediment or pollutant loads. Structural pretreatment BMPs for these systems include, but are not limited to, deep sump catch basins, proprietary separators, and oil/grit separators. They are suitable alternatives to traditional infiltration trenches and basins for space-limited sites. These systems can be installed beneath parking lots and other developed areas provided the systems can be accessed for routine maintenance.

Subsurface systems are highly prone to clogging. Pretreatment is always required unless the runoff is strictly from residential rooftops.

Effectiveness

Performance of subsurface systems varies by manufacturer and system design. Although there are limited field performance data, pollutant removal efficiency is expected to be similar to those of infiltration trenches and basins (i.e., up to 80% of TSS removal). MassDEP awards a TSS removal credit of 80% for systems designed in accordance with the specifications in this handbook.

Planning Considerations

Subsurface structures are excellent groundwater recharge alternatives where space is limited. Because infiltration systems discharge runoff to groundwater, they are inappropriate for use in areas with potentially higher pollutant loads (such as gas stations), unless adequate pretreatment is provided. In that event, oil grit separators, sand filters or equivalent BMPs must be used to remove sediment, floatables and grease prior to discharge to the subsurface structure.

Design

Unlike infiltration basins, widely accepted design standards and procedures for designing subsurface structures are not available. Generally, a subsurface structure is designed to store a “capture volume” of runoff for a specified period of “storage time.” The definition of capture volume differs depending on the

purpose of the subsurface structure and the stormwater management program being used. Subsurface structures should infiltrate good quality runoff only. Pretreatment prior to infiltration is essential. The composition, configuration and layout of subsurface structures varies considerably depending on the manufacturer. Follow the design criteria specified by vendors or system manufacturers. Install subsurface structures in areas that are easily accessible for routine and non-routine maintenance.

As with infiltration trenches and basins, install subsurface structures only in soils having suitable infiltration capacities as determined through field testing. Determine the infiltrative capacity of the underlying native soil through the soil evaluation set forth in Volume 3. Never use a standard septic system percolation test to determine soil permeability because this test tends to greatly overestimate the infiltration capacity of soils.

Subsurface structures are typically designed to function off-line. Place a flow bypass structure upgradient of the infiltration structure to convey high flows around the structure during large storms.

Design the subsurface structure so that it drains within 72 hours after the storm event and completely dewater between storms. Use a minimum draining time of 6 hours to ensure adequate pollutant removal. Design all ports to be mosquito-proof, i.e., to inhibit or reduce the number of mosquitoes able to breed within the BMP.

The minimum acceptable field infiltration rate is 0.17 inches per hour. Subsurface structures must be sized in accordance with the procedures set forth in Volume 3. Manufactured structures must also be sized in accordance with the manufacturers’ specifications. Design the system to totally exfiltrate within 72 hours.

Design the subsurface structure for live and dead loads appropriate for their location. Provide measures to dissipate inlet flow velocities and prevent channeling of the stone media. Generally, design the system so that inflow velocities are less than 2 feet per second (fps).

All of these devices must have an appropriate number of observation wells, to monitor the water surface elevation within the well, and to serve as a sampling port.

Each of these different types of structures, with the exception of perforated pipes in leaching fields similar to Title 5 systems, must have entry ports to allow worker access for maintenance, in accordance with OSHA requirements.

Adapted from:
Connecticut Department of Environmental Conservation.
Connecticut Stormwater Quality Manual. 2004.
MassHighway. Storm Water Handbook for Highways and Bridges. May 2004.

Construction

Stabilize the site prior to installing the subsurface structure. Do not allow runoff from any disturbed areas on the site to flow to the structure. Rope off the area where the subsurface structures are to be placed. Accomplish any required excavation with equipment placed just outside of this area. If the size of the area intended for exfiltration is too large to accommodate this approach, use trucks with low-pressure tires to minimize compaction. Do not allow any other vehicles within the area to be excavated. Keep the area above and immediately surrounding the subsurface structure roped off to all construction vehicles until the final top surface is installed (either paving or landscaping). This prevents additional compaction. When installing the final top surface, work from the edges to minimize compaction of the underlying soils.

Before installing the top surface, implement erosion and sediment controls to prevent sheet flow or wind blown sediment from entering the leach field. This includes, but is not limited to, minimizing land disturbances at any one time, placing stockpiles away from the area intended for infiltration, stabilizing any stockpiles through use of vegetation or tarps, and placing sediment fences around the perimeter of the infiltration field.

Provide an access port, man-way, and observation well to enable inspection of water levels within the system. Make the observation well pipe visible at grade (i.e., not buried).

Maintenance

Because subsurface structures are installed underground, they are extremely difficult to maintain. Inspect inlets at least twice a year. Remove any debris that might clog the system. Include mosquito controls in the Operation and Maintenance Plan.



United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for Plymouth County, Massachusetts



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Contents

Preface	2
How Soil Surveys Are Made	5
Soil Map	8
Soil Map.....	9
Legend.....	10
Map Unit Legend.....	11
Map Unit Descriptions.....	11
Plymouth County, Massachusetts.....	13
37A—Massasoit - Mashpee complex, 0 to 3 percent slopes.....	13
253B—Hinckley loamy sand, 3 to 8 percent slopes.....	15
316B—Scituate gravelly sandy loam, 3 to 8 percent slopes, very stony.....	17
656B—Udorthents - Urban land complex, 0 to 8 percent slopes.....	18
References	21

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

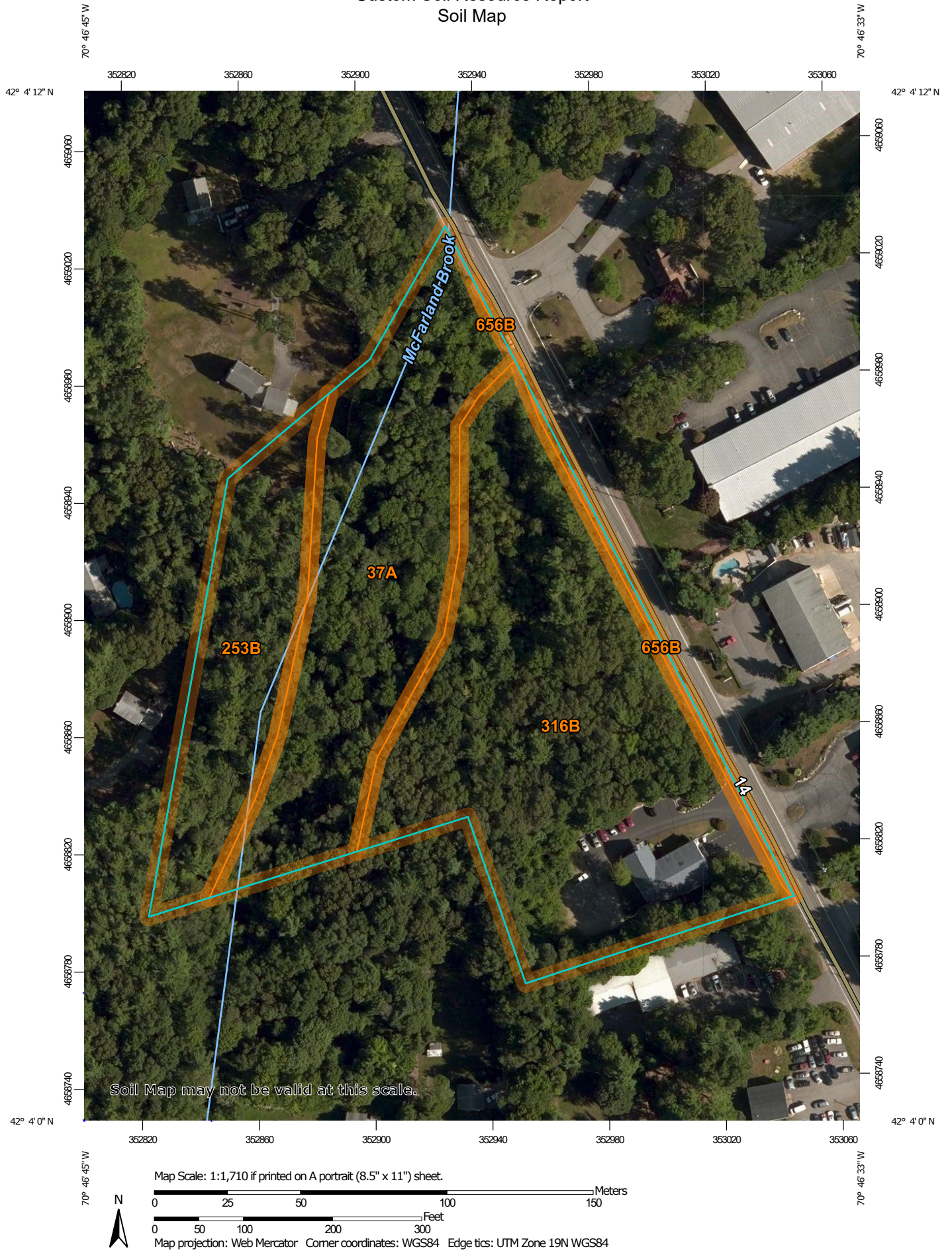
Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map


The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map




MAP LEGEND


Area of Interest (AOI)

 Area of Interest (AOI)


Soils


 Soil Map Unit Polygons


 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit


 Clay Spot


 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot


 Sinkhole

 Slide or Slip

 Sodic Spot


 Spoil Area

 Stony Spot


 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals


Transportation

 Rails


 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Plymouth County, Massachusetts
Survey Area Data: Version 12, Sep 12, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 26, 2014—Sep 4, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
37A	Massasoit - Mashpee complex, 0 to 3 percent slopes	2.3	31.7%
253B	Hinckley loamy sand, 3 to 8 percent slopes	1.2	16.8%
316B	Scituate gravelly sandy loam, 3 to 8 percent slopes, very stony	3.6	50.2%
656B	Udorthents - Urban land complex, 0 to 8 percent slopes	0.1	1.3%
Totals for Area of Interest		7.2	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Plymouth County, Massachusetts

37A—Massasoit - Mashpee complex, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: bd1q
Elevation: 0 to 400 feet
Mean annual precipitation: 41 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Massasoit and similar soils: 55 percent
Mashpee and similar soils: 35 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Massasoit

Setting

Landform: Terraces, depressions, drainageways
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Sandy and gravelly glaciofluvial deposits

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material
Oa - 1 to 3 inches: highly decomposed plant material
A - 3 to 5 inches: fine sand
Eg1 - 5 to 11 inches: fine sand
Eg2 - 11 to 13 inches: fine sand
Bhs - 13 to 17 inches: fine sand
Bsm - 17 to 23 inches: fine sand
Bs - 23 to 26 inches: fine sand
BC - 26 to 43 inches: fine sand
Cg - 43 to 80 inches: loamy very fine sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: 7 to 20 inches to ortstein
Natural drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.01 in/hr)
Depth to water table: About 0 to 12 inches
Frequency of flooding: None
Frequency of ponding: Occasional
Available water storage in profile: Very low (about 1.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w

Custom Soil Resource Report

Hydrologic Soil Group: D

Hydric soil rating: Yes

Description of Mashpee

Setting

Landform: Depressions, drainageways, terraces

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Tread

Down-slope shape: Concave

Across-slope shape: Concave

Parent material: Sandy and gravelly glaciofluvial deposits

Typical profile

Oe1 - 0 to 2 inches: moderately decomposed plant material

Oe2 - 2 to 4 inches: moderately decomposed plant material

Oa - 4 to 5 inches: highly decomposed plant material

AE - 5 to 7 inches: loamy fine sand

Eg - 7 to 11 inches: fine sand

Bh1 - 11 to 13 inches: fine sand

Bh2 - 13 to 17 inches: fine sand

Bs - 17 to 24 inches: loamy fine sand

C1 - 24 to 39 inches: fine sand

C2 - 39 to 65 inches: fine sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Poorly drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.42 to 5.95 in/hr)

Depth to water table: About 0 to 12 inches

Frequency of flooding: None

Frequency of ponding: Occasional

Available water storage in profile: Low (about 4.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: A/D

Hydric soil rating: Yes

Minor Components

Deerfield

Percent of map unit: 5 percent

Landform: Outwash plains, terraces, deltas

Landform position (two-dimensional): Footslope, summit

Landform position (three-dimensional): Tread

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: No

Rainberry

Percent of map unit: 3 percent

Landform: Depressions, kettles

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Tread

Down-slope shape: Concave

Across-slope shape: Linear

Hydric soil rating: Yes

Squamscott

Percent of map unit: 2 percent

Landform: Lake terraces, lake plains

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Talf

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

253B—Hinckley loamy sand, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2svm8

Elevation: 0 to 1,430 feet

Mean annual precipitation: 36 to 53 inches

Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 250 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Hinckley and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hinckley

Setting

Landform: Outwash terraces, outwash plains, moraines, kame terraces, outwash deltas, kames, eskers

Landform position (two-dimensional): Summit, backslope, footslope, shoulder

Landform position (three-dimensional): Nose slope, side slope, base slope, crest, riser, tread

Down-slope shape: Linear, convex, concave

Across-slope shape: Convex, linear, concave

Parent material: Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material

A - 1 to 8 inches: loamy sand

Bw1 - 8 to 11 inches: gravelly loamy sand

Bw2 - 11 to 16 inches: gravelly loamy sand

BC - 16 to 19 inches: very gravelly loamy sand

Custom Soil Resource Report

C - 19 to 65 inches: very gravelly sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water storage in profile: Very low (about 3.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3s
Hydrologic Soil Group: A
Hydric soil rating: No

Minor Components

Windsor

Percent of map unit: 8 percent
Landform: Kames, eskers, outwash terraces, kame terraces, outwash plains, moraines, outwash deltas
Landform position (two-dimensional): Summit, shoulder, backslope, footslope
Landform position (three-dimensional): Nose slope, side slope, base slope, crest, riser, tread
Down-slope shape: Linear, convex, concave
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

Sudbury

Percent of map unit: 5 percent
Landform: Kame terraces, outwash plains, moraines, outwash deltas, outwash terraces
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Side slope, base slope, head slope, tread
Down-slope shape: Concave, linear
Across-slope shape: Linear, concave
Hydric soil rating: No

Agawam

Percent of map unit: 2 percent
Landform: Kame terraces, outwash plains, moraines, outwash deltas, kames, eskers, outwash terraces
Landform position (two-dimensional): Summit, shoulder, backslope, footslope
Landform position (three-dimensional): Nose slope, side slope, base slope, crest, tread, riser
Down-slope shape: Linear, convex, concave
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

316B—Scituate gravelly sandy loam, 3 to 8 percent slopes, very stony

Map Unit Setting

National map unit symbol: bczw

Elevation: 10 to 400 feet

Mean annual precipitation: 41 to 54 inches

Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Scituate, very stony, and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scituate, Very Stony

Setting

Landform: Drumlins, ridges

Landform position (two-dimensional): Footslope, shoulder

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave

Across-slope shape: Concave

Parent material: Coarse-loamy eolian deposits over sandy lodgment till

Typical profile

Ap - 0 to 11 inches: gravelly sandy loam

Bw1 - 11 to 15 inches: gravelly sandy loam

Bw2 - 15 to 20 inches: sandy loam

BC1 - 20 to 25 inches: gravelly sandy loam

BC2 - 25 to 35 inches: sandy loam

Cd1 - 35 to 46 inches: loamy coarse sand

Cd2 - 46 to 60 inches: loamy coarse sand

Properties and qualities

Slope: 3 to 8 percent

Percent of area covered with surface fragments: 1.5 percent

Depth to restrictive feature: 20 to 35 inches to densic material

Natural drainage class: Moderately well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.20 in/hr)

Depth to water table: About 15 to 20 inches

Frequency of flooding: None

Frequency of ponding: None

Available water storage in profile: Low (about 3.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Custom Soil Resource Report

Hydrologic Soil Group: C/D

Hydric soil rating: No

Minor Components

Birchwood, very stony

Percent of map unit: 5 percent

Landform: Ground moraines, till plains, drumlins

Landform position (two-dimensional): Summit, footslope

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: No

Norwell, extremely stony

Percent of map unit: 5 percent

Landform: Depressions, drainageways

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Base slope

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

Woodbridge, very stony

Percent of map unit: 5 percent

Landform: Drumlins, till plains, hills

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Interfluve

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: No

Montauk, very stony

Percent of map unit: 5 percent

Landform: Till plains, drumlins, ground moraines

Landform position (two-dimensional): Shoulder, summit

Landform position (three-dimensional): Interfluve

Down-slope shape: Convex

Across-slope shape: Convex

Hydric soil rating: No

656B—Udorthents - Urban land complex, 0 to 8 percent slopes

Map Unit Setting

National map unit symbol: bd08

Elevation: 0 to 390 feet

Mean annual precipitation: 41 to 54 inches

Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Udorthents, loamy, and similar soils: 45 percent

Urban land: 40 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents, Loamy

Setting

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Tread

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Coarse-loamy human transported material

Typical profile

^A - 0 to 5 inches: loam

^C1 - 5 to 21 inches: gravelly loam

^C2 - 21 to 80 inches: gravelly sandy loam

Properties and qualities

Slope: 0 to 8 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to very high (0.01 to 14.17 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water storage in profile: Moderate (about 7.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2s

Hydrologic Soil Group: B

Hydric soil rating: No

Minor Components

Udipsamments, wet substratum

Percent of map unit: 5 percent

Landform: Dikes

Landform position (two-dimensional): Footslope

Landform position (three-dimensional): Tread

Down-slope shape: Linear, convex

Across-slope shape: Linear

Hydric soil rating: No

Udorthents, wet substratum

Percent of map unit: 5 percent

Landform position (two-dimensional): Footslope

Landform position (three-dimensional): Tread

Down-slope shape: Linear

Across-slope shape: Linear

Hydric soil rating: No

Udipsamments

Percent of map unit: 5 percent

Landform: Dikes

Landform position (two-dimensional): Summit

Landform position (three-dimensional): Tread

Down-slope shape: Linear, convex

Across-slope shape: Linear

Hydric soil rating: No

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Commonwealth of Massachusetts

Pembroke

, Massachusetts

Soil Suitability Assessment for On-site Sewage Disposal

Performed by: Kevin Grady
 GRADY CONSULTING, L.L.C.
 6 Grays Beach Road
 Kingston, MA 02364
 Phone: (781) 585-2300 Fax: (781) 585-2378

Date: 8/14/07Witnessed by: Fred Leary

Location Address or Lot #

Washington StreetAssessor's Lot F9-24

*Owner's Name

Edward J. Burke

*Address &

*Telephone #

25 Eustis StreetMarshfield MA 02050New Construction X Repair _____ Title V Inspection _____781 834 5660Office ReviewPublished Soil Survey Available: No X Yes _____

Year Published: _____ Publication Scale: _____ Soil Map Unit: _____

Drainage Class: _____ Soil Limitations: _____

Surficial Geology Report Available: No X Yes _____

Year Published: _____ Publication Scale: _____

Geologic Material (Map Unit): _____

Landform: _____

Flood Insurance Rate Map:Above 500 year flood boundary: No _____ Yes XWithin 500 year flood boundary: No X Yes _____Within 100 year flood boundary: No X Yes _____Wetland Area:National Wetland Inventory Map (map unit): N/A

Wetlands Conservancy Program Map (map unit): _____

Current Water Resource Conditions (USGS):Month: July

Range: Above Normal _____

Normal X

Below Normal _____

Other References Reviewed: _____Depth of Naturally Occurring Pervious Material

Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system? Yes

If not, what is the depth of naturally occurring pervious material?

Certification

I certify that on April 6, 1999 (date) I have passed the soil evaluator examination approved by the Department of Environmental Protection and that the above analysis was performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017.

Signature: Ki GradyDate: 8/14/07

TITLE 5 ON-SITE REVIEW

Deep Hole # 1 Date 8/14/07 Time 9:30 Weather SUNNY 75°
 Location(Identify on Site Plan) _____
 Land Use Commercial Slope(%) 0-2 Surface Stones none
 Vegetation woods Landform _____

Distances from: Open Water Body — ft. Possible Wet Area 150 ft. Drinking Water Well — ft.

Drainageway — ft. Propertyline 50 ft. Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
0"-6"	A	Loam	10YR 3/3		
6"-22"	B	Sandy Loam	10YR 6/6		Friable
22"-48"	C ₁	Gravelly Sand	2.5Y 6/4		20-30% gravel
48"-96"	C ₂	Gravelly Sandy Loam	2.5Y 6/4		20% gravel Tight Remove
96"-152"	C ₃	Med Sand	2.5Y 5/3	none	12% gravel loose

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: none Weeping from Pit Face none
 Estimated Seasonal High Groundwater 12'-0" assumed no water encountered

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

____ Depth observed standing in observation hole: _____ inches ____ Depth to soil mottles: _____ inches
 ____ Depth to weeping from side of observation hole: _____ inches ____ Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj.factor _____ Adj.Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole #	_____	Time at 9"	<u>9:59</u>
Depth of Perc	<u>96-114</u>	Time at 6"	<u>10:03</u>
Start Presoak	<u>9:40</u>	Time (9"-6")	<u>4 min</u>
End Presoak	<u>9:55</u>	Rate Min/Inch	<u>12 min/in</u>

Site Suitability Assessment: Site Passed X Site Failed _____ Additional Testing Needed: _____

Performed By Kevin Grady Certification # _____

Witnessed By Fred Leary

Comments:

TITLE 5 ON-SITE REVIEW

Deep Hole # 2 Date 8/14/07 Time 9:30 Weather Sunny 79°
 Location(Identify on Site Plan) _____
 Land Use Commercial Slope(%) 0-2 Surface Stones none
 Vegetation woods Landform _____

Distances from: Open Water Body — ft. Possible Wet Area 150 ft. Drinking Water Well — ft.
 Drainageway — ft. Propertyline 30 ft Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
0"-6"	A	Loam	10YR3/3		
6"-22"	B	Sand-Loam	10YR6/6		Frable
22"-48"	C	Gravelly Sand	2.5Y6/4		Frable 20-30% gravel
48"-96"	C2	Gravelly Sand-Loam	2.5Y6/4		20% gravel Tight Remove
96"-180"	C3	Med. Sand	2.5Y5/3	none	Loose

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: none Weeping from Pit Face none
 Estimated Seasonal High Groundwater 15'-0" assumed no water encountered

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

Depth observed standing in observation hole: _____ inches Depth to soil mottles: _____ inches
 Depth to weeping from side of observation hole: _____ inches Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj.factor _____ Adj.Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole # _____ Time at 9" 10:43
 Depth of Perc 108"-126" Time at 6" 10:47
 Start Presoak 10:25 Time (9"-6") 4min
 End Presoak 10:40 Rate Min/Inch <2min/in

Site Suitability Assessment: Site Passed X Site Failed _____ Additional Testing Needed: _____

Performed By Kevin Girard Certification # _____

Witnessed By Fred Leary

Comments:

TITLE 5 ON-SITE REVIEW

Deep Hole # 3 Date 8/14/07 Time 9:30 Weather Sunny 75°
 Location (Identify on Site Plan) _____
 Land Use Commercial Slope(%) 2-4 Surface Stones none
 Vegetation Woods Landform _____

Distances from: Open Water Body — ft. Possible Wet Area 120 ft. Drinking Water Well — ft.
 Drainageway — ft. Propertyline 50 ft. Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, % Gravel
0"-6"	A	Loam	10YR ³ /3		
6"-18"	B	sandy loam	10YR ⁴ /6		Friable
18"-60"	C ₁	gravelly sand	2.5Y ⁶ /4		20%-30% gravel
60"-108"	C ₂	gravelly sandy loam	2.5Y ⁶ /4		20% gravel Tight Remove
108" +		Large Rocks	2.5Y ⁵ /3	none	<2% gravel Loose

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: _____ Weeping from Pit Face _____
 Estimated Seasonal High Groundwater 9'-0" assumed no water encountered

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

____ Depth observed standing in observation hole: _____ inches ____ Depth to soil mottles: _____ inches
 ____ Depth to weeping from side of observation hole: _____ inches ____ Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj. factor _____ Adj. Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole # _____ Time at 9" _____
 Depth of Perc _____ Time at 6" _____
 Start Presoak _____ Time (9"-6") _____
 End Presoak _____ Rate Min/Inch _____

Site Suitability Assessment: Site Passed _____ Site Failed _____ Additional Testing Needed: _____

Performed By Kevin Grady Certification # _____

Witnessed By Fred Leary

Comments:

TITLE 5 ON-SITE REVIEW

Deep Hole # 4 Date 8/14/07 Time 9:30 Weather Sunny 75°
 Location(Identify on Site Plan) _____
 Land Use Commercial Slope(%) 2-5 Surface Stones none
 Vegetation Woods Landform _____

Distances from: Open Water Body 150 ft. Possible Wet Area 100 ft. Drinking Water Well — ft.

Drainageway — ft. Propertyline 100 ft. Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
0-8	A	Loam	10YR 7/3		
8-24	B	Sandy Loam	10YR 6/6		Frable
24-60	C1	Gravelly Sand	2.5Y 6/4		20-30% gravel
60-90	C2	Gravelly Sandy Loam	2.5Y 6/4		20% gravel Tight Remove
90-108	C3	Med Sand	2.5Y 5/3	none	<2% gravel Loose

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: _____ Weeping from Pit Face _____
 Estimated Seasonal High Groundwater 9'-0" assumed no water encountered

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

_____ Depth observed standing in observation hole: _____ inches _____ Depth to soil mottles: _____ inches
 _____ Depth to weeping from side of observation hole: _____ inches _____ Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj. factor _____ Adj. Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole # _____ Time at 9" _____
 Depth of Perc _____ Time at 6" _____
 Start Presoak _____ Time (9"-6") _____
 End Presoak _____ Rate Min/Inch _____

Site Suitability Assessment: Site Passed _____ Site Failed _____ Additional Testing Needed: _____

Performed By _____ Certification # _____

Witnessed By _____

Comments:



GRADY CONSULTING, L.L.C.

CIVIL ENGINEERS AND LAND SURVEYORS
6 GRAYS BEACH ROAD, KINGSTON, MA 02364
TEL. (781) 585-2300 FAX (781) 585-2378

JOB _____
SHEET _____ OF _____ SCALE: _____
CALCULATED BY: _____ DATE: _____
CHECKED BY: _____ DATE: _____

MOST PROBABLE HIGH WATER TABLE COMPUTATION

FOR: Washington Street, Pembroke

REFERENCE: USGS Report 80-1205

BASE DATA: Depth to Water 9'

SOIL TYPE: sand/valley **SIMILAR TO:** Duxbury(D4W)#79

FORMULA: $S_n = S_c - S_r / O_{wr} * (O_{wc} - O_{wmax})$ From Page 18 of Reference

$S_c =$	9.00	From Original Plan (Depth to Water)
$S_r =$	4.20	From Fig. 12 (Ref.) vally flat (using 10%)
$O_{wr} =$	4.04	Highest range of water level within last 12 months (Jan 1998 Summary)
O_{wc}	9.03	From U.S.G.S. Web Site August 14, 2007
$O_{wmax} =$	6.10	From Historical Data Duxbury (D4W)#79

Amount of Adjustment

$Sh = 9.0 - (4.2/4.04) * (9.03 - 6.1)$

$Sh = 9.0 - 3.05$ **5.95' below surface**

Commonwealth of Massachusetts
Pembroke, Massachusetts
Soil Suitability Assessment for On-site Sewage Disposal

Performed by:

Kevin Grady

Date: 8/9/19

GRADY CONSULTING, L.L.C.

71 Evergreen Street, Suite 1

Kingston, MA 02364

Phone: (781) 585-2300 Fax: (781) 585-2378

Witnessed by:

Lisa Collety

Location Address or Lot #

737 Washington Street

*Owner's Name

Rose Realty Trust

*Address &

55 Redwood Circle

*Telephone #

Mashpee MA 02649781 826 9511New Construction ☐ Repair ☐ Title V Inspection ☐**Office Review****Published Soil Survey Available:** No ☒ Yes ☐

Year Published: _____ Publication Scale: _____ Soil Map Unit: _____

Drainage Class: _____ Soil Limitations: _____

Surficial Geology Report Available: No ☒ Yes ☐

Year Published: _____ Publication Scale: _____

Geologic Material (Map Unit): _____

Landform: _____

Flood Insurance Rate Map:Above 500 year flood boundary: No ☐ Yes ☒Within 500 year flood boundary: No ☒ Yes ☐Within 100 year flood boundary: No ☒ Yes ☐**Wetland Area:**National Wetland Inventory Map (map unit): N/A

Wetlands Conservancy Program Map (map unit): _____

Current Water Resource Conditions (USGS):Range: Above Normal ☒ Normal ☐ Below Normal ☐Month: August**Other References Reviewed:** _____**Depth of Naturally Occurring Pervious Material**

Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

Yes

If not, what is the depth of naturally occurring pervious material?

Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise, and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated on the attached soil evaluation form, are accurate and in accordance with CMR 15.100 through 15.107.

Signature: AKGDate: 8/9/19

TITLE 5 ON-SITE REVIEW

Deep Hole # 1 Date 8/9/19 Time 10:00 Weather Sunny 80°
 Location (Identify on Site Plan) _____
 Land Use Commercial Slope(%) 0-2 Surface Stones none
 Vegetation Lawn/trees Landform _____

Distances from: Open Water Body — ft. Possible Wet Area — ft. Drinking Water Well — ft.

Drainageway — ft. Propertyline 20 ft Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
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0"-32"	Fill				
32"-36"	A	Loam	3/3		
36"-40"	B	Loamy Sand	10YR 5/6		Fragile
40"-132"	C1	Loamy Sand	2.5Y 6/4	none	Fragile 5% gravel - few cobbles

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: 130" Weeping from Pit Face _____
 Estimated Seasonal High Groundwater 10'-10"

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

☒ Depth observed standing in observation hole: 130 inches Depth to soil mottles: _____ inches
☐ Depth to weeping from side of observation hole: _____ inches Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj. factor _____ Adj. Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole #	_____	Time at 9"	<u>10:00</u>
Depth of Perc	<u>40-58</u>	Time at 6"	<u>10:24</u>
Start Presoak	<u>9:23</u>	Time (9"-6")	<u>8 minutes 24</u>
End Presoak	<u>9:39</u>	Rate Min/Inch	<u>8 min/in</u>

Site Suitability Assessment: Site Passed ☒ Site Failed _____ Additional Testing Needed: _____

Performed By Kevin Greedy Certification # _____

Witnessed By Lisa Collier

Comments:

TITLE 5 ON-SITE REVIEW

Deep Hole # 2 Date 8/1/19 Time 10:15 Weather Sunny 80°
 Location (identify on Site Plan) _____
 Land Use Commercial Slope(%) 6-2 Surface Stones none
 Vegetation Lawn/Woods Landform _____

Distances from: Open Water Body _____ ft. Possible Wet Area _____ ft. Drinking Water Well _____ ft.

Drainageway _____ ft. Propertyline 30 ft Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
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0-24"	Fill				
24-30	A	Loam	10YR 3/3		
30-84	C ₁	Med Sand	2.5Y 6/4		0% gravel loose
84-120	C ₂	Med Sand	2.5Y 6/2	none	5% gravel loose

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: none Weeping from Pit Face none
 Estimated Seasonal High Groundwater 10'-0" assumed no water encountered

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

____ Depth observed standing in observation hole: _____ inches ____ Depth to soil mottles: _____ inches
 ____ Depth to weeping from side of observation hole: _____ inches ____ Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj. factor _____ Adj. Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole #	_____	Time at 9"	<u>10:21</u>
Depth of Perc	<u>30-48</u>	Time at 6"	<u>10:25</u>
Start Presoak	<u>10:00</u>	Time (9"-6")	<u>4 min</u>
End Presoak	<u>10:15</u>	Rate Min/Inch	<u>< 2 min/in</u>

Site Suitability Assessment: Site Passed X Site Failed _____ Additional Testing Needed: _____

Performed By Kevin Gault Certification # _____

Witnessed By Lisa Cullity

Comments: _____

TITLE 5 ON-SITE REVIEW

Deep Hole # 3 Date 8/9/19 Time 10:00 Weather sunny 80°
 Location (Identify on Site Plan) _____
 Land Use Commercial Slope(%) 0-2 Surface Stones stonewall
 Vegetation woods Landform _____

Distances from: Open Water Body _____ ft. Possible Wet Area _____ ft. Drinking Water Well _____ ft.

Drainageway _____ ft. Propertyline 15 ft Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
0-24	Fill	Fill Loam			
24-30	A	A Loam	10YR 3/3		
30-40	B	B Loamy Sand	10YR 5/6		Fragile
40-72	C	Loamy Sand	2.5Y 4/4	none	few large boulders @ Bottom well drained unable to remove

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: none Weeping from Pit Face none
 Estimated Seasonal High Groundwater 6'-0" assumed no water

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

____ Depth observed standing in observation hole: _____ inches Depth to soil mottles: _____ inches
 ____ Depth to weeping from side of observation hole: _____ inches Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj.factor _____ Adj.Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole # _____ Time at 9" _____
 Depth of Perc _____ Time at 6" _____
 Start Presoak _____ Time (9"-6") _____
 End Presoak _____ Rate Min/Inch _____

Site Suitability Assessment: Site Passed _____ Site Failed _____ Additional Testing Needed:

Performed By _____ Certification # _____

Witnessed By _____

Comments:

TITLE 5 ON-SITE REVIEW

Deep Hole # 4 Date 8/6/19 Time 11:00 Weather Sunny 80
 Location(identify on Site Plan) _____
 Land Use commercial Slope(%) 0.2 Surface Stones stonewall
 Vegetation weeds Landform _____

Distances from: Open Water Body _____ ft. Possible Wet Area _____ ft. Drinking Water Well _____ ft.

Drainageway _____ ft. Propertyline 30 ft Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
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0"-6"	A	Loam	10YR 3/3		
6"-30"	B	Loamy Sand	10YR 5/6		Friable
30"-120"	C	Loamy Sand	2.5Y 4/4	none	5% gravel Friable

Parent Material (geologic) Glacial Till Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: _____ Weeping from Pit Face _____
 Estimated Seasonal High Groundwater 10'-6" assumed no water

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

Depth observed standing in observation hole: _____ inches Depth to soil mottles: _____ inches
 Depth to weeping from side of observation hole: _____ inches Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj.factor _____ Adj.Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole # _____	Time at 9" _____
Depth of Perc _____	Time at 6" _____
Start Presoak _____	Time (9"-6") _____
End Presoak _____	Rate Min/Inch _____

Site Suitability Assessment: Site Passed _____ Site Failed _____ Additional Testing Needed: _____

Performed By _____ Certification # _____

Witnessed By _____

Comments: _____

TITLE 5 ON-SITE REVIEW

Deep Hole # 0-1 Date 4/19/21 Time 10:30 Weather Sunny 60°
 Location (Identify on Site Plan) _____
 Land Use Vacant Slope(%) 0-2 Surface Stones stonewalls
 Vegetation woods Landform _____

Distances from: Open Water Body 200 ft. Possible Wet Area 200 ft. Drinking Water Well — ft.

Drainageway — ft. Propertyline 80 ft. Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
0"-4"	A	Loam	10YR 3/3		Frable
4"-32"	B	Sand-Loam	10YR 5/6		Frable
32"-88"	C	Sand/Loam	2.5-15/4		compact (Remane) Few Boulders
88"-150"	C2	Gravelly Medium Sand	2.5-17/4	none	Frable 10% gravel + 10% boulders

Parent Material (geologic) Outwash Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: none Weeping from Pit Face none
 Estimated Seasonal High Groundwater none

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

Depth observed standing in observation hole: _____ inches Depth to soil mottles: _____ inches
 Depth to weeping from side of observation hole: _____ inches Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj. factor _____ Adj. Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole # _____ Time at 9" _____
 Depth of Perc _____ Time at 6" _____
 Start Presoak _____ Time (9"-6") _____
 End Presoak _____ Rate Min/Inch _____

Site Suitability Assessment: Site Passed _____ Site Failed _____ Additional Testing Needed: _____

Performed By _____ Certification # _____

Witnessed By _____

Comments:

TITLE 5 ON-SITE REVIEW

Deep Hole # 0-2 Date 4/19/21 Time 11:30 Weather sunny 60°
 Location(Identify on Site Plan) _____
 Land Use Veg Slope(%) 2-5 Surface Stones few
 Vegetation woods Landform _____

Distances from: Open Water Body 150 ft. Possible Wet Area 50 ft. Drinking Water Well - ft.

Drainageway - ft. Propertyline 20 ft Other _____

DEEP OBSERVATION HOLE LOG

Depth From Surface (Inches)	Soil Horizon (USDA)	Soil Texture (Munsell)	Soil Color	Soil Mottling	Other: Structures, Stones, Boulders, Consistency, %Gravel
0"-6"	A	Loam	10YR 3/3		Frangible
6"-28"	B	Sand/Loam	10YR 5/6		Frangible
28"-90"	C ₁	Medium Gravelly Sand	2.5Y 5/4	42"	Frangible Loose 10-15% gravel
90"-108"	C ₂	Med-Loose Sand	2.5Y 7/4		Loose no gravel

Parent Material (geologic) outwash Depth to Bedrock _____
 Depth to Groundwater: Standing Water in Hole: _____ Weeping from Pit Face 58"
 Estimated Seasonal High Groundwater _____

DETERMINATION FOR SEASONAL HIGH WATER TABLE

Method Used:

____ Depth observed standing in observation hole: _____ inches ☒ Depth to soil mottles: _____ inches
 ____ Depth to weeping from side of observation hole: _____ inches ____ Groundwater adjustment _____ ft
 Index Well # _____ Reading Date _____ Index well level _____ Adj.factor _____ Adj.Groundwater level _____

PERCOLATION TEST

Date _____ Time _____

Observation Hole # _____ Time at 9" _____
 Depth of Perc _____ Time at 6" _____
 Start Presoak _____ Time (9"-6") _____
 End Presoak _____ Rate Min/Inch _____

Site Suitability Assessment: Site Passed _____ Site Failed _____ Additional Testing Needed:

Performed By _____ Certification # _____

Witnessed By _____

Comments: